

MICHAEL J. LAMOREAUX, P.E. (NY, NJ, PA, VT, VA & CT) MICHAEL W. WEEKS, P.E. (NY, NJ & PA) LYLE R. SHUTE, P.E., LEED-AP (NY, NJ, PA) PATRICK J. HINES Main Office 33 Airport Center Drive Suite 202 New Windsor, New York 12553

(845) 567-3100 fax: (845) 567-3232 e-mail: <u>mheny@mhepc.com</u>

Principal Emeritus: RICHARD D. McGOEY, P.E. (NY & PA)

TOWN OF NEWBURGH PLANNING BOARD TECHNICAL REVIEW COMMENTS

PROJECT:ZAZON- 11 LOT SUBDIVISIONPROJECT NO.:04-29PROJECT LOCATION:SECTION 20, BLOCK 1, LOT 24REVIEW DATE:26 MARCH 2021MEETING DATE:1 APRIL 2021PROJECT REPRESENTATIVE:PIETRZAK & PFAU

- **1.** Project is before the Board to consider a Final Public Hearing in compliance with Subdivision Regulation 163-8 J (2).
- 2. In response to the Lead Agency circulation the Orange County Health Department has determined that the subdivision approval has expired and a request for re-approval or extension is required.
- 3. Approval of a road name by the Town of Newburgh Town Board is required.
- 4. Updated cost estimates for public improvements must be submitted.
- **5.** Long term operation and maintenance of the rain gardens on each lot has been previously discussed. Planning Board Attorney's comments regarding whether maintenance of this can be incorporated into a deed or other legal document should be identified.
- **6.** Ownership of the detention pond facility should be discussed or whether a Drainage District Lot should be created.
- **7.** The updated cost estimates for the stormwater management improvements, landscaping, water main, and roadway construction are required.
- **8.** Approval from the Orange County Department of Public Works for the access road and the utility construction is required.
- **9.** Project proposes eleven residential lots, one of which contains an existing structure. ARB Approval or deferral by the Planning Board to the Building Department is required.

Regional Office • 111 Wheatfield Drive • Suite 1 • Milford, Pennsylvania 18337 • 570-296-2765 •



Member

Respectfully submitted,

McGoey, Hauser and Edsall Consulting Engineers, D.P.C.

Afones when

Patrick J. Hines Principal

PJH/kbw

PIETRZAK & PFAU

ENGINEERING & SURVEYING, PLLC

RECEIVED	7
MAR 2 2021	
MCGOEY, HAUSER, AND EDSALL CONSULTING ENGINEERS D.P.C.	

Mr. John Ewasutyn, Chairman Town of Newburgh Planning Board 21 Hudson Valley Professional Plaza Newburgh, NY 12550

Re: Lands of Zazon (Ref. No. 2004-29) P&P No. 23153.01

Dear Mr. Ewasutyn:

In reference to the above project, attached please find fourteen (14) copies of the updated Long Form EAF (with original neg dec and agency letters), as well as a check for escrow in the amount of \$2,000.00, as requested.

Thank you for your attention to this matter. Should you have any questions or require anything further, please do not hesitate to contact this office.

Very truly yours,

February 24, 2021

PIETRZAK & PFAU, PLLC

Caleb Pawelski, Engineer

CJP/tmp

cc: Client Pat Hines, w/Long EAF Dominick Cordisco, Esq., w/Long EAF Kenneth Wersted, P.E., w/Long EAF Karen Arent, RLA, w/Long EAF

PIETRZAK & PFAU

ENGINEERING & SURVEYING, PLLC

February 16, 2021

FEB 1 9 2021

RE((

MCGOEY, HAUSER, P CONSULTING ENGINEERS

Mr. John Ewasutyn, Chairman Town of Newburgh Planning Board 308 Gardnertown Road Newburgh, NY 12550

Re: Lands of Zazon (Ref. No. 2004-29) P&P No. 23153.01

Dear Mr. Ewasutyn:

In reference to the above project, attached please find fourteen (14) copies of the revised subdivision map, as well as one (1) copy of the revised SWPPP.

Please place this item on the next available Planning Board agenda for consideration of setting a public hearing for final approval.

Thank you for your attention to this matter. Should you have any questions or require anything further, please do not hesitate to contact this office.

Very truly yours,

PIETRZAK & PFAU, PLLC

Caleb Pawelski, Engineer

CJP/tmp cc: Client Pat Hines, w/attachments

262 GREENWICH AVENUE, SUITE A GOSHEN, NEW YORK 10924 (845) 294-0606 · FAX (845) 294-0610

2 HAMILTON AVENUE
 MONTICELLO, NEW YORK 12701
 (845) 796-4646 • FAX (845) 796-4092

Lands of Zazon Subdivision

Town of Newburgh Orange County, New York

Stormwater Pollution Prevention Plan

Narrative

PIETRZAK & PFAU ENGINEERING & SURVEYING, PLLC 262 GREENWICH AVENUE GOSHEN, NEW YORK 10924

P&P No. 23153.01 February 2005 Rev. May 2005 Rev. Sept. 2005 Rev. Feb. 2011 Rev. Feb. 2021

TABLE OF CONTENTS

- I. Executive Summary
- II. Existing Conditions
- III. Proposed Conditions
- IV. Stormwater Management
- V. Stormwater Quality and Runoff Reduction
- VI. Erosion and Sediment Control
- VII. Stormwater Infrastructure Maintenance

APPENDICES

- 1. Stormwater Pollution Prevention Plan (SWPPP) Certifications
- 2. MS4 Stormwater Pollution Prevention Plan (SWPPP) Acceptance Form
- 3. Draft Notice of Intent (NOI)
- 4. Drainage Basin Maps
- 5. TR-20 Hydro-CAD Calculations Existing Conditions
- 6. TR-20 Hydro-CAD Calculations Proposed Conditions
- 7. TR-20 Supporting Data
- 8. Stormwater Quality and Runoff Reduction Calculations & Supporting Data
- 9. Soil Restoration
- 10. Outlet Protection Calculations
- 11. Pipe Sizing Calculations
- 12. State Pollutant Discharge Elimination System for Construction Activities Construction Site Log Book
- New York State Department of Environmental Conservation Permit No. GP-0-20-002

I.

Executive Summary

This report shall serve as a Stormwater Pollution Prevention Plan for the Lands of Zazon Subdivision. The proposed project is located on the northerly side of Fostertown Road, approximately 930 feet northwest of the intersection of Fostertown Road and Frozen Ridge Road. The project is currently identified as Section 20, Block 1, Lot 24 on the Town of Newburgh Tax map. The total site area is approximately $13.56\pm$ acres, with multiple existing structures on the site.

The project includes the construction of ten (10) single family houses in addition to one (1) existing dwelling, and approximately $550\pm$ feet of new Town Road, with associated improvements. The proposed lots are to be served by a water main extension and individual septic systems.

The drainage area has been taken to include all on-site area within the parcel, contributing approximately 13.56 acres. The remainder of the drainage area consists of $5.59\pm$ acres of off-site area tributary to the existing culvert pipe spanning Frozen Ridge Road, near the intersection at Foster Town Road. The total study area was $19.15\pm$ acres.

The drainage design for this project has been incorporated to provide the appropriate water quality treatment to the stormwater runoff, utilize proposed runoff reduction practices, and ensure there are no adverse impacts to the downstream areas of the project site. In order to meet these goals, a TR-20 hydraulic analysis of the stormwater runoff has been prepared utilizing HydroCAD Stormwater Modeling software. The software was used to analyze the Channel Protection (1 year), Overbank Flood (10 year), 25 year, and Extreme Storm (100 year) storm events in accordance with the New York State Stormwater Management Design Manual. Based on this analysis, the proposed design provides the required stormwater mitigation to ensure that no adverse impact will occur to downstream areas due to the construction of the proposed project.

II. Existing Conditions

As previously mentioned, the Lands of Zazon Subdivision is located on the northerly side of Fostertown Road, approximately 930 feet northwest of the intersection of Fostertown Road and Frozen Ridge Road. The parcel contains Federal wetlands with all proposed construction and/or disturbances located outside the delineated wetland boundary as delineated by Lawler, Matusky & Skelly.

The site soils have been identified in accordance with the Orange County Soils Survey. The site consists of Mardin, Erie and Swartswood Soils, all of which are in Hydrologic Soils Group C.

For the purposes of the Pre-Developed Conditions HydroCad Analysis, the site has been taken to consist of three (3) distinct subcatchments, with three (3) ultimate design points. The

1

subcatchments have been defined based upon the ridge line running through the property, and the high point in the drainage system on the northwesterly side of Frozen Ridge Road. The drainage system running along Frozen Ridge Road ultimately drains through the culvert located at the intersection of Frozen Ridge Road and Fostertown Road.

Subcatchment No. 1S has been defined as the on-site area directly tributary to the on-site culvert pipe spanning Fostertown Road. This area consists of approximately $9.2\pm$ acres. Land cover within this subcatchment includes $6.37\pm$ acres of woodlands, 2.642 acres of grass cover, and $0.188\pm$ acres of impervious area. The upstream inverts of the culvert pipe for which Subbasin 1S is tributary to, has been modeled in HydroCad as Design Point No. 1P (On-site Culvert Pipe).

Subcatchment No. 2S consists of the on and off-site areas directly tributary to the HydroCad Design Point 2P. This area consists of approximately $3.41\pm$ acres, of which $0.02\pm$ acres is impervious area. This subcatchment also consists of $2.01\pm$ acres of woods, $1.38\pm$ acres of grasslands. Subcatchment 2S is tributary to Design Point 2P, a theoretic design point.

Subcatchment No. 3S consists of the on and off-site area ultimately tributary to the existing southeasterly culvert pipe spanning Frozen Ridge Road, at the intersection with Fostertown Road. This area consists of approximately $6.54\pm$ acres, and is composed $0.58\pm$ acres of woodland, $4.99\pm$ acres of grassland, and $0.97\pm$ acres of impervious surfaces. The upstream invert of the culvert pipe, which Sub-catchment No. 3S is tributary to, has been modeled in HydroCad as Design Point 3P (off-site culvert pipe). Approximately 1/3 of the existing conditions basin 3S flows directly at the back wall of an existing building on the adjoining property N/F Hagar. The rear wall of this building appears to act as a diversion berm, and runoff eventually flows around the building and continues to run down hill to design point 3P. At a site inspection, evidence of water damage was apparent on the rear wall of this building.

III. Proposed Conditions

The proposed development has modified the basins as delineated in the existing conditions. Basin 1S has been reduced in size from $9.2\pm$ acres to $7.06\pm$ acres in the proposed conditions. The Post Conditions Basin 1S is comprised of $3.79\pm$ acres of woods, $0.2\pm$ acres of impervious area, with the remaining $3.07\pm$ acres being grasslands. Basin 1S is directly tributary to the on-site culvert pipe spanning Fostertown Road modeled in HydroCad as Design Point 1P.

Basin 2S has been reduced in size to comprise approximately $2.97\pm$ acres in the proposed conditions of which $0.996\pm$ acres is woodland, and $1.794\pm$ acres is grasslands, and $0.18\pm$ acres is impervious areas. Runoff from this area is tributary to the design point modeled in HydroCad as Design Point 2P.

Basin 3S, being the area flowing directly to Design Point 3P, the existing off-site culvert pipe spanning Frozen Ridge road, has been reduced in size for the proposed conditions. Proposed Basin 3S includes $0.15\pm$ acres of woodlands, $0.79\pm$ acres of impervious area, and $4.31\pm$ acres of grasslands, totaling $5.25\pm$ acres.

The majority of the on-site developed area runoff has been captured in the proposed road infrastructure, and conveyed to Type F-5 Bioretention Pond 4P. Bioretention Pond 4P, in turn, outlets to the drainage ditch running along Fostertown Road. The outlet of the Bioretention Pond 4P, and the drainage ditch along Fostertown Road are directly tributary to the upstream invert of the culvert pipe modeled in HydroCad as Pond 3P.

The $3.87\pm$ acres of runoff captured in the proposed road infrastructure has been modeled as Basin 4S in the proposed conditions, and includes $1.7\pm$ acres of grasslands, $1.07\pm$ acres of woodlands, $1.1\pm$ acres of impervious surfaces, including the proposed water surface.

The runoff from Basins 4S will be captured and routed into the aforementioned Bioretention Pond, identified in the HydroCad Analysis as Pond 4P. Flows from Pond 4P will be routed via a control structure draining into the road side ditch located along Fostertown Road, and flowing to the culvert spanning Frozen Ridge Road (Design Point 3P). The ditch running along Fostertown Road has been modeled in HydroCad as Reach R5, and is tributary to Design Point 3P.

The outlet from the stormwater pond is being conveyed to the swale running along the northeasterly side of Fostertown Road, in an attempt to reduce the runoff that is directed towards the back of the building on the adjoining lot N/F Hagar. Much of the runoff that was tributary to the back of the adjoining building in existing conditions has, in proposed conditions, been captured in the stormwater pond and conveyed to the existing roadside swale. The roadside swale and all driveway culverts located between the outlet of the stormwater pond and the culvert spanning Frozen Ridge Road (Design Point 3P) have been analyzed in the existing and proposed conditions. The swale along Fostertown Road has been modeled using its maximum and minimum slope, and has been determined to have sufficient capacity to handle the increased flow from the outlet of the stormwater pond. It is proposed to replace the five (5) existing 12" CMP culverts downstream of the outlet with 15" HDPE culvert pipes to accommodate the increase in flow through the roadside swale. The calculations for the five (5) driveway culverts and roadside swale can be found in Appendix 11.

IV. Storm Water Management

The proposed conditions analysis has been performed to ensure that the project will create no downstream detrimental impacts. To meet this goal, one (1) Type F-5 Bioretention Pond will be utilized to collect and treat stormwater runoff and provide peak flow attenuation for the design points studied for this project. A Hydro-Cad TR-20 analysis was performed for both the existing and proposed conditions for the Channel Protection (1 year), Overbank Flood (10 year), and Extreme storm (100 year) storm events to ensure no adverse impacts will occur to downstream areas.

The proposed Type F-5 Bioretention Pond will collect the stormwater runoff from Subcatchment 4S, as defined in the Proposed Conditions above. This basin has been designed in accordance with Section 6.1 of the New York State Stormwater Management Design Manual to provide the required pretreatment volume, Water Quality volume and provide peak flow attentuation. The pretreatment sedimentation basin has been sized to provide 25% of the water quality volume for Subcatchment 4S. The pond has been designed with 3 horizontal to 1 vertical (3:1) interior side slopes. The pond will have a bottom pond elevation of 417.0' and a pretreatment bottom elevation of 416.0'. The pond will control the overflow stormwater from Subcatchment 4S by utilizing one (1) proposed Outlet Control Structure (OCS-1) and is proposed to outlet via a 18" HDPE pipe into Design Point 3. The outlet control structure is designed to control all design storm events studied by allowing stormwater flow to be released over time. Additionally, a twenty (20) foot wide emergency overflow broad crested weir has been incorporated into the pond design.

Runoff from the roof leaders for all proposed dwellings not tributary to the proposed stormwater pond are proposed to connect to individual rain gardens. These rain gardens are being proposed to provide water quality treatment for the runoff from the roofs of proposed dwellings not tributary to the stormwater pond. Much of the existing conditions runoff flowing to the rear of the building on the adjoining property of N/F of Hagar has, in the post development conditions, been captured in the proposed detention pond. In the proposed conditions, the amount of runoff flowing towards the rear wall of the adjoining building has been reduced, improving an existing drainage problem by outletting the proposed detention pond into the swale running along Fostertown Road. Preliminary correspondence with the Orange County DPW indicated that outletting the stormwater detention pond into the roadside swale would be acceptable, as it is being proposed in an attempt to improve an existing drainage problem.

The tables below indicate the pre and post-development flows at Design Points 1P, 2P and 3P, as well as the net changes in these flow rates. The data presented in these tables represents the results of analysis of the 1, 10, 25 and 100 year storms. As can be seen in these tables, a decrease has been provided for these design points for each of the design storms. The flows to the two (2) existing culverts spanning Fostertown Road and Frozen Ridge Road have both been decreased in the proposed conditions analysis. The decrease in flows to these two (2) culverts provide a significant benefit of reducing any potential flooding conditions over these roads.

<u>Design Point 1</u>										
Storm Event	Pre-Developed Peak Flow (cfs) Q out	Post-Developed Peak Flow (cfs) Q out	Change (cfs)	Change (%)						
1 Year	4.61	4.04	-0.57	-12.36						
10 Year	16.06	14.23	-1.83	-11.39						
25 Year	23.59	20.90	-2.69	-11.40						
100 Year	39.6	35.06	-4.54	-11.46						

]	Design Point 2		
Storm Event	Pre-Developed Peak Flow (cfs) Q out	Post-Developed Peak Flow (cfs) Q out	Change (cfs)	Change (%)
1 Year	1.65	1.56	-0.09	-5.45
10 Year	5.75	5.22	-0.53	-9.22
25 Year	8.46	7.60	-0.86	-10.17
100 Year	14.20	12.64	-1.56	-10.99

Design Point 3									
Storm Event	Pre-Developed Peak Flow (cfs) Q out	Post-Developed Peak Flow (cfs) Q out	Change (cfs)	Change (%)					
1 Year	4.56	4.17	-0.39	-8.55					
10 Year	14.08	12.51	-1.57	-11.15					
25 Year	20.14	17.92	-2.22	-11.02					
100 Year	32.78	31.31	-1.47	-4.48					

Additionally, the table below has been provided showing the water surface elevations in the proposed stormwater ponds. The elevations presented in this table illustrates the results of the analysis for the 1, 10, and 100 year design storms. The elevations indicate that a minimum of one (1) foot of freeboard has been provided in the pond to protect against overtopping.

<u>Proposed Type F-5 Bioretention Pond</u> <u>4P</u>									
Storm Event	Post- Developed Peak Water Surface Elevation	Freeboard (ft.) (Pond Berm at 422.00')							
1 Year	418.42	3.58							
10 Year	419.29	2.71							
100 Year	420.92	1.08							

V. Storm Water Quality

The stormwater water quality and runoff reduction for this project has been designed in accordance with the New York State Department of Environmental Conservation Stormwater Management Design Manual (SMDM) of January 2015. The five-step planning process outlined in the SMDM has been incorporated in the design of this project. These five steps include:

- 1. Site planning to preserve natural features and reduce impervious cover.
- 2. Calculation of the Water Quality Volume for the site.
- 3. Incorporation of Runoff Reduction Techniques and Standard SMPs with Runoff Reduction Volume (RRv) capacity.
- 4. Use of Standard SMPs, where applicable, to treat the portion of Water Quality Volume not addressed by runoff reduction techniques and Standard SMPs with RRv capacity.
- 5. Design of volume and peak rate control practices, where required.

Step one of the planning process includes the preservation of natural features and reduction of impervious coverage. Reduction of clearing and grading has been accomplished by the use of clearly delineated disturbance limit lines proposed on the project design plans.

Step two of the planning process was then completed and the Water Quality Volume (WQv) was calculated for the project site using the criteria in Chapter 4 of the Stormwater Management Design Manual. The required Water Quality Volume calculated for this project is 10,187 cubic feet (See Appendix 8 for Calculations and Supporting Data).

Step three of the process involves Runoff Reduction by incorporating the Runoff Reduction Techniques and Standard SMP's with RRv capacity outlined in the Stormwater Management Design Manual. The minimum Runoff Reduction Volume was then calculated utilizing the Specific Reduction Factor of the existing soil types located on the project site using the criteria in Chapter 4 of the design manual. The minimum RRv calculated for this project is 1,513 cubic feet (See Appendix 8 for Calculations and Supporting Data).

Runoff Reduction Technique RR-7 Rain Gardens in accordance with Section 5.3.7 of the SMDM is proposed to capture stormwater runoff from proposed houses not directly tied into the proposed road drainage system. Two 200 square foot rain gardens connected to roof drains are proposed for each house, with 18 proposed rain gardens in total. The two rain gardens per lot have been designed for 1,500 square feet of impervious area and each rain garden provides 180 square feet of Water Quality Volume. Due to the soil type on the project site being Hydrologic Soil Group C and D, 40% of trhe WQv provided by each of the rain gardens has been removed from the Water Qaulity Volume calculations resulting in a remaining WQv of 8,891 cubic feet.

The other Stormwater Management Practice with RRv capacity utilized for this project is the Type F-5 Bioretention Pond. The proposed Bioretention Pond captures the new impervious surfaces proposed for the project and has been designed to capture and treat runoff by filtration through a soil mixture. The stormwater is collected and directed to an outlet via a pond underdrain. The Bioretention Pond is located within Hydrologic Soil Group D, therefore 40% of the WQv provided by this practice has been removed from the Runoff Reduction Volume calculation resulting in a remaining WQv of 6,040 cubic feet. The Bioretention Pond has been designed manage the pre-developed and post-developed peak flow rates for the project.

Utilizing step three of the design process, the Runoff Reduction Volume provided for the project is 8,448 cubic feet, which exceeds the required RRv of 3,940 cubic feet.

Step four of the process involves applying Standard Stormwater Management Practices to address the remaining Water Quality Volume. The Bioretention Pond has been sized to provide the remaining Water Quality Volume for the site, so applying further SMPs is not necessary. The remaining Green Infrastructure Runoff Reduction Techniques and standard SMP's with RRv capacity have been evaluated and determined to be infeasible for use with the proposed project. The existing site topography include poor hydrologic soils and restricted space on-site. This along with cold climate concerns eliminates the possible incorporation of Sheetflow to Riparian Buffers, Disconnection of Rooftop Runoff, Stream Daylighting, Stormwater Planter, Rain Tank/Cisterns and also the use of Porous Pavement as a Runoff Reduction practice.

Step five of the process involves applying Volume and Peak Rate Control Practices. The downstream channel protection has been provided within the proposed pond by providing a release rate of equal to or lesser value specified by the Stream Channel Protection Volume. The outflow from the pond for this storm event has been designed such that runoff will be discharged over a 24 hour period after the design storm event. The Overbank Flood (10 year storm event) and the Extreme Storm (100 year storm event) have been managed as outlined in the Stormwater Management section of this report.

Channel Protection	n Volume (CPv) Summa	ary Table
Practice	Release Rate Required (cfs)	Release Rate Provided (cfs)
Bioretention Pond 4P	0.14	0.14

VI. Erosion and Sediment Control

Full erosion and sediment control measures will be incorporated into the project construction. These practices will be in accordance with those set forth in the New York State Department of Environmental Conservation publication entitled "Stormwater Management Design Manual".

Erosion Control Measures:

The following erosion control measures will be incorporated to minimize erosion potential:

• Filter fabric silt fence:

Silt fence shall be used to control erosion from sheet flow on slopes not to exceed two horizontal to one vertical. Concentrated flows shall not be directed toward silt fence and spacing shall vary from 50' to 200' depending on slope steepness.

• <u>Permanent and temporary seeding mixtures</u>:

Permanent and temporary seeding, mulch, fertilizer, soil amendments, and slope stabilization will be used on seeded areas. Land that is stripped of vegetation will be left bare for the shortest time possible. Any area that will remain cleared, but not under construction for 20 days or longer, will be seeded with a temporary mixture. Topsoil shall be stockpiled, stabilized with temporary seeding, and saved for reuse on the site.

• <u>Slope Stabilization</u>:

All slopes shall be stabilized to minimize erosion. Slopes shall be stabilized with temporary seeding mixtures and straw mulch. Slopes in excess of four horizontal to one vertical shall be stabilized with jute netting and hydro-seed. Existing vegetation, which is not to be removed, will also act as filter strips to protect down-slope areas. Runoff will be diverted from newly graded areas to prevent erosion until a permanent ground cover has been established.

<u>Dust Control:</u>

Measures for dust control during construction shall be implemented as needed (daily water sprays will be used during dry conditions and Calcium Chloride will be used only if necessary). In addition to water sprays, temporary plantings will aid in minimizing dust.

• Temporary Diversion Swales:

Temporary diversion swales shall be constructed to either divert clean storm water runoff from newly graded areas or direct sediment laden runoff to a sediment trapping device.

• Channel Stabilization:

Drainage channels and temporary diversion swales shall be stabilized with seed, jute netting or riprap, as specified, to minimize deterioration of the channel bed.

• Sediment Traps:

Sediment traps shall be constructed in the location, and be of size and type specified to collect sediment from sediment laden storm water runoff. Sediment traps shall be constructed downstream of disturbed areas and be in place prior to disturbance within the contributory area.

• Stabilized Construction Entrance:

Existing roads will be protected by installation of crushed stone blanket for cleaning construction vehicle wheels. Blankets shall be placed at any intersection of a construction road with a paved or publicly owned road. Stabilized construction entrances shall be installed in the location and be of size and type specified.

• <u>Tree Protection</u>:

Trees to be preserved within areas of construction shall be protected. In areas of concentrated construction activity temporary fencing will be placed around the driplines. In all other areas, construction workers will be directed to avoid the storing of equipment or soil under trees to be preserved, in order to prevent soil compaction. If necessary, trees will be preserved with tree wells in fill areas, and retaining walls in cut areas.

Erosion Control Sequence

Prior to any site disturbance, the developer should thoroughly review and familiarize the approved erosion control plan. The installation of erosion control measures should begin with the most downstream device, then working upstream. When installing erosion control measures, the sequence should generally be as follows:

- Prior to commencing construction activities, the limits of clearing and grading shall be clearly marked. Perimeter silt fence and stabilized construction entrances shall be put in place.
- Upon completion of clearing and grubbing activities, topsoil shall be stripped from all areas to be disturbed and stockpiled. Stockpiled topsoil shall be stabilized by temporary seeding and surrounded with a perimeter silt fence.
- Temporary erosion control devices shall be installed prior to commencing earth moving activities. This includes the installation of sediment traps, diversion swales, and check dams beginning at the most downstream portions of the site and then working upstream.
- Immediately after completion of rough grading, remaining temporary erosion control shall be installed as specified, including additional silt fence, diversion swales, and check dams. Any areas not requiring further earth work shall be fine graded topsoiled and stabilized as early as possible.

Maintenance of Erosion Control Devices

The maintenance of erosion control devices will be the responsibility of the developer. A critical part of an effective erosion control plan is a conscientious maintenance program. All erosion control devices will be cleaned and restored throughout construction to maintain their effectiveness. The Job Superintendent will monitor the condition of all devices and clean or replace them, as conditions require. All erosion control devices shall be installed and maintained in accordance with the approved plans, manufacturer's recommendations, and as directed by town representatives including the town engineer, highway superintendent, and building inspector.

Specific maintenance shall include:

- Maintaining seeded areas including reseeding weak areas, regrading wash outs and fertilizing.
- ^o Maintaining mulched areas including replacement of disturbed mulched areas.
- All devices shall be inspected after each rain and repaired as needed.
- Sediment shall be removed from behind silt fence when bulges start to occur and fencing reset to original condition.
- Outlets in sediment basins shall be free of silt and debris by hand raking and cleaning after each rain storm.
- Construction equipment shall not unnecessarily cross drainage swales. Crossing of drainage channels shall be by means of bridges, culverts or other approved methods.
- ^o Culverts shall be maintained free of silt or debris.
- Tree protection fencing to be inspected daily during grading and finish grading operations.
- Daily water sprays will be used as needed or as directed by the Consulting Engineer or Town representatives. Water sprays will be used to prevent pollution from dust until construction is completed and soil cover is established.

Removal of Erosion Control Devices:

No erosion control structures shall be removed until all work upstream has been completed, stabilized, and approved by the Consulting Engineer and Town Representatives.

The removal of erosion control devices should generally be as follows:

- After construction, the temporary erosion control structures are to be removed in reverse order with the most upstream structure removed first and thence proceeding downstream.
- All hay bales shall be removed and properly disposed of off-site.
- All tree protection fencing shall be removed after adjacent areas have been graded, topsoiled, seeded, and vegetation has been established.
- All temporary construction culverts shall be removed and areas graded, topsoiled, and seeded.
- Any washouts shall be re-topsoiled and seeded.

VII. Storm Water Infrastructure Maintenance:

The responsibility of the long term maintenance of all drainage structures, pipes, and treatment devices located within the proposed roadway parcel will be the responsibility of the developer until such time as the Town accepts dedication of the roadway. The work required will be financed through the creation of a drainage district.

Long term maintenance shall include the following:

<u>Inspection</u>: The drainage infrastructure should be inspected after major storm events to ensure that the orifices, if any, and inlets remain open. Particular attention should be given to:

- Evidence of clogging
- Erosion of the flow path
- Condition of the embankments

- Condition of the spillways
- Accumulation of sediment at the outlets and sumps
- Erosion of bio-swales or riprap aprons

• Sources of erosion in the contributory drainage which should be stabilized.

<u>Debris and Litter Control</u>: Removal of debris and litter should be accomplished during mowing operations. Particular attention should be given to removing debris and trash around outlets to prevent clogging.

<u>Erosion Control</u>: Eroding soils in drainage areas should be stabilized immediately with vegetative practices or other erosion control practices. Potential problems are erosion that may occur on the embankment, slopes, and spillways of grassed bio-swales. Also, attention should be given to repositioning protective riprap where appropriate.

<u>Sediment Removal</u>: Sediment should be removed periodically in order to preserve the available storm water treatment capacity of the infiltration devices, and to prevent outlets or filtering mediums from becoming clogged. Also, unless removed, accumulated sediment may become unsightly. While more frequent cleanout may be needed around outlets, at typical clean-out cycle for the entire storm water infrastructure should range form 5 to 10 years. The following specific items will require more frequent cleaning:

- Catch basins must be cleaned out at least once a year.
- Grass in the designated grassed swales is to be maintained at a height of 4 to 6 inches.
- Sediment is to be removed from the grassed swales upon an accumulated sediment level of 25% of the original swale depth.

X. <u>Stormwater Conveyance System Analysis</u>

Stormwater management for the Lands of Zazon project will be accomplished via a network of pipes, drainage swales, pipe culverts, and treatment and storage facilities. In sizing the drainage pipes and culverts an analysis was performed using the Rational Method to study all drainage areas tributary to the drainage facilities within the site. In performing the Rational Method analysis, the values of the runoff coefficient (C) were taken to be 0.90 for all impervious areas, and 0.3 for pervious areas. Rainfall intensity was taken from the Orange County Rainfall Intensity Curves for the 25 year storm event based upon a calculated time of concentration. A minimum time of concentration of 10 minutes has been utilized. Data and calculations can be found in Appendix 11.

Additionally, calculations have been provided in Appendix 11 showing the analysis of the off-site swale running along Fostertown Road, and the driveway culverts located along this swale. The swale was modeled using the minimum and maximum slopes of the swale, and was found to have sufficient capacity to handle the additional flow being received from the stormwater pond. The driveway culverts along Fostertown Road have been analyzed in both existing and proposed conditions. The proposed conditions analysis verifies that the 15" HDPE culverts being proposed to replace the existing 12" CMP culverts have sufficient capacity to handle the additional flow replace the existing 12."

11

CJP/tmp 02-03-05 rev. 05-23-05 rev. 09-13-05 rev. 02-01-11 rev. 02-11-21

APPENDIX

 $\left[\right]$

L

APPENDIX 1

1

Stormwater Pollution Prevention Plan (SWPPP) Certifications

I. Owner/Operator Information:

PROJECT:

LOCATION:

RECORD OWNER:

S&E Construction

Town of Newburgh.

Lands of Zazon

OWNER/APPLICANT ADDRESS:

5 Suhl Lane Monsey, New York 10952

Orange County, New York

PROJECT SITE ADDRESS:

Fostertown Road Newburgh NY, 12550

II. Certifications:

Contractor and Subcontractor Certification:

I hereby certify under penalty of law that I understand and agree to comply with the terms and conditions of the Storm Water Pollution Prevention Plan (SWPPP) and agree to implement any corrective actions identified by the qualified inspector during a site inspection. I also understand that the owner or operator must comply with the terms and conditions of the most current version of the New York State Pollutant Discharge Elimination System ("SPDES") general permit for stormwater discharges from construction activities and that it is unlawful for any person to cause or contribute to a violation of water quality standards. Furthermore, I am aware that there are significant penalties for submitting false information that I do not believe to be true, including the possibility of fine and imprisonment for knowing violations.

Contractor responsible for project oversight:

Contractor

Signature

Date

Print Name & Title

Name of Trained Contractor

Phone:

Address:

Subcontractor responsible for onsite construction and maintenance of erosion and sediment control practices and post-construction stormwater management practices included in the SWPPP:

Subcontractor

Print Name & Title

Signature

Date

Address:

Phone:

Name of Trained Contractor

Additional Subcontractors and responsibility:

Subcontractor

Signature

Name of Trained Contractor

Print Name & Title

Date

Address:

Phone:

Subcontractor

Signature

Name of Trained Contractor

Print Name & Title

Date

Address:

Phone:

APPENDIX 2

}

 $\left[\right]$

Γ

L

MS4 Stormwater Pollution Prevention (SWPPP) Acceptance Form



New York State Department of Environmental Conservation Division of Water 625 Broadway, 4th Floor Albany, New York 12233-3505

MS4 Stormwater Pollution Prevention Plan (SWPPP) Acceptance Form

for

Construction Activities Seeking Authorization Under SPDES General Permit *(NOTE: Attach Completed Form to Notice Of Intent and Submit to Address Above)

I. Project Owner/Operator Information

1. Owner/Operator Name: S&E Construction

2. Contact Person: Joseph Zazon

3. Street Address: 5 Suhl Lane

4. City/State/Zip: Monsey, New York 10952

II. Project Site Information

5. Project/Site Name: Lands of Zazon

6. Street Address: Fostertown Road

7. City/State/Zip: Newburgh, NY 12550

III. Stormwater Pollution Prevention Plan (SWPPP) Review and Acceptance Information

8. SWPPP Reviewed by:

9. Title/Position:

10, Date Final SWPPP Reviewed and Accepted:

IV. Regulated MS4 Information

11, Name of MS4:

12, MS4 SPDES Permit Identification Number: NYR20A _____

13. Contact Person:

14. Street Address:

15. City/State/Zip:

16, Telephone Number:

(NYS DEC - MS4 SWPPP Acceptance Form - January 2010)

MS4 SWPPP Acceptance Form - continued

V. Certification Statement - MS4 Official (principal executive officer or ranking elected official) or Duly Authorized Representative

I hereby certify that the final Stormwater Pollution Prevention Plan (SWPPP) for the construction project identified in question 5 has been reviewed and meets the substantive requirements in the SPDES General Permit For Stormwater Discharges from Municipal Separate Storm Sewer Systems (MS4s).

Note: The MS4, through the acceptance of the SWPPP, assumes no responsibility for the accuracy and adequacy of the design included in the SWPPP. In addition, review and acceptance of the SWPPP by the MS4 does not relieve the owner/operator or their SWPPP preparer of responsibility or liability for errors or omissions in the plan.

Printed Name:

Title/Position:

Signature:

Date;

VI. Additional Information

APPENDIX 3

Γ

Γ

Draft Notice of Intent (NOI)

NOTICE OF INTENT



Division of Water

625 Broadway, 4th Floor Albany, New York 12233-3505



Stormwater Discharges Associated with <u>Construction Activity</u> Under State Pollutant Discharge Elimination System (SPDES) General Permit # GP-0-15-002 All sections must be completed unless otherwise noted. Failure to complete all items may result in this form being returned to you, thereby delaying your coverage under this General Permit. Applicants must read and understand the conditions of the permit and prepare a Stormwater Pollution Prevention Plan prior to submitting this NOI. Applicants are responsible for identifying and obtaining other DEC permits that may be required.

-IMPORTANT-

RETURN THIS FORM TO THE ADDRESS ABOVE

OWNER/OPERATOR MUST SIGN FORM

-							Owne	er/Oj	pera	tor	Inf	orma	tio	'n							· · ·		
Owner	/Opera	ator	(Comp	any	Name	/Pri	.vaté	Owr	ier i	Name	/Mu	nici	pali	.ty	Name	•)		ing s La las			i ser 1 North		
														Ī									
Owner	/Opera	itor (Conta	ct P	erso	n La	st N	lame	(NO	T CC	NSU	LTAN	т)										
					ĪĪ							T	ĪŤ	T	Î	T					T		
	/Opera		lanka					Neme		.	1		1 I 			1,200						<u> </u>	
	/ Upera					<u>, r'r</u>			<u> </u>	\square				1		1		T			· 		<u> </u>
														- <u>.</u>	<u> </u>	<u> </u>					<u> </u>		<u> </u>
wner,	/Opera	itor I	Maili 	ng A	ddrê.	ss, ,						5			T	T		1				<u>т</u> т	<u>.</u>
									<u> </u>	ĻL				<u> </u>		<u> </u>				Ŀ.	<u> </u>	<u>ل_</u> ل	
ity_				<u>.</u>	1 1			.								 	i. İ F		T		<u></u>		<u> </u>
State		<u>Z</u>	ip													- 141 - 1 1 - 1 - 1 20				an fin An An fin			
	•														4			t_{i} .		а, 14			
hone	(Owne		arato	7 7			г.	 ax ((Jwno	~ / O		torl		7 9 ₆ 7			1.01			:	N ¹	• • • • • •	
												_		<u> </u>]	··· ·			· · ·.				
<u></u>		<u> </u>	1 L.					E		ĻĻĻ					_		1	· · .			. 1		
mail	(Owne	r/Ope	rato	<u>r) :</u>	1 1			• ·	<u></u>		- <u>i</u>		<u> </u>	·	r r								
			ÌT			ÌÌ		ΠŤ					\prod				<u> </u>	1	Π	Ť	T		
	X ID	<u>↓ </u>					: .:							1				- 1		.		┺	
		Î		Ţ,			المراجعة أو					1 ~ 1	i i Literati			en e Maria	÷ .						÷ .
		<u>. </u>		L] (not	requ	ιταec	I TOI	: 1n	ατνι	oua	L\$)			al de se Notes		i.						
				1		e di di Antonio		23 Ng 1									est. Alter			ан 41			
				111	÷			n. Ng se	•			1 <u>.</u>	1. j.,	1 .					· · · .		÷		1.1

· · · · · · · ·		and a second			1.542				•		
[Projec	t Site In	formatio	m .	. • • •	1.1				`
			م منظق المحمد بين مدير مرجع في				:	· · ·		-	
Project/Site N	ame		··· · · · · · · · · · · · · · · · · ·	·····			····				
J		1993 d.d.	t de la participa de la compacta de	1. A.						() () () () () () () () () () () () ()	
Street Address	(NOT P.O. B	OX)		•		•					
	그는 것 같은 것 같아요	ur segun dag	an isa	(p,t) in $($::.::. ! i i		: <u>:</u> : ·	1.1	1.11.11	
Side of Street				1947 - 194 1947 - 1947 - 1947 - 1947 - 1947 - 1947 - 1947 - 1947 - 1947 - 1947 - 1947 - 1947 - 1947 - 1947 - 1947 - 1947		: / · · · ·					
			i i								
City/Town/Vill	<u>age (THAT IS</u>	SUES BUILDIN	<u>G PERMIT)</u>			······					+ +
17			イ・イッチ いっちょうがくしょう				• •		• •		
· · · · · ·			hggarra.							•	
State Zip		Count						DE	C <u>Reg</u> i	.on .	, , .
State Zip									C Reg	.on	
	, -							DE	C Reg	.on	
								DE	C Reg	.on	
										on	
										on	
	t Cross Stre	et	y 1,247,249,442 1,247,249,442 1,448,449,442 1,448,444,444			In Re	latic				eet
N Y	t Cross Stre	et	y 1,247,249,442 1,247,249,442 1,448,449,442 1,448,444,444		oject.				Cros	 s Str	
N Y	t Cross Stre	et	y 1,247,249,442 1,247,249,442 1,448,449,442 1,448,444,444						Cros	 s Str	
N Y Name of Neares	t Cross Stre	et	y 1,247,249,442 1,247,249,442 1,448,449,442 1,448,444,444	Pr		0 So	uth	m to	Cross	 s Str	
N Y Name of Neares Distance to Ne	t Cross Stre	et	y 1,247,249,442 1,247,249,442 1,448,449,442 1,448,444,444	Pr	North	0 So	uth	m to	Cross	 s Str	
N Y Name of Neares	t Cross Stre	et	y 1,247,249,442 1,247,249,442 1,448,449,442 1,448,444,444	Pr	North	0 So	uth	m to	Cross	 s Str	

1. Provide the Geographic Coordinates for the project site in NYTM Units. To do this you must go to the NYSDEC Stormwater Interactive Map on the DEC website at:

www.dec.ny.gov/imsmaps/stormwater/viewer.htm

Zoom into your Project Location such that you can accurately click on the centroid of your site. Once you have located your project site, go to the tool boxes on the top and choose "i"(identify). Then click on the center of your site and a new window containing the X, Y coordinates in UTM will pop up. Transcribe these coordinates into the boxes below. For problems with the interactive map use the help function.

X Coordinates	Y C	oordina	ites	ing)
	4			

nature of this construction project?	
O New Construction	
O Redevelopment with no increase in impervious area	

[.

 $\left[\right]$

L

L

L

3. Select the predominant land use for both p SELECT ONLY ONE CHOICE FOR EACH	pre and post development conditions.
Pre-Development Existing Land Use	Post-Development Future Land Use
O FOREST	O SINGLE FAMILY HOME Number of Lots
O PASTURE/OPEN LAND	O SINGLE FAMILY SUBDIVISION
O CULTIVATED LAND	O TOWN HOME RESIDENTIAL
O SINGLE FAMILY HOME	O MULTIFAMILY RESIDENTIAL
O SINGLE FAMILY SUBDIVISION	O INSTITUTIONAL/SCHOOL
O TOWN HOME RESIDENTIAL	O INDUSTRIAL
O MULTIFAMILY RESIDENTIAL	O COMMERCIAL
O INSTITUTIONAL/SCHOOL	O MUNICIPAL
O INDUSTRIAL	O ROAD/HIGHWAY
O COMMERCIAL	O RECREATIONAL/SPORTS FIELD
O ROAD/HIGHWAY	O BIKE PATH/TRAIL
O RECREATIONAL/SPORTS FIELD	O LINEAR UTILITY (water, sewer, gas, etc.)
O BIKE PATH/TRAIL	O PARKING LOT
O LINEAR UTILITY	O CLEARING/GRADING ONLY
O PARKING LOT	O DEMOLITION, NO REDEVELOPMENT
O OTHER	O WELL DRILLING ACTIVITY * (Oil, Gas, etc.)
	O OTHER
*Note: for gas well drilling, non-high volume	
 *Note: for gas well drilling, non-high volume 4. In accordance with the larger common plan of enter the total project site area; the tota existing impervious area to be disturbed (f activities); and the future impervious area disturbed area. (Round to the nearest tenth 	of development or sale, al area to be disturbed; for redevelopment a constructed within the a of an acre.)
4. In accordance with the larger common plan of enter the total project site area; the tota existing impervious area to be disturbed (f activities); and the future impervious area disturbed area. (Round to the nearest tenth	of development or sale; al area to be disturbed; or redevelopment a constructed within the a of an acre.) Future Impervious
4. In accordance with the larger common plan of enter the total project site area; the tota existing impervious area to be disturbed (f activities); and the future impervious area disturbed area. (Round to the nearest tenth Total Site Total Area To Exist	of development or sale, al area to be disturbed; for redevelopment a constructed within the a of an acre.)
4. In accordance with the larger common plan of enter the total project site area; the total existing impervious area to be disturbed (f activities); and the future impervious area disturbed area. (Round to the nearest tenth Total Site Total Area To Exist	of development or sale; al area to be disturbed; or redevelopment a constructed within the a of an acre.) Future Impervious ting Impervious Area Within
4. In accordance with the larger common plan of enter the total project site area; the total existing impervious area to be disturbed (f activities); and the future impervious area disturbed area. (Round to the nearest tenth Total Site Total Area To Exist	of development or sale, al area to be disturbed; for redevelopment a constructed within the a of an acre.) Future Impervious Area Within Disturbed Area
 4. In accordance with the larger common plan of enter the total project site area; the total existing impervious area to be disturbed (factivities); and the future impervious area disturbed area. (Round to the nearest tenth Total Site Total Area To Exist Area Be Disturbed Area 5. Do you plan to disturb more than 5 acres of 6. Indicate the percentage of each Hydrologic 	of development or sale, al area to be disturbed; for redevelopment a constructed within the a of an acre.) Future Impervious Area Within To Be Disturbed Disturbed Area Soil at any one time? O Yes O No
 4. In accordance with the larger common plan of enter the total project site area; the total existing impervious area to be disturbed (factivities); and the future impervious area disturbed area. (Round to the nearest tenth Total Site Total Site Total Area Be Disturbed Area Be Disturbed Area So you plan to disturb more than 5 acres of each Hydrologic A B C B C So this a phased project? 	of development or sale, al area to be disturbed; for redevelopment a constructed within the a of an acre.) Future Impervious Area Within Disturbed Area
 4. In accordance with the larger common plan of enter the total project site area; the total existing impervious area to be disturbed (factivities); and the future impervious area disturbed area. (Round to the nearest tenth Total Site Total Area To Exist Area Be Disturbed Area 5. Do you plan to disturb more than 5 acres of 6. Indicate the percentage of each Hydrologic 	of development or sale, al area to be disturbed; for redevelopment a constructed within the a of an acre.) Future Impervious Area Within Disturbed Area

.

9: Identify the nearest surface waterbody(ies) to discharge.	which construction site runoff will:
Name	
	[1] 指令: 4:4 -
9a. Type of waterbody identified in Question 9?	
O Wetland / State Jurisdiction, On Site (Answer 9)	$_{ m o}$) where $c_{ m o}$ is the constant of the constant
O Wetland / State Jurisdiction Off Site	
O Wetland / Federal Jurisdiction On Site (Answer	9b)
O Wetland / Federal Jurisdiction Off Site	a di Banara da Banara. 1940 - Banara da Bana 1940 - Banara da Banar
O Stream / Creek On Site	"想你这些你是你的事情。"
Ó Stream / Creek Off Site	
O River On Site	How was the wetland identified?
O River Off Site	
O Lake On Site	O Regulatory Map
O Lake Off Site	O Delineated by Consultant
O Other Type On Site	O Delineated by Army Corps of Engineers
O Other Type Off Site	O Other (identify)
10. Has the surface waterbody(ies) in question 9	been identified as a O Yes O No

303(d) segment in Appendix E of GP-0-15-002?

Is this project located in one of the Watersheds identified in Appendix C of GP-0-15-002?	O Yes	O No
The second second.		

12.	Is the project located in one of the watershed		
	areas associated with AA and AA-S classified	O Yes	O No
	waters?		
	If no, skip question 13.		

Does this construction activity disturb land with no existing impervious cover and where the Soil Slope Phase is identified as an E or F on the USDA Soil Survey?	O Yes O No
If Yes, what is the acreage to be disturbed?	

14. Will the project disturb soils within a State regulated wetland or the protected 100 foot adjacent area?

O Yes O No

5764372699 Does the site runoff enter a separate storm sewer 🔅 15. system (including roadside drains, swales, ditches, O Yes O No O Unknown culverts, etc)? NE SERVER SERVER 16. What is the name of the municipality/entity that owns the separate storm sewer system? Does any runoff from the site enter a sewer classified O Yes O No. O Unknown as a Combined Sewer? 17 18. Will future use of this site be an agricultural property as O Yes O No defined by the NYS Agriculture and Markets Law? 19. Is this property owned by a state authority, state agency, federal government or local government? O Yes O No 20. Is this a remediation project being done under a Department approved work plan? (i.e. CERCLA, RCRA, Voluntary Cleanup O Yes O No Agreement, etc.) Has the required Erosion and Sediment Control component of the 21. SWPPP been developed in conformance with the current NYS O Yes O No Standards and Specifications for Erosion and Sediment Control. (aka Blue Book)? 22. Does this construction activity require the development of a SWPPP that includes the post-construction stormwater management practice component (i.e. Runoff Reduction, Water Quality and O Yes O No Quantity Control practices/techniques)? If No, skip questions 23 and 27-39. 23. Has the post-construction stormwater management practice component of the SWPPP been developed in conformance with the current NYS O Yes O No Stormwater Management Design Manual?

24. The Stormwater Pollution Prevention Plan (S	WPPP) was prepared	by:
O Professional Engineer (P.E.)		
O Soil and Water Conservation District (SWCD)		
O Registered Landscape Architect (R.L.A)		
O Certified Professional in Erosion and Sedimer	it Control (CPESC)	
O Owner/Operator		
()/ O <u>Other</u>		
SWPPP Preparer		
Contact Name (Last, Space, First)		e de la companya de l
Mailing Address		
City	╺━━┓┠┉┉┙┠┉╺╸┞╴╸┞╴╴╸┨╷╶╶┞	
State Zip		tere in the strength with the second seco
Phone Fax	ς	
Email		eren de algebre de la competencia de la

SWPPP Preparer Certification

I hereby certify that the Stormwater Pollution Prevention Plan (SWPPP) for this project has been prepared in accordance with the terms and conditions of the GP-0-15-002. Furthermore, I understand that certifying false, incorrect or inaccurate information is a violation of this permit and the laws of the State of New York and could subject me to criminal, civil and/or administrative proceedings.

First Name	MI
Last Name	
Signature	
	Date

Has a construction sequence schedule for the planned management practices been prepared? 25. OYes Ò No 26. Select all of the erosion and sediment control practices that will be employed on the project site: Temporary Structural Vegetative Measures O Check Dams O Brush Matting O Construction Road Stabilization O Dune Stabilization O Dust Control O Grassed Waterway O Earth Dike O Mulching O Level Spreader O Protecting Vegetation O Perimeter Dike/Swale O Recreation Area Improvement O Pipe Slope Drain O Seeding O Portable Sediment Tank O Sodding O Rock Dam O Straw/Hay Bale Dike O Sediment Basin O Streambank Protection O Sediment Traps O Temporary Swale O Silt Fence O Topsoiling O Stabilized Construction Entrance O Vegetating Waterways O Storm Drain Inlet Protection Permanent Structural O Straw/Hay Bale Dike O Debris Basin O Temporary Access Waterway Crossing O Diversion O Temporary Stormdrain Diversion O Grade Stabilization Structure O Temporary Swale O Land Grading O Turbidity Curtain O Lined Waterway (Rock) O Water bars O Paved Channel (Concrete) O Paved Flume Biotechnical O Retaining Wall O Brush Matting O Riprap Slope Protection O Wattling O Rock Outlet Protection O Streambank Protection Other

Post-construction Stormwater Management Practice (SMP) Requirements Completion of Questions 27-39 is not required Important: if response to Question 22 is No. Identify all site planning practices that were used to prepare the final site 27. plan/layout for the project. O Preservation of Undisturbed Areas O Preservation of Buffers O Reduction of Clearing and Grading O Locating Development in Less Sensitive Areas O Roadway Reduction O Sidewalk Reduction O Driveway Reduction O Cul-de-sac Reduction O Building Footprint Reduction O Parking Reduction 요구 문 음물과

- 27a. Indicate which of the following soil restoration criteria was used to address the requirements in Section 5.1.6("Soil Restoration") of the Design Manual (2010 version).
 - O All disturbed areas will be restored in accordance with the Soil Restoration requirements in Table 5.3 of the Design Manual (see page 5-22).
 - O Compacted areas were considered as impervious cover when calculating the WQv Required, and the compacted areas were assigned a post-construction Hydrologic Soil Group (HSG) designation that is one level less permeable than existing conditions for the hydrology analysis.

28.	Provide the total Water Quality Volume (WQv) required for this project (based on
	final site plan/layout)
*	Total WQv Required
	acre-feet

29. Identify the RR techniques (Area Reduction), RR techniques(Volume Reduction) and Standard SMPs with RRv Capacity in Table 1 (See Page 9) that were used to <u>reduce</u> the Total WQv Required(#28).

Also, provide in Table 1 the total impervious area that contributes runoff to each technique/practice selected. For the Area Reduction Techniques, provide the total contributing area (includes pervious area) and, if applicable, the total impervious area that contributes runoff to the technique/practice.

Note: Redevelopment projects shall use Tables 1 and 2 to identify the SMPs used to treat and/or reduce the WQv required. If runoff reduction techniques will not be used to reduce the required WQv, skip to question 33a after identifying the SMPs.

Table 1 - Runoff Reduction (RR) Techniques and Standard Stormwater Management Practices (SMPs)

	Total Contributing	T	otal Co	ntril	outing					
RR Techniques (Area Reduction)	Area (acres)	Imp	ervious	Area	a (acres)					
O Conservation of Natural Areas (RR-1)		and/or].						
O Sheetflow to Riparian Buffers/Filters Strips (RR-2)		and/or].						
O Tree Planting/Tree Pit (RR-3)	·	and/or								
O Disconnection of Rooftop Runoff (RR-4)		and/or	<u> </u>]•[
RR Techniques (Volume Reduction)		Г] []						
O Vegetated Swale (RR-5)		• • • • • •								
○Rain Garden (RR-6)				┛						
O Stornwater Planter (RR-7)				•						
O Rain Barrel/Cistern (RR-8)										
O Porous Pavement (RR-9)										
O Green Roof (RR-10)		[].						
Standard SMPs with RRv Capacity		f								
O Infiltration Trench (I-1) ·····		ļ								
O Infiltration Basin (I-2) ·····										
O Dry Well (I-3)		1			-					
O Underground Infiltration System (I-4)										
O Bioretention (F-5)		ł								
O Dry Swale (0-1)										
Standard SMPs		_								
O Micropool Extended Detention (P-1)										
O Wet Pond (P-2)										
O Wet Extended Detention (P-3)		· · · ·								
O Multiple Pond System (P-4)										
O Pocket Pond (P-5)										
O Surface Sand Filter (F-1)										
O Underground Sand Filter (F-2)										
O Perimeter Sand Filter (F-3)		{								
O Organic Filter (F-4)		1		*						
O Shallow Wetland (W-1)		Г								
O Extended Detention Wetland (W-2)				•						
O Pond/Wetland System (W-3)				•–						
O Pocket Wetland (W-4)		••••		•						
\bigcirc Wet Swale (0-2)										
6272372694										-
---	--	---	--	--	--	--	---	-------------------	-----------------------------------	------------
Te State of the second state of the	ible 2 -	Alternat (DO, NOT USED, FOR	INCLUDE	PRACTIC		ING				
Alternative SMP					ter en	-	and the second se	Contri ous Are		
O Hydrodynamic			lan da series Ngga ngga ngga	••••••	• • • • • • •	•••		·		
O Wet Vault O Media Filter		•••••	• • • • • • • • •	•••••	• • • • • • • 	•••				
O Other			•		 • • • • • •	••••••				
Provide the name and ma proprietary practice(s)	nufacturer) being us	c of the ed for W	Alternat Qv treat	ive SMP ment.	s (i.e	- 12 8 12 1 2 •				
Name										
Manufacturer Note: Redevelopment proj	 iects whic	h do not	use RR	technig	ues, s	 hall				
use questions 28, WQv required and t	29, 33 an	d 33a to	provide	SMPs u	sed, t	otal	ang sa	estanta ay ta	- 4 - 1999 - 1999 - 1999	
30. Indicate the Tota							olume	Reduct	ion)	and
Standard SMPs with Total RRv provic	_	city ide:	ntiried	in ques	CION Z	9.				
	acre-fee	it								
31. Is the Total RRv p total WQv required If Yes, go to ques If No, go to ques	d (#28). stion 36.		ater tha	n or eq	ual to	the		Ο¥	es . (D No
32. Provide the Minimu [Minimum RRv Requ:					ic)]					
Minimum RRv Requi	acre-fee	at								
32a. Is the Total RRv p Minimum RRv Requi If Yes, go to que <u>Note</u> : Use the <u>specific site</u> 100% of WQv red specific site 100% of the WQv SWPPP. If No, sizing cri processed. SWPPP p criteria.	red (#32)? stion 33: space prov limitation quired (#2 limitation v required teria has preparer m	ided in (s, and ju 8): A <u>de</u> s and ju (#28), m (#28), m (#28), m	question stificat etailed stificat ust also met, so fy desig	#39 to ion for evaluat ion for be inc NOI can	summa not r ion of not r luded	rize educ: the educ: in tl	the Ing	Ο¥	es () №

5736372698	
33. Identify the Standard SMPs in Table 1 and, if applicable, the Table 2 that were used to treat the remaining total WQv (=Total WQv Required in 28 - Total RRv Provided in 19 - To	
Also, provide in Table 1 and 2 the total impervious area the to each practice selected.	hat contributes runoff
Note: Use Tables 1 and 2 to identify the SMPs used on Rede	evelopment projects.
33a. Indicate the Total WQv provided (i.e. WQv treated) by the sidentified in question #33 and Standard SMPs with RRv Capacin question 29. WQv Provided WQv Provided Image: Standard SMPs with RRv capacity, the WQv provided to the standard SMPs with RRv capacity, the WQv provided to the WQv calculated using the contributing drainage area to RRv provided by the practice. (See Table 3.5 in Design Water Standard SMPs with RRv capacity.	city identified by each practice to the practice
34. Provide the sum of the Total RRv provided (#30) and the WQv provided (#33a).	
35. Is the sum of the RRv provided (#30) and the WQv provided (#33a) greater than or equal to the total WQv required (#28) If Yes, go to question 36. If No, sizing oriteria has not been met, so NOI can not be processed. SWPPP preparer must modify design to meet sizing criteria.	
36. Provide the total Channel Protection Storage Volume (CPv) r provided or select waiver (36a), if applicable.	equired and
CPv Required CPv Provid	acre-feet
36a. The need to provide channel protection has been waived becau O Site discharges directly to tidal waters or a fifth order or larger stream. O Reduction of the total CPv is achieved on site through runoff reduction techniques or infiltration s	
37. Provide the Overbank Flood (Qp) and Extreme Flood (Qf) cont select waiver (37a), if applicable.	rol criteria or
Total Overbank Flood Control Criteria (Qp)	
Pre-Development Post-devel	.opment CFS
Total Extreme Flood Control Criteria (Qf)	
	opmont
Pre-Development Post-devel	CFS

Γ

Ĺ.

L

-

10503	372)	693																												
37a.	The		ite or a own	d f st		har h c m a	rge ord ina	s d ler ly:	di) o) sis	rec r 1 s r	tl; arg ev@	y t gez eal	ະວ ເຸຣ	ti tr	da. ear	1 v m.	wat	er	s.			· '	d 1	.,	au	se				
I I	Has post deve (f Y)per	-co elop (es,	nst ed? Id	ru ent	ctio tify	on y t	st he	orr er	nwa uti	ate: .ty	гı	nar	ag	em	ent	t t	bra	lct	ic	e (s	3)	be	en	ı		·	0	Yes	ļ	O NG
Γ						-																								
Ī					T										-					1	Ì	Γ	·							T
	Jse for This	not	re	du	cind	g 1	00	응 (of	WQ	່ມ	rec	rui.	re	d (\$28	3).	(Se	ec	jue	st	ior	ı 3	32a	}		Lcat		
							•																							

.

l

5396372690

0.	Identify	other	DEC	permits,	existing	and	new,	that	are	required	for	this
	project/f	tacili	cy.									

O Air Pollution Control

O Coastal Erosion

O Hazardous Waste

O Long Island Wells

O Mined Land Reclamation

O Solid Waste

O Navigable Waters Protection / Article 15

O Water Quality Certificate

O Dam Safety

O Water Supply

O Freshwater Wetlands/Article 24

O Tidal Wetlands

O Wild, Scenic and Recreational Rivers

O Stream Bed or Bank Protection / Article 15

O Endangered or Threatened Species (Incidental Take Permit)

O Individual SPDES

O SPDES Multi-Sector GP	NY	R	
O Other			

O None

41. Does this project require a US Army Corps of Engineers Wetland Permit? If Yes, Indicate Size of Impact.

42. Is this project subject to the requirements of a regulated, traditional land use control MS4? (If No, skip question 43)

O Yes O No

43. На	s the "M	IS4 SWPPP, Acceptanc	e" form been	signed by the	principal		
ex	ecutive	officer or ranking	elected offi	cial and subm	itted along	OYes	ONO
WJ.	th this	IS4 SWPPP Acceptance officer or ranking NOI?	영화 영상 영화	ave estavas			

44.	If this NOI is being submitted for the purpose of continuing or transferring
	coverage under a general permit for stormwater runoff from construction
	activities, please indicate the former SPDES number assigned. N Y R

u ti av	Owner/Operator Ce have read or been advised of the permit conditions and inderstand that, under the terms of the permit, there may hat this document and the corresponding documents were ware that there are significant penalties for submittin ine and imprisonment for knowing violations. I further	d believe that I understand them ay be reporting requirements. I prepared under my direction or ng false information, including understand that coverage under	hereby certify supervision. I am the possibility of the general permit
W: b(51	Le identified in the acknowledgment that I will red as long as sixty (60) business days as provided for abmitting this NOI, I am acknowledging that the SWPPP b irst element of construction, and agreeing to comply with ermit for which this NOI is being submitted. Print First Name	ceive as a result of submitting in the general permit. I also un has been developed and will be a	this NOI and can nderstand that, by implemented as the
調査を行うし	Print Last Name Owner/Operator Signature		

APPENDIX 4

L

Drainage Basin Maps





APPENDIX 5

1

Ł

L

L

TR-20 Hydro-CAD Calculations – Existing Conditions



Existing Conditions 2021 Type III 24-hr 1 YEAR STORM Rainfall=2.61" Prepared by Pietrzak & Pfau Engineering & Surveying PLLC Page 2 HydroCAD® 7.00 s/n 001436 © 1986-2003 Applied Microcomputer Systems Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points Runoff by SCS TR-20 method, UH=SCS Reach routing by Stor-Ind+Trans method ~ Pond routing by Stor-Ind method Runoff Area=9.200 ac Runoff Depth=0.60" Subcatchment 1S: SUBCATCHMENT 1S Flow Length=976' Tc=16.9 min CN=74 Runoff=4.61 cfs 0.462 af Runoff Area=3.410 ac Runoff Depth=0.60" Subcatchment 2S: SUBCATCHMENT 2S Flow Length=745' Tc=18.5 min CN=74 Runoff=1.65 cfs 0.171 af Runoff Area=6.540 ac Runoff Depth=0.74" Subcatchment 3S: SUBCATCHMENT3S Flow Length=1,192' Tc=13.2 min CN=77 Runoff=4.56 cfs 0.401 af Inflow=4.61 cfs 0.462 af Pond 1P: ON SITE CULVERT PIPE Primary=4.61 cfs 0.462 af Inflow=1.65 cfs 0.171 af Pond 2P: DESIGN POINT 2P Primary=1.65 cfs 0.171 af inflow=4.56 cfs 0.401 af Pond 3P: OFF SITE CULVERT PIPE Primary=4.56 cfs 0.401 af

Total Runoff Area = 19.150 ac Runoff Volume = 1.034 af Average Runoff Depth = 0.65"

		<u>s/n 00143</u>													
			Su	ıbcat	chme	ent 1	IS: S	UBC/	ATCI	HMEN	NT 19	5			
Runoff	Ξ	4.61 cf	s @	12.27	′ hrs, ˈ	Volur	me=		0.46	62 af,	Deptł	n= 0.6	30"		
		R-20 met					pan=	5.00-2	20.00	hrs, d	lt= 0.0)5 hrs			
ype III 2	4-hr 1 Y	EAR STO	ORM	Rainfa	all=2.6	51"									
Area (criptic ad na		& roof:	<u> </u>						• :			
2.6	642 7	74 >75 ⁰	% Ġra	ass co	ver, G		HSG	С							
				air, H Avera						<i>.</i>					
Тс	Length	Slope	-		-	city	Desc	ription	1						
(min)	(feet)	<u>(ft/ft)</u>		sec)	-	city cfs)									
13.1	100	0.0700		0.1				t Flow ds: Lig		derbru	ush	n= 0.4	400 F	2= 3	.15"
[,] 3.8	876	0.0580		3.9					oncer	ntrate	d Flo				
							1 Inna		Ku= 1	16 1 fn	1C				
16.9	976	Total					Unpa	ived	Kv= 1	<u> 6.1 fp</u>	s				
16.9	976	Total	 6	hoat	obmo			ived							
16.9	976	Total	Su	lbcat	chme	•.	S: SI	ived				6			
	976	Total	Su	bcat		ent 1 lydrog	S: SI	ived				5			
16.9	976	Total	Su	bcat		•.	S: SI graph	UBCA	ATCH	HMEN	NT 18	- - - - - - - - - - - - - - - - - - -	STO	RM	- E Runc
	976	Total	Su	bcat		lydrog	S: SI graph	UBCA	ATCH		NT 18 1 Y	EAR	STO		- E Runc
	976	Total	Su	bcat		lydrog	S: SI graph	UBCA	ATCH e III :	HMEN 24-hr Runo	NT 19 1 Y R	EAR ainfa rea=§	=2.).200	61‴ ac	- E Runc
5-	976	Total	Su	bcat		lydrog	S: SI graph	UBCA	ATCH e III :	HMEN 24-hr Runc noff	1 Y R ff Ai Volu	EAR ainfa ea={ me=	=2.).200 0.462	61''' ac 2 af	<u>Runc</u>
5	976	Total	Su	bcat		lydrog	S: SI graph	UBCA	ATCH e III :	HMEN 24-hr Runc noff	1 Y R ff Ai Volu	EAR ainfa ea={ me= Dept	ll=2.).200 0.462 h=0.(61'' ac 2 af 60''	
5	976	Total	Su	bcat		lydrog	S: SI graph	UBCA	ATCH e III :	HMEN 24-hr Runc noff	1 Y R ff Au Volu noff	EAR ainfa ea={ me= Dept Leng	=2.).200 0.462	61'' ac 2 af 60'' 76'	
5	976	Total	Su	bcat		lydrog	S: SI graph	UBCA	ATCH e III :	HMEN 24-hr Runc noff	1 Y R ff Au Volu noff	EAR ainfa ea={ me= Dept Leng	ll=2.).200 0.462 h=0. th=9	61'' ac 2 af 60'' 76' nin	Runc
Flow (cfs)	976	Total	Su	bcat		lydrog	S: SI graph	UBCA	ATCH e III :	HMEN 24-hr Runc noff	1 Y R ff Au Volu noff	EAR ainfa ea={ me= Dept Leng	ll=2. 0.200 0.462 h=0. th=9 6.9 r	61'' ac 2 af 60'' 76' nin	
Flow (cfs)	976	Total	Su	bcat		lydrog	S: SI graph	UBCA	ATCH e III : Ru	HMEN 24-hr Runo Runo F	1 Y R ff Au Volu noff	EAR ainfa ea={ me= Dept Leng	ll=2. 0.200 0.462 h=0. th=9 6.9 r	61'' ac 2 af 60'' 76' nin	
Flow (cfs)	976	Total	Su	bcat		lydrog	S: SI graph	UBCA	ATCH e III :	HMEN 24-hr Runo Runo F	1 Y R ff Au Volu noff	EAR ainfa ea={ me= Dept Leng	ll=2. 0.200 0.462 h=0. th=9 6.9 r	61'' ac 2 af 60'' 76' nin	

Γ

 \int

ſ

l.

Existing Conditions 2021 Prepared by Pietrzak & Pfau Engineering & Surveying PLLC HydroCAD® 7.00 s/n 001436 © 1986-2003 Applied Microcomputer Systems Subcatchment 2S: SUBCATCHMENT 2S 1.65 cfs @ 12.29 hrs, Volume= 0.171 af, Depth= 0.60" Runoff Runoff by SCS TR-20 method, UH=SCS, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 1 YEAR STORM Rainfall=2.61" . ..

Area	(ac)	<u>CN De</u>	scription		
0	.020	98 Pa	ved parking	& roofs	
0	.120	76 W	oods/grass	comb., Fair,	, HSG C
1	.890	73 W	oods, Fair, ł	ISG C	
1	.380	74 >7	5% Grass c	over, Good	, HSG C
3	.410	74 W	eighted Ave	rage	
Tc (min)	Length (feet)		*	Capacity (cfs)	Description
16.4	100	0.040	0.1		Sheet Flow,
2.1	645	5 0.100	D 5.1		Woods: Light underbrush n= 0.400 P2= 3.15" Shallow Concentrated Flow, Unpaved Kv= 16.1 fps
40.5	745	Total			

18.5 745 Total

Subcatchment 2S: SUBCATCHMENT 2S



Type III 24-hr 1 YEAR STORM Rainfall=2.61"

Page 4

Page 5

Prepared by Pietrzak & Pfau Engineering & Surveying PLLC HydroCAD® 7.00 s/n 001436 © 1986-2003 Applied Microcomputer Systems

Subcatchment 3S: SUBCATCHMENT3S

Runoff = 4.56 cfs @ 12.20 hrs, Volume= 0.401 af, Depth= 0.74"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 1 YEAR STORM Rainfall=2.61"

_	Area	(ac) (<u>DN</u> Des	cription	_		
	0.	970	98 Pav	ed parking	& roofs	· · · · · · · · · · · · · · · · · · ·	
	0.:	580	72 Woo	ods/grass o	comb., Goo	d, HSG C	
_	4.	990	<u>74 >75</u>	<u>% Grass c</u>	over, Good,	HSG C	
	6.	540	77 Wei	ghted Aver	age		
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description	
	8.3	62	0.0300	0.1		Sheet Flow,	
	4.0	869	0.0510	3.6		Grass: Dense n= 0.240 P2= 3.15" Shallow Concentrated Flow, Unpaved Kv= 16.1 fps	
	0.9	261	0.0550	4.8		Shallow Concentrated Flow, Paved Kv= 20.3 fps	
	122	1 102	Lotol				

13.2 1,192 Total

Subcatchment 3S: SUBCATCHMENT3S



Type III 24-hr 1 YEAR STORM Rainfall=2.61"

Page 6

Prepared by Pietrzak & Pfau Engineering & Surveying PLLC HydroCAD® 7.00 s/n 001436 © 1986-2003 Applied Microcomputer Systems

Pond 1P: ON SITE CULVERT PIPE

Inflow Area =	9.200 ac, Inflow Depth = 0.60"	for 1 YEAR STORM event
Inflow =	4.61 cfs @ 12.27 hrs, Volume=	0.462 af
Primary =	4.61 cfs @ 12.27 hrs, Volume=	0.462 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs



Pond 1P: ON SITE CULVERT PIPE

Existing Conditions 2021Type III 24-hr 1 YEAR STORM Rainfall=2.61"Prepared by Pietrzak & Pfau Engineering & Surveying PLLCPage 7HydroCAD® 7.00 s/n 001436 © 1986-2003 Applied Microcomputer SystemsPage 7

Pond 2P: DESIGN POINT 2P

Inflow Area =	3.410 ac, Inflow Depth = 0.60 "	for 1 YEAR STORM event
Inflow =	1.65 cfs @ 12.29 hrs, Volume=	0.171 af
Primary =	1.65 cfs @ 12.29 hrs, Volume=	0.171 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs



Pond 2P: DESIGN POINT 2P

Type III 24-hr 1 YEAR STORM Rainfall=2.61"

Page 8

Prepared by Pietrzak & Pfau Engineering & Surveying PLLC HydroCAD® 7.00 s/n 001436 © 1986-2003 Applied Microcomputer Systems

Pond 3P: OFF SITE CULVERT PIPE

Inflow Area =	6.540 ac, Inflow Depth = 0.74 "	for 1 YEAR STORM event
Inflow =	4.56 cfs @ 12.20 hrs, Volume=	0.401 af
Primary =	4.56 cfs @ 12.20 hrs, Volume=	0.401 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Existing Conditions 2021



Pond 3P: OFF SITE CULVERT PIPE

Type III 24-hr 10 YEAR STORM Rainfall=4.66" **Existing Conditions 2021** Prepared by Pietrzak & Pfau Engineering & Surveying PLLC Page 9 HydroCAD® 7.00 s/n 001436 © 1986-2003 Applied Microcomputer Systems Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points Runoff by SCS TR-20 method, UH=SCS Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method Subcatchment 1S: SUBCATCHMENT 1S Runoff Area=9.200 ac Runoff Depth=1.93" Flow Length=976' Tc=16.9 min CN=74 Runoff=16.06 cfs 1.480 af Subcatchment 2S: SUBCATCHMENT 2S Runoff Area=3.410 ac Runoff Depth=1.93" Flow Length=745' Tc=18.5 min CN=74 Runoff=5.75 cfs 0.548 af . Subcatchment 3S: SUBCATCHMENT3S Runoff Area=6.540 ac Runoff Depth=2.17" Flow Length=1,192' Tc=13.2 min CN=77 Runoff=14.08 cfs 1.182 af Pond 1P: ON SITE CULVERT PIPE inflow=16.06 cfs 1.480 af Primary=16.06 cfs 1.480 af Pond 2P: DESIGN POINT 2P Inflow=5.75 cfs 0.548 af Primary=5.75 cfs 0.548 af Pond 3P: OFF SITE CULVERT PIPE Inflow=14.08 cfs 1.182 af Primary=14.08 cfs 1.182 af Total Runoff Area = 19.150 ac Runoff Volume = 3.210 af Average Runoff Depth = 2.01"

16.9

976

Total

Type III 24-hr 10 YEAR STORM Rainfall=4.66"

Prepared by Pietrzak & Pfau Engineering & Surveying PLLCPage 10HydroCAD® 7.00 s/n 001436 © 1986-2003 Applied Microcomputer SystemsPage 10

Subcatchment 1S: SUBCATCHMENT 1S

Runoff	=	16.06 cfs	s@ 12.24	4 hrs, Volu	me= 1.480 af, Depth= 1.93"
				CS, Time S nfall=4.66"	Span= 5.00-20.00 hrs, dt= 0.05 hrs
Area	(ac) C	N Dese	cription		
0.	188 9	98 Pave	ed parking	& roofs	
2.	642	74 >759	% Grass c	over, Good	, HSG C
6.	370 7	7 <u>3 Woo</u>	ds, Fair, ⊦	ISG C	
9.	200 7	74 Weię	ghted Aver	age	
Tc (min)_	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
13.1	100	0.0700	0.1		Sheet Flow,
3.8	876	0.0580	3.9		Woods: Light underbrush n= 0.400 P2= 3.15" Shallow Concentrated Flow,

Subcatchment 1S: SUBCATCHMENT 1S

Unpaved Kv= 16.1 fps



Prepared by Pietrzak & Pfau Engineering & Surveying PLLC HydroCAD® 7.00 s/n 001436 © 1986-2003 Applied Microcomputer Systems

Page 11

	Subcatchment 2S: SUBCATCHMENT 2S								
Runoff	=	5.75 cf	s @ 12.2	6 hrs, Volu	ume= 0.548 af, Depth= 1.93"				
Runoff by SCS TR-20 method, UH=SCS, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 10 YEAR STORM Rainfall=4.66"									
Area	(ac) C	N Des	cription		:				
. 0.	.020	98 Pav	ed parking	& roofs					
0.	.120			comb., Fair	HSG C				
1.	890		ods, Fair, H		· · · · · ·				
1.				over, Good	I. HSG C				
3.	410 .		ghted Ave						
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description				
16.4	100	0.0400	0.1		Sheet Flow,				
2.1	645	0.1000	5.1		Woods: Light underbrush n= 0.400 P2= 3.15" Shallow Concentrated Flow, <u>Unpaved Kv= 16.1 fps</u>				
18.5	745	Total							

Subcatchment 2S: SUBCATCHMENT 2S



Type III 24-hr 10 YEAR STORM Rainfall=4.66"

Page 12

Prepared by Pietrzak & Pfau Engineering & Surveying PLLC HydroCAD® 7.00 s/n 001436 © 1986-2003 Applied Microcomputer Systems

Subcatchment 3S: SUBCATCHMENT3S

Runoff = 14.08 cfs @ 12.19 hrs, Volume= 1.182 af, Depth= 2.17"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 10 YEAR STORM Rainfall=4.66"

Area	(ac) <u>C</u>	N Desc	cription						
0.	0.970 98 Paved parking & roofs								
0.	0.580 72 Woods/grass comb., Good, HSG C								
4.	4.990 74 >75% Grass cover, Good, HSG C								
6.	540 7	77 Weig	ghted Aver	age					
Тс	Length	Slope	Velocity	Capacity	Description				
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)	<u></u>				
8.3	62	0.0300	0.1		Sheet Flow,				
					Grass: Dense n= 0.240 P2= 3.15"				
4.0	869	0.0510	3.6		Shallow Concentrated Flow,				
					Unpaved Kv= 16.1 fps				
0.9	261	0.0550	4.8		Shallow Concentrated Flow,				
					Paved Kv= 20.3 fps				
13.2	1,192	Total							

Subcatchment 3S: SUBCATCHMENT3S



Prepared by Pietrzak & Pfau Engineering & Surveying PLLC HydroCAD® 7.00 s/n 001436 © 1986-2003 Applied Microcomputer Systems

Pond 1P: ON SITE CULVERT PIPE

Inflow Area	a =	9.200 ac, Inflow Depth = 1.93"	for 10 YEAR STORM event
Inflow	=	16.06 cfs @ 12.24 hrs, Volume=	1.480 af
Primary	Ξ	16.06 cfs @ 12.24 hrs, Volume=	1.480 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs



Pond 1P: ON SITE CULVERT PIPE

Existing Conditions 2021Type III 24-hr 10 YEAR STORM Rainfall=4.66"Prepared by Pietrzak & Pfau Engineering & Surveying PLLCPage 14HydroCAD® 7.00 s/n 001436 © 1986-2003 Applied Microcomputer SystemsPage 14

Pond 2P: DESIGN POINT 2P

Inflow Are	a =	3.410 ac, inflow Depth = 1.93"	for 10 YEAR STORM event
Inflow	=	5.75 cfs @ 12.26 hrs, Volume=	0.548 af
Primary	=	5.75 cfs @ 12.26 hrs, Volume=	0.548 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs



Pond 2P: DESIGN POINT 2P

Prepared by Pietrzak & Pfau Engineering & Surveying PLLC HydroCAD® 7.00 s/n 001436 © 1986-2003 Applied Microcomputer Systems

Pond 3P: OFF SITE CULVERT PIPE

Inflow Area	a =	6.540 ac, Inflow Depth = 2.17"	for 10 YEAR STORM event
Inflow	=	14.08 cfs @ 12.19 hrs, Volume=	1.182 af
Primary	=	14.08 cfs @ 12.19 hrs, Volume=	1.182 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs



Pond 3P: OFF SITE CULVERT PIPE

Type III 24-hr 25 YEAR STORM Rainfall=5.84"

Runoff Area=3,410 ac Runoff Depth=2.83"

Inflow=23.59 cfs 2.168 af

Inflow=8.46 cfs 0.803 af Primary=8.46 cfs 0.803 af

Inflow=20.14 cfs 1.695 af

Primary=20.14 cfs 1.695 af

Primary=23.59 cfs 2.168 af

Page 16

Prepared by Pietrzak & Pfau Engineering & Surveying PLLC HydroCAD® 7.00 s/n 001436 © 1986-2003 Applied Microcomputer Systems

> Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points Runoff by SCS TR-20 method, UH=SCS Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Runoff Area=9.200 ac Runoff Depth=2.83" Subcatchment 1S: SUBCATCHMENT 1S Flow Length=976' Tc=16.9 min CN=74 Runoff=23.59 cfs 2.168 af

Subcatchment 2S: SUBCATCHMENT 2S Flow Length=745' Tc=18.5 min CN=74 Runoff=8.46 cfs 0.803 af

Subcatchment 3S: SUBCATCHMENT3S

Runoff Area=6.540 ac Runoff Depth=3.11" Flow Length=1,192' Tc=13.2 min CN=77 Runoff=20.14 cfs 1.695 af

Pond 1P: ON SITE CULVERT PIPE

Pond 2P: DESIGN POINT 2P

Pond 3P: OFF SITE CULVERT PIPE

Total Runoff Area = 19.150 ac Runoff Volume = 4.666 af Average Runoff Depth = 2.92"

Existing Conditions 2021 Prepared by Pietrzak & Pfau Engineering & Surveyi HydroCAD® 7.00 s/n 001436 © 1986-2003 Applied Microco									
Subcatchment 1S: SUBCATCHMENT 1S									
Runoff = 23.59 cfs @ 12.24 hrs, Volume=	2.168 af, Depth= 2.83"								
Runoff by SCS TR-20 method, UH=SCS, Time Span= 5. Type III 24-hr 25 YEAR STORM Rainfall=5.84"	00-20.00 hrs, dt= 0.05 hrs								
Area (ac) CN Description									
0.188 98 Paved parking & roofs									
2.642 74 >75% Grass cover, Good, HSG C									
6.370 73 Woods, Fair, HSG C	· · · · · · · · · · · · · · · · · · ·								
9.200 74 Weighted Average									

Γ

Ì.

L

 $\left[\right]$

L

	Тс	Length	Slope	Velocity	Capacity	Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	13.1	100	0.0700	0.1		Sheet Flow,
						Woods: Light underbrush n= 0.400 P2= 3.15"
	3.8	876	0.0580	3.9		Shallow Concentrated Flow,
_						Unpaved Kv= 16.1 fps
	16.9	976	Total			

Subcatchment 1S: SUBCATCHMENT 1S



Type III 24-hr 25 YEAR STORM Rainfall=5.84"

Prepared by Pietrzak & Pfau Engineering & Surveying PLLCPage 18HydroCAD® 7.00 s/n 001436 © 1986-2003 Applied Microcomputer SystemsPage 18

Subcatchment 2S: SUBCATCHMENT 2S

Runoff = 8.46 cfs @ 12.26 hrs, Volume= 0.803 af, Depth= 2.83"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 25 YEAR STORM Rainfall=5.84"

_	Area	(ac) C	N Des	cription		
	0.	020 9		ed parking		
	0.	, HSG C				
1.890 73 Woods, Fair, HSG C						
					over, Good	, HSG C
3.410 74 Weighted Average						
_	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
	16.4	100	0.0400	0.1		Sheet Flow,
	2.1	645	0.1000	5.1		Woods: Light underbrush n= 0.400 P2= 3.15" Shallow Concentrated Flow, Unpaved Kv= 16.1 fps
	40 5	745	T ()			

18.5 745 Total

Subcatchment 2S: SUBCATCHMENT 2S



Prepared by Pietrzak & Pfau Engineering & Surveying PLLC HydroCAD® 7.00 s/n 001436 © 1986-2003 Applied Microcomputer Systems

Page 19

Subcatchment 3S: SUBCATCHMENT3S										
Runoff	=	20.14 cfs	s@ 12.1	9 hrs, Volu	ime= 1.695 af, Depth= 3.11"					
	Runoff by SCS TR-20 method, UH=SCS, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 25 YEAR STORM Rainfall=5.84"									
Area	(ac) C	N Des	cription							
0. 0.	0.970 98 Paved parking & roofs 0.580 72 Woods/grass comb., Good, HSG C 4.990 74 >75% Grass cover, Good, HSG C									
6.	540 7	77 Weig	ghted Aver	age						
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description					
8.3	62	0.0300	0.1		Sheet Flow,					
4.0	869	0.0510	3.6		Grass: Dense n= 0.240 P2= 3.15" Shallow Concentrated Flow, Unpaved Kv= 16.1 fps					
0.9	261	0.0550	4.8		Shallow Concentrated Flow, Paved Kv= 20.3 fps					
13.2	1,192	Total			· · · · · · · · · · · · · · · · · · ·					
			~ 1							

Subcatchment 3S: SUBCATCHMENT3S



Type III 24-hr 25 YEAR STORM Rainfall=5.84" Surveying PLLC Page 20

Prepared by Pietrzak & Pfau Engineering & Surveying PLLC HydroCAD® 7.00 s/n 001436 © 1986-2003 Applied Microcomputer Systems

Pond 1P: ON SITE CULVERT PIPE

Inflow Area =	9.200 ac, Inflow Depth = 2.83"	for 25 YEAR STORM event
Inflow =	23.59 cfs @ 12.24 hrs, Volume=	2.168 af
Primary =	23.59 cfs @ 12.24 hrs, Volume=	2.168 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs



Pond 1P: ON SITE CULVERT PIPE

Type III 24-hr 25 YEAR STORM Rainfall=5.84" /ing PLLC Page 21

Prepared by Pietrzak & Pfau Engineering & Surveying PLLC HydroCAD® 7.00 s/n 001436 © 1986-2003 Applied Microcomputer Systems

Pond 2P: DESIGN POINT 2P

Inflow Area =	3.410 ac, Inflow Depth = 2.83"	for 25 YEAR STORM event
Inflow =	8.46 cfs @ 12.26 hrs, Volume=	0.803 af
Primary =	8.46 cfs @ 12.26 hrs, Volume=	0.803 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs



Pond 2P: DESIGN POINT 2P

Type III 24-hr 25 YEAR STORM Rainfall=5.84" Prepared by Pietrzak & Pfau Engineering & Surveying PLLC HydroCAD® 7.00 s/n 001436 © 1986-2003 Applied Microcomputer Systems

Pond 3P: OFF SITE CULVERT PIPE

Inflow Are	ea =	6.540 ac, Inflow Depth = 3.11"	for 25 YEAR STORM event
Inflow	=	20.14 cfs @ 12.19 hrs, Volume=	1.695 af
Primary	=	20.14 cfs @ 12.19 hrs, Volume=	1.695 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs



Pond 3P: OFF SITE CULVERT PIPE

Page 22

Existing Conditions 2021

Existing Conditions 2021 Prepared by Pietrzak & Pfau Engine HydroCAD® 7.00 s/n 001436 © 1986-200	ering & Surveying		0 YEAR	STO	RM Rainf	all=8.22″ Page 23
•	5.00-20.00 hrs, dt= f by SCS TR-20 me d+Trans method -	thod, UH=SC	5	Ind me	thod	
Subcatchment 1S: SUBCATCHMENT	IS Flow Length=976'				Runoff De ff=39.60 cfs	•
Subcatchment 2S: SUBCATCHMENT 2	2 S Flow Length=745'				Runoff De ff=14.20 cfs	
Subcatchment 3S: SUBCATCHMENT3	S Flow Length=1,192'				Runoff De ff=32.78 cfs	-
Pond 1P: ON SITE CULVERT PIPE					v=39.60 cfs y=39.60 cfs	
Pond 2P: DESIGN POINT 2P					v=14.20 cfs y=14.20 cfs	
Pond 3P: OFF SITE CULVERT PIPE					v=32.78 cfs y=32.78 cfs	
Total Runoff Area = 19.	150 ac Runoff Vol	ume = 7.824 a	af Avera	age Ru	inoff Dept	h = 4.90"

1

Γ

 $\left[\right]$

-

Type III 24-hr 100 YEAR STORM Rainfall=8.22"

Prepared by Pietrzak & Pfau Engineering & Surveying PLLCPage 24HydroCAD® 7.00 s/n 001436 © 1986-2003 Applied Microcomputer SystemsPage 24

Subcatchment 1S: SUBCATCHMENT 1S

Runoff		39.60 cfs	s@ 12.2	3 hrs, Volu	ıme= 3.667 af, Depth= 4.78"
				CS, Time S ainfall=8.22	Span= 5.00-20.00 hrs, dt= 0.05 hrs "
Area	(ac) C	N Dese	cription		
2	.642 7	74 >75	ed parking % Grass c ids, Fair, H	over, Good	, HSG C
9	.200 7		ghted Avei		
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
13.1	100	0.0700	0.1		Sheet Flow,
3.8	876	0.0580	3.9		Woods: Light underbrush n= 0.400 P2= 3.15" Shallow Concentrated Flow, Unpaved Kv= 16.1 fps
16.9	976	Total			
			Subca	tchment	1S' SUBCATCHMENT 1S

Subcatchment 1S: SUBCATCHMENT 1S



		3/11 00 140	0 0 1900-	2003 App	lied Mic	rocomp	uter S	ystem	S				<u></u>
			Subca	tchmen	nt 2S: \$	SUBC	ATCI	HME	NT 28	S			
Runoff	=	14.20 cf	s@ 12.2	5 hrs, Vo	olume=		1.35	9 af,	Depti	בי= 4.1	78"		
			hod, UH=S TORM Ra			= 5.00-,	20.00	hrs, c	dt= 0.0)5 hrs			
Area	(ac) C	N Des	cription										
			ed parking ods/grass o		air HSC	20							
1.8	890 7	73 Woo	ods, Fair, F	ISG C									
			<u>% Grass c</u> ghted Avei		oa, HSC	5 C							
Tc (min)	Length (feet)	Slope (ft/ft)	- Velocity (ft/sec)	Capacil (cfs		criptior	י ו						
16.4	100	0.0400	0.1		She	et Flov			1				
2.1	645	0.1000	5.1		Sha	ods: Lig I llow C oaved	oncer	ntrate	d Flov		100 H	-2= 3.1	15"
18.5	745	Totai			011	<u>aroa</u>		<u></u> ,	<u></u>				
			Subca	tchmen	t 2S: \$	SUBC	ATC	IMEI	NT 25	5			
				Нус	Irograph								
15													🖪 Runoff
14-1/				L	14.20 cfs	Туре	III 24	1-hr '	100 Y	EAR	STO	RM	
13	, 										all=8.		
12-1 11-									off A				
10-							R		f Volu Inoff		t .		
ifs)	,								Flow				
Flow (cfs)					- 84						18.5 I		
				+								=74	
6+ 5+								: 					
4						1					••••		
3-	-					2							
2						E AND	181 March						
					1		1318181	101.000	<u> </u>				

[

Γ

.

Type III 24-hr 100 YEAR STORM Rainfall=8.22" veving PLLC Page 26

Prepared by Pietrzak & Pfau Engineering & Surveying PLLC HydroCAD® 7.00 s/n 001436 © 1986-2003 Applied Microcomputer Systems

Subcatchment 3S: SUBCATCHMENT3S

Runoff = 32.78 cfs @ 12.18 hrs, Volume= 2.798 af, Depth= 5.13"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 100 YEAR STORM Rainfall=8.22"

Area	(ac) C	N Dese	cription					
0.	0.970 98 Paved parking & roofs							
0.	580			comb., Goo				
4.	990	74 >759	% Grass co	over, Good,	HSG C			
6.	540	77 Weię	ghted Aver	age				
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description			
8.3	62	0.0300	0.1		Sheet Flow,			
4.0	869	0.0510	3.6		Grass: Dense n= 0.240 P2= 3.15" Shallow Concentrated Flow,			
0.9	261	0.0550	4.8		Unpaved Kv= 16.1 fps Shallow Concentrated Flow, Paved Kv= 20.3 fps			
13.2	1,192	Total						

Subcatchment 3S: SUBCATCHMENT3S



Prepared by Pietrzak & Pfau Engineering & Surveying PLLC HydroCAD® 7.00 s/n 001436 © 1986-2003 Applied Microcomputer Systems

Pond 1P: ON SITE CULVERT PIPE

Inflow Area =	9.200 ac, Inflow Depth = 4.78"	for 100 YEAR STORM event
Inflow =	39.60 cfs @ 12.23 hrs, Volume=	3.667 af
Primary =	39.60 cfs @ 12.23 hrs, Volume=	3.667 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs



Pond 1P: ON SITE CULVERT PIPE
Existing Conditions 2021

Type III 24-hr 100 YEAR STORM Rainfall=8.22" urveying PLLC Page 28

Prepared by Pietrzak & Pfau Engineering & Surveying PLLC HydroCAD® 7.00 s/n 001436 © 1986-2003 Applied Microcomputer Systems

Pond 2P: DESIGN POINT 2P

Inflow Area =	3.410 ac, Inflow Depth = 4.78"	for 100 YEAR STORM event
Inflow ≔	14.20 cfs @ 12.25 hrs, Volume=	1.359 af
Primary =	14.20 cfs @ 12.25 hrs, Volume=	1.359 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs



Pond 2P: DESIGN POINT 2P

Existing Conditions 2021Type III 24-hr 100 YEAR STORM Rainfall=8.22"Prepared by Pietrzak & Pfau Engineering & Surveying PLLCPage 29HydroCAD® 7.00 s/n 001436 © 1986-2003 Applied Microcomputer SystemsPage 29

Pond 3P: OFF SITE CULVERT PIPE

Inflow Area =	6.540 ac, Inflow Depth = 5.13"	for 100 YEAR STORM event
Inflow =	32.78 cfs @ 12.18 hrs, Volume=	2.798 af
Primary . =	32.78 cfs @ 12.18 hrs, Volume=	2.798 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs



Pond 3P: OFF SITE CULVERT PIPE

Prepared by Pietrzak & Pfau Engineering & Surveying PLLC Page 30 HydroCAD® 7.00 s/n 001436 © 1986-2003 Applied Microcomputer Systems Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points Runoff by SCS TR-20 method, UH=SCS Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method Subcatchment 1S: SUBCATCHMENT 1S Runoff Area=9.200 ac Runoff Depth=0.09" Flow Length=976' Tc=16.9 min CN=74 Runoff=0.39 cfs 0.072 af Subcatchment 2S: SUBCATCHMENT 2S Runoff Area=3.410 ac Runoff Depth=0.09" Flow Length=745' Tc=18.5 min CN=74 Runoff=0.14 cfs 0.027 af Runoff Area=6.540 ac Runoff Depth=0.14" Subcatchment 3S: SUBCATCHMENT3S Flow Length=1,192' Tc=13.2 min CN=77 Runoff=0.56 cfs 0.078 af

Pond 1P: ON SITE CULVERT PIPE

Pond 2P: DESIGN POINT 2P

Pond 3P: OFF SITE CULVERT PIPE

Total Runoff Area = 19.150 ac Runoff Volume = 0.177 af Average Runoff Depth = 0.11"

Existing Conditions 2021

Type III 24-hr WQ STORM Rainfall=1.39"

Inflow=0.39 cfs 0.072 af Primary=0.39 cfs 0.072 af

Inflow=0.14 cfs 0.027 af Primary=0.14 cfs 0.027 af

Inflow=0.56 cfs 0.078 af Primary=0.56 cfs 0.078 af

Existing	Conditions	2021

Prepared by Pietrzak & Pfau Engineering & Surveying PLLC HydroCAD® 7.00 s/n 001436 © 1986-2003 Applied Microcomputer Systems

Subcatchment 1S: SUBCATCHMENT 1S

Runoff	0.072 af, Depth= 0.09"	
Runon	J.072 at.	. Depth= 0.09"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr WQ STORM Rainfall=1.39"

_	Area	(ac)	CN	Desc	ription		
		188			d parking		
		642				over, Good,	HSG C
		370			<u>ds, Fair, H</u>		<u> </u>
	9.	200	74	vveig	hted Aver	age	
	Тс	Length		ре	Velocity	Capacity	Description
_	<u>(min)</u>	(feet	<u>) (f</u>	<u>t/ft)</u>	(ft/sec)	(cfs)	
	13.1	100	0.07	700	0.1		Sheet Flow,
	3.8	876	6 0.08	580	3.9		Woods: Light underbrush n= 0.400 P2= 3.15" Shallow Concentrated Flow, Unpaved Kv= 16.1 fps
	16.9	976	6 Tota	al			····

Subcatchment 1S: SUBCATCHMENT 1S



Existing Conditions 2021

Type III 24-hr WQ STORM Rainfall=1.39"

Page 32

Prepared by Pietrzak & Pfau Engineering & Surveying PLLC HydroCAD® 7.00 s/n 001436 © 1986-2003 Applied Microcomputer Systems

Subcatchment 2S: SUBCATCHMENT 2S

Runoff	T	0.14 cfs	s@ 12.52	2 hrs, Volu	me= 0.027 af, Depth= 0.09"
			nod, UH=S Rainfall=		Span= 5.00-20.00 hrs, dt= 0.05 hrs
Area	(ac) C	N Desc	ription		
0.	020 9	8 Pave	d parking	& roofs	
0.	120 7			omb., Fair,	, HSG C
1.	890 7	′3 Woo	ds, Fair, F	ISG C	
1.	380 7	<u>′4 >75%</u>	6 Grass co	over, Good	, HSG C
3.	410 7	4 Weig	phted Aver	age	
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
16.4	100	0.0400	0.1	· · · · · · · · · · · · · · · · · · ·	Sheet Flow,
2 .1	645	0.1000	5.1		Woods: Light underbrush n= 0.400 P2= 3.15" Shallow Concentrated Flow, Unpaved Kv= 16.1 fps
18.5	745	Total			

Subcatchment 2S: SUBCATCHMENT 2S



	d by Pi		[⊃] fau Engi	neering & 2003 Applie		ing PLLC						ainfall=1.39 Page 33
		· · .	Subca	tchment	3S: SU	JBCATCH	IMEN	NT3S				
Runoff	=	0.56 cfs	s @ 12.3	2 hrs, Volu	ime=	0.07	8 af, 1	Depth	= 0.1	4"		
			nod, UH=S Rainfall=	CS, Time \$ 1.39"	Span= 5	i.00-20.00	hrs, d	t= 0.05	5 hrs			
Area	(ac) (CN Desc	cription									
			ed parking			~						
				omb., Goo over, Good								
			ghted Aver		, 1100 0					<u></u>		
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Descri	ption						
8.3	62	0.0300	0.1		Sheet				- 0	4 17 18		
4.0	869	0.0510	3.6		Shallo	: Dense n w Concen	trated	d Flow		15		
						red Kv≓1						
0.9	261	0.0550	4.8			w Concen		d Flow	Ι,			
			4.8			W Concen Kv= 20.3		d Flow	Ι,			
0.9	1,192				Paved	Kv= 20.3	3 fps		Ι,			
				tchment Hvdro	Paved 3S: SU	Kv= 20.3	3 fps		<i>ו</i> ,			
					Paved	Kv= 20.3	3 fps		<i>'</i> ,			
	1,192			Hydro	Paved 3S: SU	Kv= 20.3	3 fps		<i>'</i> ,			🖸 Runoff
13.2	1,192			Hydro	Paved 3S: SU graph	Kv= 20.3	imen	IT3S	-	TOF	RM	☐ Runoff
13.2 0.6	1,192			Hydro	Paved 3S: SU graph	Kv= 20.3	3 fps 1MEN 24-1	IT3S	QS			☐ Runoff
13.2 0.6	1,192			Hydro	Paved 3S: SU graph	Kv= 20.3 JBCATC⊦ Type III	3 fps IMEN 24-1	NT3S hr W	Q S Ifall	=1.3	9"	Runoff
13.2 0.6 0.55	1,192			Hydro	Paved 3S: SU graph	Kv= 20.3 JBCATC⊦ Type III	3 fps IMEN 24-1	NT3S hr W Rain Area	Q S Ifall	=1.3 540	9" ac	Runoff
13.2 0.6 0.55 0.55 0.45	1,192			Hydro	Paved 3S: SU graph	Kv= 20.3 JBCATCH Type III Rur Runof	3 fps 1MEN 24-1 1 off f Vo	NT3S Ar W Rain Area Iume	Q S Ifall =6.	=1.3 540 078	9" ac af	Runoff
13.2 0.6 0.55 0.55 0.45	1,192			Hydro	Paved 3S: SU graph	Kv= 20.3 JBCATCH Type III Runof Runof	3 fps IMEN 24-1 off f Vo unot	NT3S hr W Rain Area lume ff De	Q S Ifall =6. =0. pth	=1.3 540 078 =0.1	9" ac af 4"	Runoff
13.2 0.6 0.55 0.45 0.45 0.45	1,192			Hydro	Paved 3S: SU graph	Kv= 20.3 JBCATCH Type III Runof Runof	3 fps IMEN 24-1 off f Vo unot	NT3S hr W Rain Area lum f De Leng	Q S Ifall =6. e=0. pth	=1.3 540 078 =0.1	9" ac af 4" 92'	Runoff
13.2 0.6 0.55 0.45 0.45 0.45 0.45 0.45 0.45 0.45	1,192			Hydro	Paved 3S: SU graph	Kv= 20.3 JBCATCH Type III Runof Runof	3 fps IMEN 24-1 off f Vo unot ow	NT3S Ar W Rain Area Iume f De Lenc Tc	Q S Ifall =6. =0. pth gth= =13	=1.3 540 078 =0.1 :1,1 :2 m	9" ac af 4" 92' nin	Runoff
13.2 0.6 0.55 0.45 0.45 0.45 0.45 0.45 0.45 0.35	1,192			Hydro	Paved 3S: SU graph	Kv= 20.3 JBCATCH Type III Runof Runof	3 fps IMEN 24-1 off f Vo unot ow	NT3S Ar W Rain Area Iume f De Lenc Tc	Q S Ifall =6. =0. pth gth= =13	=1.3 540 078 =0.1	9" ac af 4" 92' nin	Runoff
13.2 0.6 0.55 0.45 0.45 0.45 0.45 0.45 0.45 0.45	1,192			Hydro	Paved 3S: SU graph	Kv= 20.3 JBCATCH Type III Runof Runof	3 fps IMEN 24-1 off f Vo unof ow	NT3S Ar W Rain Area Iume f De Lenc Tc	Q S Ifall =6. =0. pth gth= =13	=1.3 540 078 =0.1 :1,1 :2 m	9" ac af 4" 92' nin	Runoff
13.2 0.6 0.55 0.55 0.45 0.45 0.45 0.45 0.45 0.35 0.25 0.25	1,192			Hydro	Paved 3S: SU graph	Kv= 20.3 JBCATCH Type III Runof Runof	3 fps IMEN 24-1 off f Vo unof ow	NT3S Ar W Rain Area Iume f De Lenc Tc	Q S Ifall =6. =0. pth gth= =13	=1.3 540 078 =0.1 :1,1 :2 m	9" ac af 4" 92' nin	Runoff

[]

L

L

نـا

Existing Conditions 2021

Type III 24-hr WQ STORM Rainfall=1.39"

 Prepared by Pietrzak & Pfau Engineering & Surveying PLLC
 Page 34

 HydroCAD® 7.00 s/n 001436 © 1986-2003 Applied Microcomputer Systems
 Page 34

Pond 1P: ON SITE CULVERT PIPE

Inflow Area	=	9.200 ac, Inflow Depth = 0.09" for WQ STORM ev	/ent
Inflow =	=	0.39 cfs @ 12.49 hrs, Volume= 0.072 af	
Primary =	=	0.39 cfs @ 12.49 hrs, Volume= 0.072 af, Att	ten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs



Pond 1P: ON SITE CULVERT PIPE

Existing Conditions 2021

Type III 24-hr WQ STORM Rainfall=1.39" LLC Page 35

Prepared by Pietrzak & Pfau Engineering & Surveying PLLC HydroCAD® 7.00 s/n 001436 © 1986-2003 Applied Microcomputer Systems

Pond 2P: DESIGN POINT 2P

Inflow Area	a =	3.410 ac, Inflow Depth = 0.09" fo	or WQ STORM event
Inflow	-	0.14 cfs @ 12.52 hrs, Volume=	0.027 af
Primary	=	0.14 cfs @ 12.52 hrs, Volume=	0.027 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs



Pond 2P: DESIGN POINT 2P

Prepared by Pietrzak & Pfau Engineering & Surveying PLLC HydroCAD® 7.00 s/n 001436 © 1986-2003 Applied Microcomputer Systems

Pond 3P: OFF SITE CULVERT PIPE

Inflow Are	a =	6.540 ac, Inflow Depth = 0.14" for WQ STORM event	
Inflow	=	0.56 cfs @ 12.32 hrs, Volume= 0.078 af	
Primary	=	0.56 cfs @ 12.32 hrs, Volume= 0.078 af, Atten= 0%, Lag= 0.0 mir	n

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs



Pond 3P: OFF SITE CULVERT PIPE

Existing Conditions 2021

*Type III 24-hr W*Q STORM Rainfall=1.39" LLC Page 36

APPENDIX 6

[_

Į_

L

L

TR-20 Hydro-CAD Calculations – Proposed Conditions



HydroCAD® 7.00 s/n 001436 © 1986-2003 Applied Microcomputer Systems Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points Runoff by SCS TR-20 method, UH=SCS Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method Runoff Area=7.060 ac Runoff Depth=0.60" Subcatchment 1S: SUBCATCHMENT 1S Flow Length=408' Tc=11.5 min CN=74 Runoff=4.04 cfs 0.355 af Runoff Area=2.970 ac Runoff Depth=0.64" Subcatchment 2S: SUBCATCHMENT 2S Flow Length=745' Tc=18.5 min CN=75 Runoff=1.56 cfs 0.159 af Runoff Area=5.210 ac Runoff Depth=0.78" Subcatchment 3S: SUBCATCHMENT 3S Flow Length=951' Tc=11.3 min CN=78 Runoff=4.14 cfs 0.340 af Runoff Area=3.910 ac Runoff Depth=0.94" Subcatchment 4S: SUBCATCHMENT 4S Flow Length=832' Tc=16.5 min CN=81 Runoff=3.30 cfs 0.305 af Peak Depth=0.03' Max Vel=1.3 fps Inflow=0.14 cfs 0.082 af Reach 5R: (new Reach) n=0.030 L=616.0' S=0.0610 '/' Capacity=104.69 cfs Outflow=0.14 cfs 0.079 af Inflow=4.04 cfs 0.355 af Pond 1P: ON SITE CULVERT PIPE Primary=4.04 cfs 0.355 af Inflow=1.56 cfs 0.159 af Pond 2P: DESIGN POINT 2P Primarv=1.56 cfs 0.159 af Inflow=4.14 cfs 0.419 af Pond 3P: OFF SITE CULVERT PIPE Primary=4.14 cfs 0.419 af Peak Elev=418.42' Storage=9,916 cf Inflow=3.30 cfs 0.305 af Pond 4P: Type F-5 Bioretention Pond Outflow=0.14 cfs 0.082 af

Proposed Conditions 2021

Prepared by Pietrzak & Pfau Engineering & Surveying PLLC

Type III 24-hr 1 YEAR STORM Rainfall=2.61"

Page 2

Total Runoff Area = 19.150 ac Runoff Volume = 1.159 af Average Runoff Depth = 0.73"

Page 3

Prepared by Pietrzak & Pfau Engineering & Surveying PLLC HydroCAD® 7.00 s/n 001436 © 1986-2003 Applied Microcomputer Systems

Subcatchment 1S: SUBCATCHMENT 1S

Runoff	=	4.04 cfs @	12.18 hrs.	Volume=	0.355 af, Depth= 0.60"
runon		7.07 013 @	12.10183,	voiume-	0.000 al, Deptil- 0.00

Runoff by SCS TR-20 method, UH=SCS, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 1 YEAR STORM Rainfall=2.61"

Area (ac)	C	N Desi	cription			
0.200			ed parking	& roofe		
3.070				over, Good		
3.790				· ·	, 136 0	
			<u>ds, Fair, F</u>			
7.060) /	4 Weig	ghted Aver	age		
	ngth feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description	
10.0	100	0.0500	0.2		Sheet Flow,	
1.5	308	0.0460	3.5		Grass: Dense n= 0.240 P2= 3.15" Shallow Concentrated Flow, Unpaved Kv= 16.1 fps	
11.5	408	Total				

Subcatchment 1S: SUBCATCHMENT 1S



Type III 24-hr 1 YEAR STORM Rainfall=2.61" **Proposed Conditions 2021** Prepared by Pietrzak & Pfau Engineering & Surveying PLLC HydroCAD® 7.00 s/n 001436 © 1986-2003 Applied Microcomputer Systems

Subcatchment 2S: SUBCATCHMENT 2S

Page 4

1.56 cfs @ 12.29 hrs, Volume= 0.159 af, Depth= 0.64" Runoff

Runoff by SCS TR-20 method, UH=SCS, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 1 YEAR STORM Rainfall=2.61"

_	Area	(ac) (CN Des	cription		
	0.	180	98 Pav	ed parking	& roofs	
	0.	996		ods, Fair, F		
_	<u> </u>	794	<u>74 >75</u>	% Grass c	over, Good	, HSG C
	2.	970	75 Wei	ghted Ave	rage	
	Tc (min)	Length (feet)		Velocity (ft/sec)	Capacity (cfs)	Description
	16.4	100	0.0400	0.1		Sheet Flow,
	2.1	645	0.1000	5.1		Woods: Light underbrush n= 0.400 P2= 3.15" Shallow Concentrated Flow, Unpaved Kv= 16.1 fps
	18.5	745	Total			

Subcatchment 2S: SUBCATCHMENT 2S



Type III 24-hr 1 YEAR STORM Rainfall=2.61"

Page 5

Prepared by Pietrzak & Pfau Engineering & Surveying PLLC HydroCAD® 7.00 s/n 001436 © 1986-2003 Applied Microcomputer Systems

Subcatchment 3S: SUBCATCHMENT 3S

Runoff = 4.14 cfs @ 12.17 hrs, Volume= 0.340 af, Depth= 0.78"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 1 YEAR STORM Rainfall=2.61"

Area	(ac)	CN De	escription			
0.	790	98 Pa	aved parking	& roofs		
4.	270	74 >7	'5% Grass c	over, Good	, HSG C	
0.	150	76 W	oods/grass	comb., Fair,	HSG C	
5.	210	78 W	eighted Ave	rage		
Tc (min)	Length (feet)	•		Capacity (cfs)	Description	
7.6	100	0.100	0 0.2		Sheet Flow, Grass: Dense n= 0.240 P2= 3.15"	
2.8	590	0.049	0 3.6		Shallow Concentrated Flow, Unpaved Kv= 16.1 fps	
0.9	261	0.055	0 4.8		Shallow Concentrated Flow, Paved Kv= 20.3 fps	
11.3	951	Total			······································	

Subcatchment 3S: SUBCATCHMENT 3S



Type III 24-hr 1 YEAR STORM Rainfall=2.61" ving PLLC Page 6

Prepared by Pietrzak & Pfau Engineering & Surveying PLLC HydroCAD® 7.00 s/n 001436 © 1986-2003 Applied Microcomputer Systems

Subcatchment 4S: SUBCATCHMENT 4S

Runoff = 3.30 cfs @ 12.24 hrs, Volume= 0.305 af, Depth= 0.94"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 1 YEAR STORM Rainfall=2.61"

 Area	(ac) C	N Des	cription		
 1.	050 🖇	8 Pave	ed parking	& roofs	
1.	650 7	74 >75	% Grass c	over, Good,	, HSG C
1.	070 7	73 Woo	ods, Fair, ⊦	ISG C	
0.	140 9	98 PON	ID SURFA	CE	
З.	910 8	31 Weig	ghted Aver	age	
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
 15.0	100	0.0500	0.1	x	Sheet Flow,
0.5	153	0.1100	5.3		Woods: Light underbrush n= 0.400 P2= 3.15" Shallow Concentrated Flow, Unpaved Kv= 16.1 fps
0.5	163	0.0610	5.0		Shallow Concentrated Flow,
0.5	416	0.0625	14.9	26.26	Paved Kv= 20.3 fps Circular Channel (pipe),
					Diam= 18.0" Area= 1.8 sf Perim= 4.7' r= 0.38' n= 0.013

16.5 832 Total

Subcatchment 4S: SUBCATCHMENT 4S



~

Prepared by Pietrzak & Pfau Engineering & Surveying PLLC HydroCAD® 7.00 s/n 001436 © 1986-2003 Applied Microcomputer Systems

Reach 5R: (new Reach)

								•			,					
Inflow Area Inflow Outflow	=	0.14	0 ac, cfs @ cfs @	D 17	.64 h	rs, V	olum	ie=	or 1	YEAF 0.082 0.079	2 af			Lag	= 12.6	3 min
Routing by Max. Veloc Avg. Veloci	ity= 1.3	fps,	Min. T	Trave	l Tim	e= 7.9	9 mir	۱	0.00 H	nrs, dt	= 0.0	5 hrs				
Peak Depth Capacity at Inlet Invert 3.00' x 1.5 Side Slope	bank fi = 410.0 50' dee	ulĪ= 10 0', Οι p chai	04.69 utlet li nnel,	cfs nvert=	= 372).030	.42' Len	gth=	616.0)' Slo	ope= (0.061	0 '/'				
					Ī	Reac	h 5F	R: (ne	ew R	leach	1)					
						Hye	drogra	aph			-					
	<u></u>								ł		÷		<u> </u>			
0.45-2).14 cfs	<u> </u>		Inflow Outflow
0.15 0.14	(JIn	flo	xr 🛆	roa	=3	91() a	^			- 18 1 8 18 1		4 cfs	ちょうちょう	93/81	
0.13							4	Y .								
0.12	∕] P	eak	De	ptr	<u>}=0</u>	.03							+			
0.11-	M	ax	Vēl	=1	3 fr	S										
0.1		*******			<u> </u>											
0.09	n	=0.(130								· · · · · · · · · · · · · · · · · · ·			+	••••••	
(si) 0.08 0.08 0.07		=61	6.0	.							*	·\	+			
0.07					6									1		
0.06	_ − ⊃	=0.0	101	U-1		/ 	1									
0.05	C	apa	icit	v=1	04.	69	cfs									
0.04	·]										; ; ;	; ; ; ;				
0.03	<u></u>															
0.02						2 2 2					ļ,	ļ	<u> </u>	ļ	<u> </u>	J
0.01	mm			1 SM		1888										
0 . 144 5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	

Time (hours)

Prepared by Pietrzak & Pfau Engineering & Surveying PLLC HydroCAD® 7.00 s/n 001436 © 1986-2003 Applied Microcomputer Systems

Pond 1P: ON SITE CULVERT PIPE

Inflow Are	a =	7.060 ac, Inflow Depth = 0.60°	for 1 YEAR STORM event
Inflow	=	4.04 cfs @ 12.18 hrs, Volume=	0.355 af
Primary	H	4.04 cfs @ 12.18 hrs, Volume=	0.355 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs



Pond 1P: ON SITE CULVERT PIPE

Proposed Conditions 2021Type III 24-hr 1 YEAR STORM Rainfall=2.61"Prepared by Pietrzak & Pfau Engineering & Surveying PLLCPage 9HydroCAD® 7.00 s/n 001436 © 1986-2003 Applied Microcomputer SystemsPage 9

Pond 2P: DESIGN POINT 2P

Inflow Area =	2.970 ac, Inflow Depth = 0.64"	for 1 YEAR STORM event
Inflow =	1.56 cfs @ 12.29 hrs, Volume=	0.159 af
Primary =	1.56 cfs @ 12.29 hrs, Volume=	0.159 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs



Pond 2P: DESIGN POINT 2P

Proposed Conditions 2021 Type

Type III 24-hr 1 YEAR STORM Rainfall=2.61" ig PLLC Page 10

Prepared by Pietrzak & Pfau Engineering & Surveying PLLC HydroCAD® 7.00 s/n 001436 © 1986-2003 Applied Microcomputer Systems

Pond 3P: OFF SITE CULVERT PIPE

Inflow Area =	9.120 ac, Inflow Depth = 0.55"	for 1 YEAR STORM event
Inflow =	4.14 cfs @ 12.17 hrs, Volume=	0.419 af
Primary =	4.14 cfs @ 12.17 hrs, Volume=	0.419 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs



Pond 3P: OFF SITE CULVERT PIPE

Prepared by Pietrzak & Pfau Engineering & Surveying PLLC HydroCAD® 7.00 s/n 001436 © 1986-2003 Applied Microcomputer Systems

Pond 4P: Type F-5 Bioretention Pond

Inflow Area =	3.910 ac, Inflow Depth = 0.94"	
Inflow =	3.30 cfs @ 12.24 hrs, Volume=	
Outflow =	0.14 cfs @ 17.64 hrs, Volume=	
Primary =	0.14 cfs @ 17.64 hrs, Volume=	0.082 af

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 418.42' @ 17.64 hrs Surf.Area= 0 sf Storage= 9,916 cf Plug-Flow detention time= 261.6 min calculated for 0.082 af (27% of inflow) Center-of-Mass det. time= 163.6 min (981.1 - 817.5)

#	Invert	Avail.St	orage	Storage Description
1	417.00'			Custom Stage Data Listed below
		_		
Elev	ation	Cum.Store		
(feet)	(cubic-feet)		
	7.00	0		
	8.00	6,396		
	20,00	23,049		
	22.00	45,796		
	2.00			
#	Routing	Invert		Devices
1	Primary	413.00'	18.0"	x 70.0' long Culvert RCP, rounded edge headwall, Ke= 0.100
•			Outlet	Invert= 410.00' S= 0.0429 '/' n= 0.011 Cc= 0,900
2	Device 1	417.50'	2.4" V	ert. Orifice/Grate C= 0.600
3	Device 1			(0.50' Vert. Orifice/Grate C= 0.600
4	Device 1	420.00'	1.00' x	1.00' Vert. Orifice/Grate C= 0.600
5	Device 1	421.00	4.00' x	2.50' Horiz. Orifice/Grate Limited to weir flow C= 0.600
Ŭ	Donico .			
Prim	arv OutFlo	w Max=0.14	4 cfs @	17.64 hrs HW=418.42' (Free Discharge)
1_1:	=Culvert (Passes 0.14	cfs of 2	25.08 cfs potential flow)
∱	-2=Orifice	/Grate (Orifi	ce Con	trols 0.14 cfs @ 4.4 fps)

(Controls 0.00 cfs)
(Controls 0.00 cfs)
(Controls 0.00 cfs)

Type III 24-hr 1 YEAR STORM Rainfall=2.61"

Prepared by Pietrzak & Pfau Engineering & Surveying PLLC HydroCAD® 7.00 s/n 001436 © 1986-2003 Applied Microcomputer Systems Page 12



Pond 4P: Type F-5 Bioretention Pond

Proposed Conditions 2021 Type III 24-hr 10 YEAR STORM Rainfall=4.66" Prepared by Pietrzak & Pfau Engineering & Surveying PLLC Page 13 HydroCAD® 7.00 s/n 001436 © 1986-2003 Applied Microcomputer Systems Page 13
Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points Runoff by SCS TR-20 method, UH=SCS Reach routing by Stor-Ind+Trans method , Pond routing by Stor-Ind method
Subcatchment 1S: SUBCATCHMENT 1S Runoff Area=7.060 ac Runoff Depth=1.93" Flow Length=408' Tc=11.5 min CN=74 Runoff=14.23 cfs 1.138 af
Subcatchment 2S: SUBCATCHMENT 2S Runoff Area=2.970 ac Runoff Depth=2.01" Flow Length=745' Tc=18.5 min CN=75 Runoff=5.22 cfs 0.497 af
Runoff Area=5.210 ac Runoff Depth=2.25" Subcatchment 3S: SUBCATCHMENT 3S Flow Length=951' Tc=11.3 min CN=78 Runoff=12.33 cfs 0.978 af
Subcatchment 4S: SUBCATCHMENT 4S Flow Length=832' Tc=16.5 min CN=81 Runoff=8.92 cfs 0.815 af
Reach 5R: (new Reach) Peak Depth=0.21' Max Vel=3.9 fps Inflow=2.84 cfs 0.559 af n=0.030 L=616.0' S=0.0610 '/' Capacity=104.69 cfs Outflow=2.83 cfs 0.556 af
Pond 1P: ON SITE CULVERT PIPEInflow=14.23 cfs1.138 afPrimary=14.23 cfs1.138 af
Pond 2P: DESIGN POINT 2P Inflow=5.22 cfs 0.497 af Primary=5.22 cfs 0.497 af
Pond 3P: OFF SITE CULVERT PIPEInflow=12.42 cfs1.533 afPrimary=12.42 cfs1.533 af
Pond 4P: Type F-5 Bioretention Pond Peak Elev=419.29' Storage=17,173 cf Inflow=8.92 cfs 0.815 af Outflow=2.84 cfs 0.559 af

Γ

 $\left[\right]$

.

() ()

[]

Total Runoff Area = 19.150 ac Runoff Volume = 3.427 af Average Runoff Depth = 2.15"

<u>riyaloo,</u>		<u>J_s/n_0014</u>	<u>36_© 1986</u>	5-2003 Appli	& Surveying PLLC	Page 14
			Subc	atchment	1S: SUBCATCHMENT 1S	
Runoff	=		fs @ 12.	17 hrs, Vol	ume= 1.138 af. Depth= 1.93"	
Runoff & Type III	by SCS 1 24-hr 10	FR-20 mei YEAR ST	hod, UH= FORM_Ra	SCS, Time infall=4.66"		
<u>Area</u>		<u>CN Des</u>	cription			
3. 3.	.070 .790	74 >75 7 <u>3 Woc</u>	ed parking % Grass c ods, Fair, H ghted Ave	over, Good ISG C	, HSG C	
Tc (min)	Length (feet)	Siope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description	
10.0	100	0.0500	0.2	(013)	Sheet Flow,	
1.5	308	0.0460	3.5		Grass: Dense n= 0.240 P2= 3.15" Shallow Concentrated Flow	
11.5	408	Total			Unpaved Kv= 16.1 fps	
ς.			Subcate	chment 1	S: SUBCATCHMENT 1S	



Proposed Conditions 2021 Prepared by Pietrzak & Pfau F

Type III 24-hr 10 YEAR STORM Rainfall=4.66

Proposed Co	nditions 2021
-------------	---------------

Prepared by Pietrzak & Pfau Engineering & Surveying PLLC HydroCAD® 7.00 s/n 001436 © 1986-2003 Applied Microcomputer Systems

Subcatchment 2S: SUBCATCHMENT 2S

Runoff	=	5.22 cfs @	12.26 hrs, Volume=	0.497 af, Depth= 2.01"
			· · · · · · · · · · · · · · · · · · ·	

Runoff by SCS TR-20 method, UH=SCS, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 10 YEAR STORM Rainfall=4.66"

Area	(ac) (CN Des	cription			
0.	180	98 Pav	ed parking	& roofs		
0.	996	73 Wo	ods, Fair, F	ISG C		
1.	794	74 >75	<u>% Grass co</u>	over, Good	, HSG C	
2.	970	75 Wei	ghted Aver	age		
Tc (min)	Length (feet)	+	Velocity (ft/sec)	Capacity (cfs)	Description	
16.4	100	0.0400	0.1		Sheet Flow,	
2.1	645	0.1000	5.1		Woods: Light underbrush n= 0.400 F Shallow Concentrated Flow, Unpaved Kv= 16.1 fps	2= 3.15"
18.5	745	Total				

Subcatchment 2S: SUBCATCHMENT 2S



Type III 24-hr 10 YEAR STORM Rainfall=4.66"

Page 16

Prepared by Pietrzak & Pfau Engineering & Surveying PLLC HydroCAD® 7.00 s/n 001436 © 1986-2003 Applied Microcomputer Systems

Subcatchment 3S: SUBCATCHMENT 3S

Runoff	=	12.33 cfs	s@ 12.1	6 hrs, Volu	ume= 0.978 af, Depth= 2.25"						
Runoff b Type III	Runoff by SCS TR-20 method, UH=SCS, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 10 YEAR STORM Rainfall=4.66"										
Area	(ac) C	N Des	cription								
			ed parking								
				over, Good							
				comb., Fair	, HSG C						
5.	.210 7	78 Weig	ghted Aver	rage							
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description						
7.6	100	0.1000	0.2		Sheet Flow,						
2.8	590	0.0490	3.6		Grass: Dense n= 0.240 P2= 3.15" Shallow Concentrated Flow, Unpaved Kv= 16.1 fps						
0.9	261	0.0550	4.8		Shallow Concentrated Flow,						
			.		Paved Kv= 20.3 fps						
11.3	951	Total									

Subcatchment 3S: SUBCATCHMENT 3S



1

Type III 24-hr 10 YEAR STORM Rainfall=4.66" Page 17

18

16

15

14

13

12 13 Time (hours)

11

10

17

19

20

Prepared by Pietrzak & Pfau Engineering & Surveying PLLC HydroCAD® 7.00 s/n 001436 © 1986-2003 Applied Microcomputer Systems

Subcatchment 4S: SUBCATCHMENT 4S

0.815 af, Depth= 2.50" 8.92 cfs @ 12.23 hrs, Volume= Runoff =

Runoff by SCS TR-20 method, UH=SCS, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 10 YEAR STORM Rainfall=4.66"

<u>Area (a</u>			ription		·····
1.05	50 74	4 >75%	d parking 6 Grass co ds, Fair, H	over, Good,	1, HSG C
1.01 0.14			D SURFA		
3.9		1 Weig	hted Aver	age	
Tc ((min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	
15.0	100	0.0500	0.1		Sheet Flow, Woods: Light underbrush n= 0.400 P2= 3.15"
0.5	153	0.1100	5.3		Shallow Concentrated Flow, Unpaved Kv= 16.1 fps
0.5	163	0.0610	5.0		Shallow Concentrated Flow, Paved Kv= 20.3 fps
0.5	416	0.0625	14.9	26.26	Circular Channel (pipe), Diam= 18.0" Area= 1.8 sf Perim= 4.7' r= 0.38' n= 0.01
16.5	832	Total	· · ·		
			Subca	tchment	t 4S: SUBCATCHMENT 4S
			Subca		t 4S: SUBCATCHMENT 4S
	1		Subca		t 4S: SUBCATCHMENT 4S rograph
			Subca	Hydro	rograph
9			Subca	Hydro	8.82 cfs Type III 24-hr 10 YEAR STORM
Í			Subca	Hydro	Type III 24-hr 10 YEAR STORM Rainfall=4.66"
9 9 8 1			Subca	Hydro	rograph ^{8.82 cfs} Type III 24-hr 10 YEAR STORM Rainfall=4.66 ^m Runoff Area=3.910 ac
9- 9- 8- 7-			Subca	Hydro	Type III 24-hr 10 YEAR STORM Rainfall=4.66" Runoff Area=3.910 ac Runoff Volume=0.815 af
9- 9- 8- 7-			Subca	Hydro	rograph ^{8.92 cfs} Type III 24-hr 10 YEAR STORM Rainfall=4.66 ⁿ Runoff Area=3.910 ac Runoff Volume=0.815 af Runoff Depth=2.50 ^m
9 9 8 7			Subca	Hydro	Type III 24-hr 10 YEAR STORM Rainfall=4.66" Runoff Area=3.910 ac Runoff Volume=0.815 af Runoff Depth=2.50" Flow Length=832'
9 9 8 7			Subca	Hydro	rograph ^{8.92 cfs} Type III 24-hr 10 YEAR STORM Rainfall=4.66 ⁿ Runoff Area=3.910 ac Runoff Volume=0.815 af Runoff Depth=2.50" Flow Length=832' Tc=16.5 min
efs) 1.1.1.1.1.1.1.1.1.1.1.1.1.1.1.1.1.1.1.			Subca	Hydro	Type III 24-hr 10 YEAR STORM Rainfall=4.66" Runoff Area=3.910 ac Runoff Volume=0.815 af Runoff Depth=2.50" Flow Length=832'
Flow (cfs)			Subca	Hydro	rograph ^{8.92 cfs} Type III 24-hr 10 YEAR STORM Rainfall=4.66 ⁿ Runoff Area=3.910 ac Runoff Volume=0.815 af Runoff Depth=2.50" Flow Length=832' Tc=16.5 min

Type III 24-hr 10 YEAR STORM Rainfall=4.66" Prepared by Pietrzak & Pfau Engineering & Surveying PLLC HydroCAD® 7.00 s/n 001436 © 1986-2003 Applied Microcomputer Systems Page 18

Reach 5R: (new Reach)



Prepared by Pietrzak & Pfau Engineering & Surveying PLLC HydroCAD® 7.00 s/n 001436 © 1986-2003 Applied Microcomputer Systems

Pond 1P: ON SITE CULVERT PIPE

Inflow Area =	7.060 ac, Inflow Depth = 1.93 "	for 10 YEAR STORM event
Inflow =	14.23 cfs @ 12.17 hrs, Volume=	1.138 af
Primary ≐	14.23 cfs @ 12.17 hrs, Volume=	1.138 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs.



Pond 1P: ON SITE CULVERT PIPE

Type III 24-hr 10 YEAR STORM Rainfall=4.66" veying PLLC Page 20

Prepared by Pietrzak & Pfau Engineering & Surveying PLLC HydroCAD® 7.00 s/n 001436 © 1986-2003 Applied Microcomputer Systems

Pond 2P: DESIGN POINT 2P

Inflow Area	a =	2.970 ac, Inflow Depth = 2.01"	for 10 YEAR STORM event
Inflow	=	5.22 cfs @ 12.26 hrs, Volume=	• 0.497 af
Primary	=	5.22 cfs @ 12.26 hrs, Volume=	• 0.497 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs



Pond 2P: DESIGN POINT 2P

1.

١.

Prepared by Pietrzak & Pfau Engineering & Surveying PLLC HydroCAD® 7.00 s/n 001436 © 1986-2003 Applied Microcomputer Systems

Pond 3P: OFF SITE CULVERT PIPE

Inflow Area	П	9.120 ac, Inflow Depth = 2.02" for 10 YEAR STORM event
Inflow	=	12.42 cfs @ 12.16 hrs, Volume= 1.533 af
Primary	=	12.42 cfs @ 12.16 hrs, Volume= 1.533 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs



Pond 3P: OFF SITE CULVERT PIPE

Type III 24-hr 10 YEAR STORM Rainfall=4.66"

Proposed Conditions 2021Type III 24-hr 10Prepared by Pietrzak & Pfau Engineering & Surveying PLLCHydroCAD® 7.00 s/n 001436 © 1986-2003 Applied Microcomputer Systems Page 22

Pond 4P: Type F-5 Bioretention Pond

Inflow Area = Inflow = Outflow = Primary =	3.910 ac, Inflow Depth = 2.50" for 10 YEAR STORM event 8.92 cfs @ 12.23 hrs, Volume= 0.815 af 2.84 cfs @ 12.69 hrs, Volume= 0.559 af, Atten= 68%, Lag= 27.4 min 2.84 cfs @ 12.69 hrs, Volume= 0.559 af									
Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 419.29' @ 12.69 hrs Surf.Area= 0 sf Storage= 17,173 cf Plug-Flow detention time= 140.4 min calculated for 0.557 af (68% of inflow) Center-of-Mass det. time= 74.2 min (869.8 - 795.6)										
# Invert	Avail.Storage Storage Description									
1 417.00'	45,796 cf Custom Stage Data Listed below									
	cum.Store ubic-feet) 0									
418.00	6,396									
420.00	23,049									
422.00	45,796									
# Routing	Invert Outlet Devices									
1 Primary	413.00' 18.0" x 70.0' long Culvert RCP, rounded edge headwall, Ke= 0.100									
2 Device 1	Outlet invert= 410.00' S= $0.0429'/$ n= 0.011 Cc= 0.900									
2 Device 1 3 Device 1	417.50' 2.4" Vert. Orifice/Grate C= 0.600									
3 Device 1 4 Device 1	418.50' 1.50' x 0.50' Vert. Orifice/Grate C= 0.600									
	420.00' 1.00' x 1.00' Vert. Orifice/Grate C= 0.600									
O Device I	421.00 4.00 X 2.50 Horiz. Orifice/Grate Limited to weir flow C= 0.600									
-1=Culvert (Pas -2=Orifice/Gr -3=Orifice/Gr -4=Orifice/Gr	5 Device 1 421.00' 4.00' x 2.50' Horiz. Orifice/Grate Limited to weir flow C= 0.600 Primary OutFlow Max=2.83 cfs @ 12.69 hrs HW=419.29' (Free Discharge) 1=Culvert (Passes 2.83 cfs of 27.32 cfs potential flow) 2=Orifice/Grate (Orifice Controls 0.20 cfs @ 6.3 fps) 3=Orifice/Grate (Orifice Controls 2.64 cfs @ 3.5 fps) 4=Orifice/Grate (Controls 0.00 cfs) 5=Orifice/Grate (Controls 0.00 cfs)									



Pond 4P: Type F-5 Bioretention Pond

Proposed Conditions 2021	Type III 24-hr 25 YEAR STORM Rainfall=5.84"
Prepared by Pietrzak & Pfau Engineering	& Surveying PLLC Page 24
HydroCAD® 7.00 s/n 001436 © 1986-2003 App	lied Microcomputer Systems
Runoff by SC	20.00 hrs, dt=0.05 hrs, 301 points CS TR-20 method, UH=SCS ns method - Pond routing by Stor-Ind method
Subcatchment 1S: SUBCATCHMENT 1S	Runoff Area=7.060 ac Runoff Depth=2.83"
Flow	Length=408' Tc=11.5 min CN=74 Runoff=20.90 cfs 1.667 af
Subcatchment 2S: SUBCATCHMENT 2S	Runoff Area=2.970 ac Runoff Depth=2.92"
Flo	w Length=745' Tc=18.5 min CN=75 Runoff=7.60 cfs 0.722 af
Subcatchment 3S: SUBCATCHMENT 3S	Runoff Area=5.210 ac Runoff Depth=3.21"
Flow	Length=951' Tc=11.3 min CN=78 Runoff=17.50 cfs 1.393 af
Subcatchment 4S: SUBCATCHMENT 4S	Runoff Area=3.910 ac Runoff Depth=3.50"
Flow	Length=832' Tc=16.5 min CN=81 Runoff=12.42 cfs 1.139 af
Reach 5R: (new Reach)	Peak Depth=0.27' Max Vel=4.5 fps Inflow=4.24 cfs 0.877 af
n=0.030 L=616.0	' S=0.0610 '/' Capacity=104.69 cfs Outflow=4.24 cfs 0.873 af
Pond 1P: ON SITE CULVERT PIPE	Inflow≍20.90 cfs 1.667 af Primary=20.90 cfs 1.667 af
Pond 2P: DESIGN POINT 2P	Inflow=7.60 cfs 0.722 af Primary=7.60 cfs 0.722 af
Pond 3P: OFF SITE CULVERT PIPE	Inflow=17.86 cfs 2.266 af Primary=17.86 cfs 2.266 af
Pond 4P: Type F-5 Bioretention Pond P	eak Elev=419.99' Storage=22,932 cf Inflow=12.42 cfs 1.139 af Outflow=4.24 cfs 0.877 af

Total Runoff Area = 19.150 ac Runoff Volume = 4.920 af Average Runoff Depth = 3.08"

ال ۶

٦ زا

Ţ

Prepared by Pietrzak & Pfau Engineering & Surveying PLLC HydroCAD® 7.00 s/n 001436 © 1986-2003 Applied Microcomputer Systems

Subcatchment 1S: SUBCATCHMENT 1S

Runoff	=	20.90 cfs @	12.16 hrs.	Volume=	1.667 af, Depth= 2.83"
--------	---	-------------	------------	---------	------------------------

Runoff by SCS TR-20 method, UH=SCS, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 25 YEAR STORM Rainfall=5.84"

_	_Area	(ac)	CN	Desc	ription			
	0.	200	98	Pave	d parking	& roofs		
	3.	070				over, Good,	, HSG C	
·	3.	790	73	Woo	<u>ds, Fair, ⊢</u>	ISG C		
	7.	060	74	Weig	phted Aver	age		
	Tc (min)	Length (feet)		ope ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description	
	10.0	100) 0.0	500	0.2		Sheet Flow, Grass: Dense n= 0.240 P2= 3.15"	
	1.5	308	3 0.0	460	3.5		Shallow Concentrated Flow, Unpaved Kv= 16.1 fps	
	11.5	408	3 Tot	al			· · · ·	

Subcatchment 1S: SUBCATCHMENT 1S



Type III 24-hr 25 YEAR STORM Rainfall=5.84"

Prepared by Pietrzak & Pfau Engineering & Surveying PLLC HydroCAD® 7.00 s/n 001436 © 1986-2003 Applied Microcomputer Systems

Subcatchment 2S: SUBCATCHMENT 2S

Runoff	Ξ	7.60 cf	s@ 12.2	6 hrs, Voli	ume= 0.722 af, Depth= 2.92"
Runoff by SCS TR-20 method, UH=SCS, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 25 YEAR STORM Rainfall=5.84"					
Area (ac) CN Description					
0.180 98 Paved parking & roofs					
0.996 73 Woods, Fair, HSG C					
<u>1.794 74 >75% Grass cover, Good, HSG C</u>					
2.970 75 Weighted Average					
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
16.4	100	0.0400	0.1		Sheet Flow,
2.1	645	0.1000	5.1		Woods: Light underbrush n= 0.400 P2= 3.15" Shallow Concentrated Flow, Unpaved Kv= 16.1 fps
18.5	745	Total		· · · ·	
Subcatchment 2S: SUBCATCHMENT 2S Hydrograph					



Page 26
Page 27

Prepared by Pietrzak & Pfau Engineering & Surveying PLLC HydroCAD® 7.00 s/n 001436 © 1986-2003 Applied Microcomputer Systems

Subcatchment 3S: SUBCATCHMENT 3S

Runoff = 17.50 cfs @ 12.16 hrs, Volume= 1.393 af, Depth= 3.21"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 25 YEAR STORM Rainfall=5.84"

•	Area	(ac) (CN De	scription			
		790	98 Pa	ved parking	& roofs		
	4.3	270		5% Ġrass c		, HSG C	
	0.	150	<u>76</u> Wo	ods/grass o	comb., Fair,	HSG C	
	5.	210	78 We	eighted Ave	rage		
	Tc (min)	Length (feet)		•	Capacity (cfs)	Description	
	7.6	100	0.1000	0.2		Sheet Flow,	
	2.8	590	0.0490) 3.6		Grass: Dense n= 0.240 P2= 3.15" Shallow Concentrated Flow, Unpaved Kv= 16.1 fps	
	0.9	261	0.0550	4.8		Shallow Concentrated Flow,	
						Paved Kv= 20.3 fps	40.0
	113	951	Total				

Subcatchment 3S: SUBCATCHMENT 3S



Type III 24-hr 25 YEAR STORM Rainfall=5.84" veving PLLC Page 28

Prepared by Pietrzak & Pfau Engineering & Surveying PLLC HydroCAD® 7.00 s/n 001436 © 1986-2003 Applied Microcomputer Systems

Subcatchment 4S: SUBCATCHMENT 4S

Runoff = 12.42 cfs @ 12.22 hrs, Volume= 1.139 af, Depth= 3.50"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 25 YEAR STORM Rainfall=5.84"

Area	(ac) C	N Des	cription		
1.	.050 §	98 Pave	ed parking	& roofs	
1.	.650	74 >75	% Grass c	over, Good	, HSG C
1.	.070 7	73 Woo	ods, Fair, F	ISGC	
0	.140 9	<u>98 PON</u>	ID SURFA	CE	
3.	.910 8	31 Weig	ghted Aver	rage	
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
15.0	100	0.0500	0.1	···- · · · · · · · · · · · · · ·	Sheet Flow,
0.5	153	0.1100	5.3		Woods: Light underbrush n= 0.400 P2= 3.15" Shallow Concentrated Flow, Unpaved Kv= 16.1 fps
0.5	163	0.0610	5.0		Shallow Concentrated Flow,
0.5	416	0.0625	14.9	26.26	Paved Kv= 20.3 fps Circular Channel (pipe), Diam= 18.0" Area= 1.8 sf Perim= 4.7' r= 0.38' n= 0.013
40.5					Diam- 10.0 Alea- 1.0 31 1 Chilli- 4.7 1- 0.30 11- 0.013

16.5 832 Total

Subcatchment 4S: SUBCATCHMENT 4S



],

Prepared by Pietrzak & Pfau Engineering & Surveying PLLC HydroCAD® 7.00 s/n 001436 © 1986-2003 Applied Microcomputer Systems

Reach 5R: (new Reach)

Inflow Area =	3.910 ac, Inflow Depth = 2.69"	for 25 YEAR STORM event
Inflow =	4.24 cfs @ 12.65 hrs, Volume=	
Outflow =	4.24 cfs @ 12.72 hrs, Volume=	0.873 af, Atten= 0%, Lag= 3.9 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Max. Velocity= 4.5 fps, Min. Travel Time= 2.3 min Avg. Velocity = 2.5 fps, Avg. Travel Time= 4.1 min

Peak Depth= 0.27' @ 12.68 hrs Capacity at bank full= 104.69 cfs Inlet Invert= 410.00', Outlet Invert= 372.42' 3.00' x 1.50' deep channel, n= 0.030 Length= 616.0' Slope= 0.0610 '/' Side Slope Z-value= 2.0 '/'



Type III 24-hr 25 YEAR STORM Rainfall=5.84"Gurveying PLLCPage 30

Prepared by Pietrzak & Pfau Engineering & Surveying PLLC HydroCAD® 7.00 s/n 001436 © 1986-2003 Applied Microcomputer Systems

Pond 1P: ON SITE CULVERT PIPE

Inflow Are	ea =	7.060 ac, Inflow Depth = 2.83"	for 25 YEAR STORM event
Inflow	=	20.90 cfs @ 12.16 hrs, Volume=	
Primary	=	20.90 cfs @ 12.16 hrs, Volume=	1.667 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs



Pond 1P: ON SITE CULVERT PIPE

Proposed Conditions 2021Type III 24-hr 25 YEAR STORM Rainfall=5.84"Prepared by Pietrzak & Pfau Engineering & Surveying PLLCPage 31HydroCAD® 7.00 s/n 001436 © 1986-2003 Applied Microcomputer SystemsPage 31

Pond 2P: DESIGN POINT 2P

Inflow Area =	2.970 ac, Inflow Depth = $2.92"$	for 25 YEAR STORM event
Inflow =	7.60 cfs @ 12.26 hrs, Volume=	0.722 af
Primary =	7.60 cfs @ 12.26 hrs, Volume=	0.722 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs



Pond 2P: DESIGN POINT 2P

Type III 24-hr 25 YEAR STORM Rainfall=5.84" ying PLLC Page 32

Prepared by Pietrzak & Pfau Engineering & Surveying PLLC HydroCAD® 7.00 s/n 001436 © 1986-2003 Applied Microcomputer Systems

Pond 3P: OFF SITE CULVERT PIPE

Inflow Are	ea =	9.120 ac, Inflow Depth = 2.98"	for 25 YEAR STORM event
Inflow	=	17.86 cfs @ 12.17 hrs, Volume=	2.266 af
Primary	=	17.86 cfs @ 12.17 hrs, Volume=	2.266 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs



Pond 3P: OFF SITE CULVERT PIPE

Type III 24-hr 25 YEAR STORM Rainfall=5.84" Prepared by Pietrzak & Pfau Engineering & Surveying PLLC Page 33 HydroCAD® 7.00 s/n 001436 © 1986-2003 Applied Microcomputer Systems

Pond 4P: Type F-5 Bioretention Pond

Inflow Area =	3.910 ac, Inflow Depth = 3.50"	for 25 YEAR STORM event
Inflow =	12.42 cfs @ 12.22 hrs, Volume=	1.139 af
Outflow =	4.24 cfs @ 12.65 hrs, Volume=	0.877 af, Atten= 66%, Lag= 25.7 min
Primary =	4.24 cfs @ 12.65 hrs, Volume=	0.877 af

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 419.99' @ 12.65 hrs Surf.Area= 0 sf Storage= 22,932 cf Plug-Flow detention time= 122.7 min calculated for 0.874 af (77% of inflow) Center-of-Mass det. time= 67.1 min (854.9 - 787.8)

#	Inver	t Avail.St	torage Storage Description
1	417.00	' 45,	796 cf Custom Stage Data Listed below
	ation (feet)	Cum.Store (cubic-feet)	
	7.00	0	
	8.00	6,396	
	20.00	23,049	
42	22.00	45,796	
#	Routing	Invert	Outlet Devices
1	Primary	413.00'	18.0" x 70.0' long Culvert RCP, rounded edge headwall, Ke= 0.100
_		•	Outlet Invert= 410.00' S= 0.0429 '/' n= 0.011 Cc= 0.900
2	Device 1	417.50'	2.4" Vert. Orifice/Grate C= 0.600
3	Device 1	418,50'	1.50' x 0.50' Vert. Orifice/Grate C= 0.600
4	Device 1	420.00'	1.00' x 1.00' Vert. Orifice/Grate C= 0.600
5	Device 1	421.00'	4.00' x 2.50' Horiz. Orifice/Grate Limited to weir flow C= 0.600
Prim	ary OutFlo	w Max=4.24	t cfs @ 12.65 hrs HW=419.99' (Free Discharge)
			cfs of 28.97 cfs potential flow)
↑ _			ce Controls 0.23 cfs @ 7.4 fps)
-			ce Controls 4.01 cfs @ 5.3 fps)
			ntrols 0.00 cfs)

5=Orifice/Grate (Controls 0.00 cfs)

1

Type III 24-hr 25 YEAR STORM Rainfall=5.84" veying PLLC Page 34

Prepared by Pietrzak & Pfau Engineering & Surveying PLLC HydroCAD® 7.00 s/n 001436 © 1986-2003 Applied Microcomputer Systems



Pond 4P: Type F-5 Bioretention Pond

Proposed Conditions 2021 Prepared by Pietrzak & Pfau Engineering HydroCAD® 7.00 s/n 001436 © 1986-2003 App	& Surveying PL	<i>II 24-hr 100</i> LLC er Systems) YEAR	STOF	RM Rainfa	all=8.22" Page 35			
Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points Runoff by SCS TR-20 method, UH=SCS Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method									
Subcatchment 1S: SUBCATCHMENT 1S	w Length=408' To				Runoff De =35.06 cfs				
Subcatchment 2S: SUBCATCHMENT 2S	w Length=745' To				Runoff De =12.64 cfs				
Subcatchment 3S: SUBCATCHMENT 3S	w Length=951' To				Runoff Dej =28.21 cfs				
Subcatchment 4S: SUBCATCHMENT 4S Flow	w Length=832' Tc				Runoff De =19.50 cfs				
Reach 5R: (new Reach) n=0.030 L=616.	Peak Depth=0. 0' S=0.0610 '/' C								
Pond 1P: ON SITE CULVERT PIPE			F		=35.06 cfs =35.06 cfs				
Pond 2P: DESIGN POINT 2P			F		=12.64 cfs =12.64 cfs				
Pond 3P: OFF SITE CULVERT PIPE			F		=31.61 cfs =31.61 cfs				
Pond 4P: Type F-5 Bioretention Pond	Peak Elev=420.92'	Storage=3	-		=19.50 cfs v=8.43 cfs				

Γ

 $\left[\right]$

[

Î.

Ŀ

.

L

L

Total Runoff Area = 19.150 ac Runoff Volume = 8.134 af Average Runoff Depth = 5.10"

Type III 24-hr 100 YEAR STORM Rainfall=8.22" eving PLLC Page 36

Prepared by Pietrzak & Pfau Engineering & Surveying PLLC HydroCAD® 7.00 s/n 001436 © 1986-2003 Applied Microcomputer Systems

Subcatchment 1S: SUBCATCHMENT 1S

Runoff = 35.06 cfs @ 12.16 hrs, Volume= 2.819 af, Depth= 4.79"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 100 YEAR STORM Rainfall=8.22"

Area	(ac) (CN Des	cription			
3.	.070	74 >75	ed parking % Grass co ods, Fair, H	over, Good,	, HSG C	
7.	.060	74 Wei	ghted Aver	age		
Tc (min)	Length (feet)	•	Velocity (ft/sec)	Capacity (cfs)	Description	
10.0	100	0.0500	0.2		Sheet Flow, Grass: Dense n= 0.240 P2= 3.15"	
1.5	308	0.0460	3.5		Shallow Concentrated Flow, Unpaved Kv= 16.1 fps	
445	400		1.1.00			

11.5 408 Total

Subcatchment 1S: SUBCATCHMENT 1S



Proposed Conditions 2021Type III 24-hr 100 YEAR STORM Rainfall=8.22"Prepared by Pietrzak & Pfau Engineering & Surveying PLLCPage 37HydroCAD® 7.00 s/n 001436 © 1986-2003 Applied Microcomputer SystemsPage 37

Subcatchment 2S: SUBCATCHMENT 2S

Runoff = 12.64 cfs @ 12.25 hrs, Volume= 1.212 af, Depth= 4.90"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 100 YEAR STORM Rainfall=8.22"

Area ((ac) (CN Des	cription		
0.1	180	98 Pav	ed parking	& roofs	
0.9	996	73 Woo	ods, Fair, F	ISG C	
1.7	<u>794</u>	74 >75	% Grass c	over, Good,	HSG C
2.8	970	75 Wei	ghted Aver	age	
Tc (min)	Length (feet)		Velocity (ft/sec)	Capacity (cfs)	Description
16.4	100	0.0400	0.1		Sheet Flow,
2.1	645	0.1000	5.1		Woods: Light underbrush n= 0.400 P2= 3.15" Shallow Concentrated Flow, Unpaved Kv= 16.1 fps
18.5	745	Total			

Subcatchment 2S: SUBCATCHMENT 2S



Type III 24-hr 100 YEAR STORM Rainfall=8.22" veving PLLC Page 38

Prepared by Pietrzak & Pfau Engineering & Surveying PLLC HydroCAD® 7.00 s/n 001436 © 1986-2003 Applied Microcomputer Systems

Subcatchment 3S: SUBCATCHMENT 3S

Runoff = 28.21 cfs @ 12.16 hrs, Volume= 2.281 af, Depth= 5.25"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 100 YEAR STORM Rainfall=8.22"

	Area	(ac) C	N Des	cription				
	0.	790 9	98 Pave	ed parking	& roofs			
	4.	270	74 >75 [°]	% Grass c	over, Good	, HSG C		
0.150 76 Woods/grass comb., Fair, HSG C								
5.210 78 Weighted Average								
	Тс	Length	Slope	Velocity	Capacity	Description		
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)			
	7.6	100	0.1000	0.2		Sheet Flow,		
						Grass: Dense n= 0.240 P2= 3.15"		
	2.8	590	0.0490	3.6		Shallow Concentrated Flow,		
						Unpaved Kv= 16.1 fps		
	0.9	261	0.0550	4.8		Shallow Concentrated Flow,		
						Paved Kv= 20.3 fps	· ·	
	11.3	951	Total					

Subcatchment 3S: SUBCATCHMENT 3S



Prepared by Pietrzak & Pfau Engineering & Surveying PLLC HydroCAD® 7.00 s/n 001436 © 1986-2003 Applied Microcomputer Systems

Subcatchment 4S: SUBCATCHMENT 4S

Runoff = 19.50 cfs @ 12.22 hrs, Volume= 1.822 af, Depth= 5.59"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr 100 YEAR STORM Rainfall=8.22"

Area	(ac) <u>(</u>	CN Des	cription		
1.	050	98 Pav	ed parking	& roofs	
1.0	650			over, Good	HSG C
			ds. Fair, H		,
			ID SURFA		
			ahted Aver		,
5.3	910		gnieu Aver	age	
Тс	Length	Slope	Velocity	Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	Description
· · · · · ·		· · · · · · · · · · · · · · · · · · ·		(05)	
15.0	100	0.0500	0.1		Sheet Flow,
					Woods: Light underbrush n= 0.400 P2= 3.15"
0.5	153	0.1100	5.3		Shallow Concentrated Flow,
					Unpaved Kv= 16.1 fps
0.5	163	0.0610	5.0		Shallow Concentrated Flow,
					Paved Kv= 20.3 fps
0.5	416	0.0625	14.9	26.26	Circular Channel (pipe),
					Diam= 18.0" Area= 1.8 sf Perim= 4.7' r= 0.38' n= 0.013
16.5	832	Total	h		

Subcatchment 4S: SUBCATCHMENT 4S



Prepared by Pietrzak & Pfau Engineering & Surveying PLLC HydroCAD® 7.00_s/n 001436 © 1986-2003 Applied Microcomputer Systems

Reach 5R: (new Reach)

 Inflow Area =
 3.910 ac, Inflow Depth = 4.76" for 100 YEAR STORM event

 Inflow =
 8.43 cfs @ 12.57 hrs, Volume=
 1.550 af

 Outflow =
 8.40 cfs @ 12.63 hrs, Volume=
 1.546 af, Atten= 0%, Lag= 3.2 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Max. Velocity= 5.6 fps, Min. Travel Time= 1.8 min Avg. Velocity = 2.8 fps, Avg. Travel Time= 3.7 min

Peak Depth= 0.39' @ 12.59 hrs Capacity at bank full= 104.69 cfs Inlet Invert= 410.00', Outlet Invert= 372.42' 3.00' x 1.50' deep channel, n= 0.030 Length= 616.0' Slope= 0.0610 '/' Side Slope Z-value= 2.0 '/'



Reach 5R: (new Reach)

Type III 24-hr 100 YEAR STORM Rainfall=8.22" Prepared by Pietrzak & Pfau Engineering & Surveying PLLC HydroCAD® 7.00 s/n 001436 © 1986-2003 Applied Microcomputer Systems Page 41

Pond 1P: ON SITE CULVERT PIPE

Inflow Area	a =	7.060 ac, Inflow Depth = 4.79" for 100 YEAR STORM event
Inflow	=	35.06 cfs @ 12.16 hrs, Volume= 2.819 af
Primary	=	35.06 cfs @ 12.16 hrs, Volume= 2.819 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs



Pond 1P: ON SITE CULVERT PIPE

Type III 24-hr 100 YEAR STORM Rainfall=8.22" Prepared by Pietrzak & Pfau Engineering & Surveying PLLC HydroCAD® 7.00 s/n 001436 © 1986-2003 Applied Microcomputer Systems Page 42

Pond 2P: DESIGN POINT 2P

Inflow Are	ea =	2.970 ac, Inflow Depth = 4.90" for 100 YEAR STORM event
Inflow	=	12.64 cfs @ 12.25 hrs, Volume= 1.212 af
Primary	=	12.64 cfs @ 12.25 hrs, Volume= 1.212 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs



Pond 2P: DESIGN POINT 2P

Proposed Conditions 2021

Prepared by Pietrzak & Pfau Engineering & Surveying PLLC HydroCAD® 7.00 s/n 001436 © 1986-2003 Applied Microcomputer Systems

Pond 3P: OFF SITE CULVERT PIPE

Inflow Are	a =	9.120 ac; Inflow Depth = 5.04"	for 100 YEAR STORM event
Inflow	Ξ.	31.61 cfs @ 12.16 hrs, Volume=	3.827 af
Primary	=	31.61 cfs @ 12.16 hrs, Volume=	3.827 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs



Pond 3P: OFF SITE CULVERT PIPE

Type III 24-hr 100 YEAR STORM Rainfall=8.22"

Page 44

Prepared by Pietrzak & Pfau Engineering & Surveying PLLC HydroCAD® 7.00 s/n 001436 © 1986-2003 Applied Microcomputer Systems

Pond 4P: Type F-5 Bioretention Pond

Inflow Area =	3.910 ac, Inflow Depth = 5.59"	for 100 YEAR STORM event
Inflow =	19.50 cfs @ 12.22 hrs, Volume=	1.822 af
Outflow =	8.43 cfs @ 12.57 hrs, Volume=	1.550 af, Atten= 57%, Lag= 21.0 min
Primary =	8.43 cfs @ 12.57 hrs, Volume=	1.550 af

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 420.92' @ 12.57 hrs Surf.Area= 0 sf Storage= 33,519 cf Plug-Flow detention time= 106.4 min calculated for 1.545 af (85% of inflow) Center-of-Mass det. time= 63.7 min (840.4 - 776.7)

....

..

#	Invert	Avail.St	orage	Storage Description
1	417.00'	45,	796 cf	Custom Stage Data Listed below
		_		
Elev	ation	Cum.Store		
	feet)	(cubic-feet)		
41	7.00	0		
41	8.00	6,396		
42	20.00	23,049		
42	2.00	45,796		
#	Routing	Invert	Outlet	Devices
1	Primary	413.00'	18.0"	x 70.0' long Culvert RCP, rounded edge headwall, Ke= 0.100
	2			Invert= 410.00' S= 0.0429 '/' n= 0.011 Cc= 0.900
2	Device 1	417.50'	2.4" V	ert. Orifice/Grate C= 0.600
3	Device 1	418.50'	1.50' x	(0.50' Vert. Orifice/Grate C= 0.600
4	Device 1	420.00'	1.00' x	1.00' Vert. Orifice/Grate C= 0.600
5	Device 1	421.00'	4.00' x	2.50' Horiz. Orifice/Grate Limited to weir flow C= 0.600
				12.57 hrs HW=420.92' (Free Discharge)
				0.65 cfs potential flow)
F	-2=Orifice/	Grate (Orifi	ce Con	trols 0.28 cfs @ 8.8 fps)

. ..

---3=Orifice/Grate (Orifice Controls 5.31 cfs @ 7.1 fps)

-4=Orifice/Grate (Orifice Controls 2.82 cfs @ 3.1 fps)

--5=Orifice/Grate (Controls 0.00 cfs)

Γ

Li

Ŀ

Type III 24-hr 100 YEAR STORM Rainfall=8.22" Prepared by Pietrzak & Pfau Engineering & Surveying PLLC Page 45 HydroCAD® 7.00 s/n 001436 © 1986-2003 Applied Microcomputer Systems



Pond 4P: Type F-5 Bioretention Pond

Time span=5.00-20.00 hrs, dt=0.05 hrs, 301 points Runoff by SCS TR-20 method, UH=SCS Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method							
Subcatchment 1S: SUBCATCHMENT 1S Flow Le	Runoff Area=7.060 ac Runoff Depth=0.09" ngth=408' Tc=11.5 min CN=74 Runoff=0.32 cfs 0.055 af						
Subcatchment 2S: SUBCATCHMENT 2S Flow Le	Runoff Area=2.970 ac Runoff Depth=0.11" ngth=745' Tc=18.5 min CN=75 Runoff=0.15 cfs 0.027 af						
Subcatchment 3S: SUBCATCHMENT 3S Flow Le	Runoff Area=5.210 ac Runoff Depth=0.16" ngth=951' Tc=11.3 min CN=78 Runoff=0.57 cfs 0.071 af						
Subcatchment 4S: SUBCATCHMENT 4S Flow Le	Runoff Area=3.910 ac Runoff Depth=0.23" ngth=832' Tc=16.5 min CN=81 Runoff=0.67 cfs 0.075 af						
· · · · ·	eak Depth=0.00' Max Vel=0.7 fps Inflow=0.00 cfs 0.000 af =0.0610 '/' Capacity=104.69 cfs Outflow=0.00 cfs 0.000 af						
Pond 1P: ON SITE CULVERT PIPE	Inflow=0.32 cfs 0.055 af Primary=0.32 cfs 0.055 af						
Pond 2P: DESIGN POINT 2P	Inflow=0.15 cfs 0.027 af Primary=0.15 cfs 0.027 af						
Pond 3P: OFF SITE CULVERT PIPE	Inflow≕0.57 cfs_0.071 af Primary=0.57 cfs_0.071 af						
Pond 4P: Type F-5 Bioretention Pond Pea	ak Elev=417.51' Storage=3,250 cf Inflow=0.67 cfs 0.075 af Outflow=0.00 cfs 0.000 af						

Type III 24-hr WQ STORM Rainfall=1.39"

Page 46

Proposed Conditions 2021Type III 24-IPrepared by Pietrzak & Pfau Engineering & Surveying PLLCHydroCAD® 7.00 s/n 001436 © 1986-2003 Applied Microcomputer Systems

Total Runoff Area = 19.150 ac Runoff Volume = 0.228 af Average Runoff Depth = 0.14"

Proposed Conditions 2021 Prepared by Pietrzak & Pfau Engineering & Surveying F HydroCAD® 7.00 s/n 001436 © 1986-2003 Applied Microcompu									NQ (STOF	RM Ra	infall=1.39 Page 47
			Subca	tchment [,]	1S: SUBCA	TCF	IMEN	T 1S	1			
Runoff	=	0.32 cfs	s@ 12.4	1 hrs, Volu	me=	0.05	5 af, I	Depth	= 0.0)9"		
			nod, UH=S Rainfall=		Span= 5.00-2	0.00	hrs, di	= 0.0	5 hrs			
Area	(ac) C	N Desc	cription									
			ed parking									·
			⁄₀ Grass c₀ ds, Fair, ⊦	over, Good,	, HSG C							
			hted Ave					÷				
			,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	.90								
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description							
10.0	100	0.0500	0.2		Sheet Flow				~ ~	4 - 11		
1.5	308	0.0460	3,5		Grass: Dens Shallow Co					15.		
	000	010 100			Unpaved k				-,			
11.5	408	Total										
			Subca		1S: SUBCA	TCF	IMEN	T 1S	İ			
		:	: :	Hydro	grapn		1			ļ		
0.34	[/]											Runoff
0.32					Typ	5 III	94 I	ır W	0.5	TOF	2M	
0.3-	2			••••			: :			=1.3		
0.28- 0.26-						·····	1			1		
0.20							off			1		
0.22	·				Ru		f Vo					
(si) 0.2 (si) 0.18			• • • • • • • • • • • • • • • • • • •			R i	unof	f De	pth	=0.0	9''	
≥ 0.18	1						Flov	o tra	nat	h-71	no	

Γ

LJ

Li

Ŀ



Type III 24-hr WQ STORM Rainfall=1.39"

Prepared by Pietrzak & Pfau Engineering & Surveying PLLCPage 48HydroCAD® 7.00 s/n 001436 © 1986-2003 Applied Microcomputer SystemsPage 48

Subcatchment 2S: SUBCATCHMENT 2S

Runoff = 0.15 cfs @ 12.49 hrs, Volume= 0.027 af, Depth= 0.11"	
Runoff by SCS TR-20 method, UH=SCS, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr WQ STORM_Rainfall=1.39"	
Area (ac) CN Description	
0.180 98 Paved parking & roofs	
0.996 73 Woods, Fair, HSG C	
1.794 74 >75% Grass cover, Good, HSG C	
2.970 75 Weighted Average	_
Tc Length Slope Velocity Capacity Description	
(min) (feet) (ft/ft) (ft/sec) (cfs)	
16.4 100 0.0400 0.1 Sheet Flow,	
Woods: Light underbrush n= 0.400 P2= 3.15"	
2.1 645 0.1000 5.1 Shallow Concentrated Flow,	
Unpaved Kv= 16.1 fps	
18.5 745 Total	

Subcatchment 2S: SUBCATCHMENT 2S



HydroCA			Pfau Engi	ineering & 2003 Applie		ing PLLC						Page 4
			Subca	itchment	3S: SU	BCATCH	IMEN	T 3S	i			
Runoff	=	0.57 cf	s@ 12.2	3 hrs, Volu	ime=	0.07	1 af, [Depth	= 0.1	16"		
				SCS, Time S	Span= 5.	.00-20.00	hrs, dt	= 0.0	5 hrs			
			Rainfall=	-1.59								
<u>Area</u>			cription	9 roofo							:	
		÷	ed parking % Grass c	over, Good	. HSG C							
0.	150	<u>76 Woo</u>	ods/grass o	comb., Fair,								
5.	210	78 Wei	ghted Ave	rage								
Tc	Length		Velocity		Descrip	otion						
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	01. (1	<u> </u>	<u>-</u>					
7.6	100	0.1000	0.2		Sheet I Grass:	Flow, Dense r	ו= 0.24	0 P	2=3	15"		
2.8	590	0.0490	3.6		Shallo	w Concer	ntrated	Flov				
0.9	261	0:0550	4.8			ed Kv=1 w _. Concer			,			
0.8	201	0.0000	4.0				າມສເຮບ					
						Kv= 20.3	3 fps	1 101	•,			
11.3	951	Total	· · · · · · · · · · · · · · · · · · ·				3 fps		· ,			
11.3	951	Total	Cubes	to have out f	Paved	Kv= 20.3						
11.3	951	Total	Subca	tchment :	Paved 3S: SUI	Kv= 20.3						
11.3	951	Total	Subca	tchment : Hydro	Paved 3S: SUI	Kv= 20.3						······································
:		Total	Subca	Hydro	Paved 3S: SUI graph	Kv= 20.3						I Runoff
0.6		Total	Subca	Hydro	Paved 3S: SUI graph	Kv= 20.3	IMEN	T 3S		TOF	RM	Runoff
:		Total	Subca	Hydro	Paved 3S: SUI graph	Kv= 20.3	IMEN 24-h	T 3S ir W	QS	1		🛛 Runoff
0.6 - 0.55-		Total	Subca	Hydro	Paved 3S: SUI graph	Kv= 20.3 BCATCH	IMEN 24-h	T 3S ir W Rair	Q S Ifall	=1.3	9"	E Runoff
0.6 - 0.55- 0.6-		Total	Subca	Hydro	Paved 3S: SUI graph	Kv= 20.3 BCATCH Type III Rur	IMEN 24-h I 1off /	T 3S ir W Rair Area	Q S Ifall	=1.3 210	9" ac	Runoff
0.6 0.55 0.45 0.4		Total	Subca	Hydro	Paved 3S: SUI graph	Kv= 20.3 BCATCH Fype III Rur Runof	IMEN 24-h I off f Vol	T 3S ir W Rair Area um	Q S Ifall =5.	=1.3 210 .071	9" ac af	Runoff
0.6 0.55 0.45 0.4		Total	Subca	Hydro	Paved 3S: SUI graph	Kv= 20.3 BCATCH Fype III Rur Runof	IMEN 24-h 10ff / f Vol unof	T 3S Ir W Rair Area um f De	Q S Ifall =5. e=0 pth	=1.3 210 071 =0.1	9" ac af 6"	I Runoff
0.6 0.55 0.5 0.45 0.45 0.35 0.35		Total	Subca	Hydro	Paved 3S: SUI graph	Kv= 20.3 BCATCH Fype III Rur Runof	IMEN 24-h I off f Vol	T 3S T 3S T W Rair Area um f De / Le	Q S Ifall =5. e=0 pth ngt	=1.3 210 .071 =0.1 h=9{	9" ac af 6" 51'	E Runoff
0.6 0.55 0.45 0.45 0.45 0.45 0.35 0.35 0.35		Total	Subca	Hydro	Paved 3S: SUI graph	Kv= 20.3 BCATCH Fype III Rur Runof	IMEN 24-h 10ff / f Vol unof	T 3S T 3S T W Rair Area um f De / Le	Q S ifall =5. =0 pth ngt =11	=1.3 210 071 =0.1 h=9{ .3 m	9" ac af 6" 51' iin	Runoff
0.6 0.55 0.45 0.45 0.45 0.45 0.35 0.35 0.25 0.25		Total	Subca	Hydro	Paved 3S: SUI graph	Kv= 20.3 BCATCH Fype III Runof Runof	IMEN 24-h 1 off f Vol unof Flow	T 3S T 3S T W Rair Area um f De / Le	Q S ifall =5. =0 pth ngt =11	=1.3 210 .071 =0.1 h=9{	9" ac af 6" 51' iin	I Runoff
0.6 0.55 0.4 0.45 0.45 0.35 0.35 0.35 0.35 0.35 0.25 0.25		Total	Subca	Hydro	Paved 3S: SUI graph	Kv= 20.3 BCATCH Fype III Runof Runof	IMEN 24-h 1 off f Vol unof Flow	T 3S T 3S T W Rair Area um f De / Le	Q S ifall =5. =0 pth ngt =11	=1.3 210 071 =0.1 h=9{ .3 m	9" ac af 6" 51' iin	E Runoff
0.6 0.55 0.45 0.45 0.45 0.45 0.35 0.35 0.25 0.25		Total	Subca	Hydro	Paved 3S: SUI graph	Kv= 20.3 BCATCH Fype III Rur Runof	IMEN 24-h 1 off f Vol unof Flow	T 3S T 3S T W Rair Area um f De / Le	Q S ifall =5. =0 pth ngt =11	=1.3 210 071 =0.1 h=9{ .3 m	9" ac af 6" 51' iin	Runoff

Γ

Γ

Γ

 $\left[\right]$

: . .

....

.

L

Type III 24-hr WQ STORM Rainfall=1.39"

Page 50

Prepared by Pietrzak & Pfau Engineering & Surveying PLLC HydroCAD® 7.00 s/n 001436 © 1986-2003 Applied Microcomputer Systems

Subcatchment 4S: SUBCATCHMENT 4S

Runoff = 0.67 cfs @ 12.29 hrs, Volume= 0.075 af, Depth= 0.23"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Type III 24-hr WQ STORM Rainfall=1.39"

Area	(ac)	CN Des	cription		
1.	.050	98 Pav	ed parking	& roofs	
1.	.650	74 >75	% Grass c	over, Good	, HSG C
1.	.070	73 Woo	ods, Fair, H	ISGC	
0.	.140	98 PON	ND SURFA	CE	
3.	.910	81 Wei	ghted Avei	rage	
Tc	Length			Capacity	Description
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
15.0	100	0.0500	0.1		Sheet Flow,
					Woods: Light underbrush n= 0.400 P2= 3.15"
0.5	153	0.1100	5.3		Shallow Concentrated Flow,
					Unpaved Kv= 16.1 fps
0.5	163	0.0610	5.0		Shallow Concentrated Flow,
					Paved Kv= 20.3 fps
0.5	416	0.0625	14.9	26.26	Circular Channel (pipe),
				1.10	Diam= 18.0" Area= 1.8 sf Perim= 4.7' r= 0.38' n= 0.013
16.5	832	Total			

Subcatchment 4S: SUBCATCHMENT 4S



0.000 0.000 0.000

0

6

0.00 cfs

20

Prepared by Pietrzak & Pfau Engineering & Surveying PLLC HydroCAD® 7.00 s/n 001436 © 1986-2003 Applied Microcomputer Systems

Reach 5R: (new Reach)

Inflow Area = 3.910 ac, Inflow Depth = 0.00" for WQ STORM event Inflow = 0.00 cfs @ 20.00 hrs, Volume= 0.000 af Outflow = 0.00 cfs @ 20.00 hrs, Volume= 0.000 af, Atten= 85%, Lag= 0.0 min Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Max. Velocity= 0.7 fps, Min. Travel Time= 13.9 min Avg. Velocity = 0.7 fps, Avg. Travel Time= 13.9 min Peak Depth= 0.00' @ 20.00 hrs Capacity at bank full= 104.69 cfs Inlet Invert= 410.00', Outlet Invert= 372.42' 3.00' x 1.50' deep channel, n= 0.030 Length= 616.0' Slope= 0.0610 '/' Side Slope Z-value= 2.0 '/' Reach 5R: (new Reach) Hydrograph Inflow 0.001 D Outflow 0.00 cfs 0.001 Inflow Area=3.910 ac 0.001 Peak Depth=0.00' 0.001-0.001 Max Vel=0.7 fps 0.001 0.001 n=0.030 0.000 (cfs) L=616.0 0.000 Flow 0.000 S=0.0610 '/' 0.000-Capacity=104.69 cfs

> 12 13 Time (hours)

14

15

16

17

18

19

10

11

ġ

ġ.

Prepared by Pietrzak & Pfau Engineering & Surveying PLLC HydroCAD® 7.00 s/n 001436 © 1986-2003 Applied Microcomputer Systems

Pond 1P: ON SITE CULVERT PIPE

Inflow Area =	7.060 ac, Inflow Depth = 0.09"	for WQ STORM event
Inflow =	0.32 cfs @ 12.41 hrs, Volume=	0.055 af
Primary =	0.32 cfs @ 12.41 hrs, Volume=	0.055 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs



Pond 1P: ON SITE CULVERT PIPE

Prepared by Pietrzak & Pfau Engineering & Surveying PLLC HydroCAD® 7.00 s/n 001436 © 1986-2003 Applied Microcomputer Systems

Pond 2P: DESIGN POINT 2P

Inflow Area =	2.970 ac, Inflow Depth = 0.11"	for WQ STORM event
Inflow =	0.15 cfs @ 12.49 hrs, Volume=	0.027 af
Primary =	0.15 cfs @ 12.49 hrs, Volume=	0.027 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs



Pond 2P: DESIGN POINT 2P

Prepared by Pietrzak & Pfau Engineering & Surveying PLLC HydroCAD® 7.00 s/n 001436 © 1986-2003 Applied Microcomputer Systems

Pond 3P: OFF SITE CULVERT PIPE

Inflow Area =	9.120 ac, Inflow Depth = 0.09"	for WQ STORM event
Inflow =	0.57 cfs @ 12.23 hrs, Volume=	0.071 af
Primary =	0.57 cfs @ 12.23 hrs, Volume=	0.071 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs



Pond 3P: OFF SITE CULVERT PIPE

Type III 24-hr WQ STORM Rainfall=1.39" Page 55

Proposed Conditions 2021Type III 24-Prepared by Pietrzak & Pfau Engineering & Surveying PLLCHydroCAD® 7.00 s/n 001436 © 1986-2003 Applied Microcomputer Systems

Pond 4P: Type F-5 Bioretention Pond

Inflow Area = 3.910 ac, Inflow Depth = 0.23" for WQ STORM event Inflow = 0.67 cfs @ 12.29 hrs, Volume= 0.075 af Outflow = 0.00 cfs @ 20.00 hrs, Volume= 0.000 af, Atten= 100%, Lag= Primary = 0.00 cfs @ 20.00 hrs, Volume= 0.000 af			
Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs Peak Elev= 417.51' @ 20.00 hrs Surf.Area= 0 sf Storage= 3,250 cf Plug-Flow detention time= 482.2 min calculated for 0.000 af (0% of inflow) Center-of-Mass det. time= 341.9 min (1,192.4 - 850.5)			
# Invert Avail.Storage Storage Description			
1 417.00' 45,796 cf Custom Stage Data Listed below			
Elevation Cum.Store (feet) (cubic-feet) 417.00 0 418.00 6,396 420.00 23,049 422.00 45,796			
# Routing Invert Outlet Devices			
1 Primary 413.00' 18.0'' x 70.0' long Culvert RCP, rounded edge headwall, Ke= 0.100 2 Device 1 417.50' 2.4'' Vert. Orifice/Grate C= 0.600 3 Device 1 418.50' 1.50' x 0.50' Vert. Orifice/Grate C= 0.600 4 Device 1 420.00' 1.00' x 1.00' Vert. Orifice/Grate C= 0.600 5 Device 1 420.00' 4.00' x 2.50' Horiz. Orifice/Grate Limited to weir flow C= 0.600			
Primary OutFlow Max=0.00 cfs @ 20.00 hrs HW=417.51' (Free Discharge) -1=Culvert (Passes 0.00 cfs of 22.49 cfs potential flow) -2=Orifice/Grate (Orifice Controls 0.00 cfs @ 0.3 fps) -3=Orifice/Grate (Controls 0.00 cfs) -4=Orifice/Grate (Controls 0.00 cfs) -5=Orifice/Grate (Controls 0.00 cfs)			

Type III 24-hr WQ STORM Rainfall=1.39" Page 56

Proposed Conditions 2021Type III 24Prepared by Pietrzak & Pfau Engineering & Surveying PLLCHydroCAD® 7.00 s/n 001436 © 1986-2003 Applied Microcomputer Systems



Pond 4P: Type F-5 Bioretention Pond

APPENDIX 7

Γ

Γ

ſ

TR-20 Supporting Data



Γ

ł

-

L

L



Extreme Precipitation Estimates

Į

].

[.

1

L

L

F فسيقا



[

Γ

[]

1



Extreme Precipitation Estimates 24hr 100yr

Γ

L

New York State Stormwater Management Design Manual

Chapter 4:	Unified Stormwater Sizing Criteria
Section 4.2	Water Quality Volume (WQv)

Figure 4.1: 90th Percentile Rainfall in New York State (NYSDEC, 2013)


Extreme Precipitation Tables

Northeast Regional Climate Center

Data represents point estimates calculated from partial duration series. All precipitation amounts are displayed in inches.

Smoothing	Yes	
State	New York	
Location		
Longitude	74.030 degrees West	
Latitude	41.548 degrees North	
Elevation	0 feet	
Date/Time	Tue, 09 Feb 2021 23:47:43 -0500	

Extreme Precipitation Estimates

	5min	10min	15min	30min	60min	120min		1hr	2hr	3hr	6hr	12hr	24hr	48hr		1day	2day	4day	7day	10day	
1yr	0.32	0.50	0.62	0.81	1.01	1.26	1yr	0.87	1.18	1.44	1.76	2.14	2.61	2.97	1yr	2.31	2.86	3.31	3.96	4.59	1yr
2yr	0.39	0.59	0.74	0.97	1.22	1.52	2yr	1.06	1.42	1.74	2.13	2.60	3.15	3.56	2yr	2.78	3.42	3.92	4.62	5.26	2yr
5yr	0.45	0.71	0.89	1.19	1.52	1.91	5yr	1.31	1.75	2.19	2.69	3.26	3:93	4.49	5yr	3,48	4.32	4.96	5.72	6.48	5yr
10yr	0.51	0.80	1.01	1.38	1.79	2.26	10yr	1.54	2.06	2.61	3.20	3.88	4.66	5.37	10yr	4.13	5.16	5.93	6.73	7.59	10yr
25yr	0.59	0.95	1.21	1.67	2.22	2.84	25yr	1.92	2.55	3.29	4.04	4.89	5.84	6.79	25yr	5.17	6.53	7.51	8.34	9.36	25yr
50yr	0.68	1.09	1.39	1.95	2.62	3.38	50yr	2.26	2.99	3.91	4.81	5.81	6.93	8.12	50yr	6.13	7.81	8.99	9.81	10.98	50yr
100yr	0.77	1.24	1.60	2.28	3.10	4.02	100yr	2.68	3.52	4.67	5.74	6.92	8.22	9.71	100yr	7.28	9.34	10.77	11.56	12.89	100yr
200yr	0.87	1.43	1.85	2.66	3.67	4.78	200yr	3.17	4.14	5.57	6.84	8.24	9.77	11.62	200yr	8.64	11.18	12.90	13.61	15.13	200yr
500yr	1.05	1.73	2.26	3.28	4.60														and the second second second	18.73	

Lower Confidence Limits

	5min	10min	15min	30min	60min	120min	[1hr	2hr	3hr	6hr	12hr	24hr	48hr		1day	2day	4day	7day	10day	[
1yr	0.29	0.45	0.54	0.73	0.90	1.07	1yr	0.78	1.05	1.23	1.55	1.99	2.12	2.39	1yr	1.88	2.30	2.72	3.25	3.90	1yr
2yr	0.37	0.58	0.71	0.96	1.18	1.41	2yr	1.02	1.38	1.60	2.04	2.57	3.05	3.43	2yr	2.70	3.29	3.74	4.44	5.08	2yr
5yr	0.42	0.64	0.80	1.10	1.40	1.64	5yr	1.21	1.60	1.86	2.40	2.99	3.62	4.10	5yr	3.20	3.95	4.51	5.17	5.98	5yr
10yr	0.46	0.71	0.88	1.23	1.59	1.83	10yr	1.38	1.79	2.08	2.69	3.35	4.10	4.69	10yr	3.63	4.51	5.16	5.78	6.76	10yr
25yr	0.53	0.81	1.01	1.44	1.90	2.11	25yr	1.64	2.06	2.41	3.03	3.90	4.83	5.61	25yr	4.27	5.39	6.15	6.68	7.98	25yr
50yr	0.59	0.90	1.12	1.62	2.18	2,34	50yr	1.88	2.29	2.70	3.39	4.38	5.47	6.41	50yr	4.84	6.17	7.03	7.44	9.07	50yr
100yr	0.67	1.01	1.26	1.83	2.50	2.62	100yr	2.16	2.56	3.04	3.78	4.94	6.18	7.33	100yr	5.47	7.05	8.04	8.24	10.33	100yr
200yr	0.75	1.14	1.44	2.08	2.90	2,92	200yr	2.51	2.85	3.41	4.24	5.56	6.95	8.40	200yr	6.15	8.08	9.20	9.10	11.80	200yr
500yr	0.90	1.33	1.72	2.49	3.55	3.39	500yr	3.06	3.31	4.00	4.94	6.54	8.14	10.03	500yr	7.21	9.65	11.02	10.34	14.10	500yr

Upper Confidence Limits

	5min	10min	15min	30min	60min	120min		1hr	2hr	3hr	6hr	12hr	24hr	48hr		1day	2day	4day	7day	10day	
1yr	0.35	0.55	0.67	0.90	1.11	1.34	1yr	0.95	1.31	1.52	1.94	2.41	2.85	3.25	1yr	2.52	3.13	3.68	4.24	5.04	1yr
2yr	0.40	0.62	0.76	1.03	1.27	1.53	2yr	1.10	1.49	1.73	2,23	2.78	3.30	3.70	2yr	2.92	3.56	4.14	4.83	5.49	2yr
5yr	0.49	0.76	0.94	1.29	1.65	1.95	5yr	1.42	1.91	2.25	2.87	3.63	4.25	4.90	5yr	3.76	4.71	5.42	6.28	7.02	5yr
10yr	0.58	0.90	1.11	1.55	2.01	2.38	10yr	1.73	2.32	2.74	3.52	4.44	5.20	6.05	10yr	4.60	5.81	6.71	7.69	8.49	10yr
25yr	0.73	1.11	1.38	1.97	2.59	3.08	25yr	2.24	3.01	3.58	4.72	5.81	6.78	8.02	25yr	6.00	7.71	8.94	10.08	10.92	25yr
50yr	0.86	1.31	1.63	2.35	3:16	3.76	50yr	2.73	3.68	4.38	5.82	7.11	8.29	9.93	50yr	7.34	9.55	11.12	12.39	13.21	50yr
100yr	1.02	1.55	1.94	2.80	3.84	4.59	100yr	3.31	4.49	5.36	7.19	8.72	10.16	12.31	100yr	8.99	11.83	13.83	15.25	15.97	100yr
200yr	1.21	1.82	2.31	3.34	4.66	5.60	200yr	4.02	5.47	6.57	8,88	10.68	12.45	15.28	200yr	11.02	14.69	17.21	18.80	19.31	200yr
500yr	1.52	2.27	2,92	4.24	6.03	No. of Concession, Name					the second second second second second second second second second second second second second second second s	And the second se	a construction of the second	Address of the second sec		COLUMN TWO IS NOT THE OWNER.		and a standard standards	when we have a second second second	24.81	500yr

Powered by CIS

Northeast Regional Climate Center



Γ

Γ

[]

I

Figure B.2.— Approximate geographic boundaries for SCS referalt distributions.



United States Department of Agriculture

Natural Resources Conservation Service A product of the National Cooperative Soil Survey, a joint effort of the United States Department of Agriculture and other Federal agencies, State agencies including the Agricultural Experiment Stations, and local participants

Custom Soil Resource Report for Orange County, New York

Lands of Zazon Soils Report



Preface

Soil surveys contain information that affects land use planning in survey areas. They highlight soil limitations that affect various land uses and provide information about the properties of the soils in the survey areas. Soil surveys are designed for many different users, including farmers, ranchers, foresters, agronomists, urban planners, community officials, engineers, developers, builders, and home buyers. Also, conservationists, teachers, students, and specialists in recreation, waste disposal, and pollution control can use the surveys to help them understand, protect, or enhance the environment.

Various land use regulations of Federal, State, and local governments may impose special restrictions on land use or land treatment. Soil surveys identify soil properties that are used in making various land use or land treatment decisions. The information is intended to help the land users identify and reduce the effects of soil limitations on various land uses. The landowner or user is responsible for identifying and complying with existing laws and regulations.

Although soil survey information can be used for general farm, local, and wider area planning, onsite investigation is needed to supplement this information in some cases. Examples include soil quality assessments (http://www.nrcs.usda.gov/wps/portal/nrcs/main/soils/health/) and certain conservation and engineering applications. For more detailed information, contact your local USDA Service Center (https://offices.sc.egov.usda.gov/locator/app?agency=nrcs) or your NRCS State Soil Scientist (http://www.nrcs.usda.gov/wps/portal/nrcs/detail/soils/contactus/? cid=nrcs142p2_053951).

Great differences in soil properties can occur within short distances. Some soils are seasonally wet or subject to flooding. Some are too unstable to be used as a foundation for buildings or roads. Clayey or wet soils are poorly suited to use as septic tank absorption fields. A high water table makes a soil poorly suited to basements or underground installations.

The National Cooperative Soil Survey is a joint effort of the United States Department of Agriculture and other Federal agencies, State agencies including the Agricultural Experiment Stations, and local agencies. The Natural Resources Conservation Service (NRCS) has leadership for the Federal part of the National Cooperative Soil Survey.

Information about soils is updated periodically. Updated information is available through the NRCS Web Soil Survey, the site for official soil survey information.

The U.S. Department of Agriculture (USDA) prohibits discrimination in all its programs and activities on the basis of race, color, national origin, age, disability, and where applicable, sex, marital status, familial status, parental status, religion, sexual orientation, genetic information, political beliefs, reprisal, or because all or a part of an individual's income is derived from any public assistance program. (Not all prohibited bases apply to all programs.) Persons with disabilities who require

alternative means for communication of program information (Braille, large print, audiotape, etc.) should contact USDA's TARGET Center at (202) 720-2600 (voice and TDD). To file a complaint of discrimination, write to USDA, Director, Office of Civil Rights, 1400 Independence Avenue, S.W., Washington, D.C. 20250-9410 or call (800) 795-3272 (voice) or (202) 720-6382 (TDD). USDA is an equal opportunity provider and employer.

Contents

Preface	2
How Soil Surveys Are Made	
Soil Map	
Soil Map	
Legend	
Map Unit Legend	11
Map Unit Descriptions	
Orange County, New York	
ESB-Erie extremely stony soils, gently sloping	
MdB—Mardin gravelly silt loam, 3 to 8 percent slopes	14
MdC-Mardin gravelly silt loam, 8 to 15 percent slopes	
SXC—Swartswood and Mardin soils, sloping, very stony	
References	

How Soil Surveys Are Made

Soil surveys are made to provide information about the soils and miscellaneous areas in a specific area. They include a description of the soils and miscellaneous areas and their location on the landscape and tables that show soil properties and limitations affecting various uses. Soil scientists observed the steepness, length, and shape of the slopes; the general pattern of drainage; the kinds of crops and native plants; and the kinds of bedrock. They observed and described many soil profiles. A soil profile is the sequence of natural layers, or horizons, in a soil. The profile extends from the surface down into the unconsolidated material in which the soil formed or from the surface down to bedrock. The unconsolidated material is devoid of roots and other living organisms and has not been changed by other biological activity.

Currently, soils are mapped according to the boundaries of major land resource areas (MLRAs). MLRAs are geographically associated land resource units that share common characteristics related to physiography, geology, climate, water resources, soils, biological resources, and land uses (USDA, 2006). Soil survey areas typically consist of parts of one or more MLRA.

The soils and miscellaneous areas in a survey area occur in an orderly pattern that is related to the geology, landforms, relief, climate, and natural vegetation of the area. Each kind of soil and miscellaneous area is associated with a particular kind of landform or with a segment of the landform. By observing the soils and miscellaneous areas in the survey area and relating their position to specific segments of the landform, a soil scientist develops a concept, or model, of how they were formed. Thus, during mapping, this model enables the soil scientist to predict with a considerable degree of accuracy the kind of soil or miscellaneous area at a specific location on the landscape.

Commonly, individual soils on the landscape merge into one another as their characteristics gradually change. To construct an accurate soil map, however, soil scientists must determine the boundaries between the soils. They can observe only a limited number of soil profiles. Nevertheless, these observations, supplemented by an understanding of the soil-vegetation-landscape relationship, are sufficient to verify predictions of the kinds of soil in an area and to determine the boundaries.

Soil scientists recorded the characteristics of the soil profiles that they studied. They noted soil color, texture, size and shape of soil aggregates, kind and amount of rock fragments, distribution of plant roots, reaction, and other features that enable them to identify soils. After describing the soils in the survey area and determining their properties, the soil scientists assigned the soils to taxonomic classes (units). Taxonomic classes are concepts. Each taxonomic classes has a set of soil characteristics with precisely defined limits. The classes are used as a basis for comparison to classify soils systematically. Soil taxonomy, the system of taxonomic classification used in the United States, is based mainly on the kind and character of soil properties and the arrangement of horizons within the profile. After the soil

scientists classified and named the soils in the survey area, they compared the individual soils with similar soils in the same taxonomic class in other areas so that they could confirm data and assemble additional data based on experience and research.

The objective of soil mapping is not to delineate pure map unit components; the objective is to separate the landscape into landforms or landform segments that have similar use and management requirements. Each map unit is defined by a unique combination of soil components and/or miscellaneous areas in predictable proportions. Some components may be highly contrasting to the other components of the map unit. The presence of minor components in a map unit in no way diminishes the usefulness or accuracy of the data. The delineation of such landforms and landform segments on the map provides sufficient information for the development of resource plans. If intensive use of small areas is planned, onsite investigation is needed to define and locate the soils and miscellaneous areas.

Soil scientists make many field observations in the process of producing a soil map. The frequency of observation is dependent upon several factors, including scale of mapping, intensity of mapping, design of map units, complexity of the landscape, and experience of the soil scientist. Observations are made to test and refine the soil-landscape model and predictions and to verify the classification of the soils at specific locations. Once the soil-landscape model is refined, a significantly smaller number of measurements of individual soil properties are made and recorded. These measurements may include field measurements, such as those for color, depth to bedrock, and texture, and laboratory measurements, such as those for content of sand, silt, clay, salt, and other components. Properties of each soil typically vary from one point to another across the landscape.

Observations for map unit components are aggregated to develop ranges of characteristics for the components. The aggregated values are presented. Direct measurements do not exist for every property presented for every map unit component. Values for some properties are estimated from combinations of other properties.

While a soil survey is in progress, samples of some of the soils in the area generally are collected for laboratory analyses and for engineering tests. Soil scientists interpret the data from these analyses and tests as well as the field-observed characteristics and the soil properties to determine the expected behavior of the soils under different uses. Interpretations for all of the soils are field tested through observation of the soils in different uses and under different levels of management. Some interpretations are modified to fit local conditions, and some new interpretations are developed to meet local needs. Data are assembled from other sources, such as research information, production records, and field experience of specialists. For example, data on crop yields under defined levels of management are assembled from farm records and from field or plot experiments on the same kinds of soil.

Predictions about soil behavior are based not only on soil properties but also on such variables as climate and biological activity. Soil conditions are predictable over long periods of time, but they are not predictable from year to year. For example, soil scientists can predict with a fairly high degree of accuracy that a given soil will have a high water table within certain depths in most years, but they cannot predict that a high water table will always be at a specific level in the soil on a specific date.

After soil scientists located and identified the significant natural bodies of soil in the survey area, they drew the boundaries of these bodies on aerial photographs and

Custom Soil Resource Report

identified each as a specific map unit. Aerial photographs show trees, buildings, fields, roads, and rivers, all of which help in locating boundaries accurately.

Li

Soil Map

The soil map section includes the soil map for the defined area of interest, a list of soil map units on the map and extent of each map unit, and cartographic symbols displayed on the map. Also presented are various metadata about data used to produce the map, and a description of each soil map unit.



Custom Soil Resource Report

Area of Interest (AOI) Soils Soil Map Unit Polygons Soil Map Unit Lines Soil Map Unit	il Area ry Spot / Story Spot / Spot er cial Line Features ams and Canats ams and Canats s ristate Highways Routes
 Area of Interest (AOI) Soil Map Unit Polygons Soil Map Unit Polygons Soil Map Unit Points Marsh or Swater Miscellaneous Water 	ry Spot / Stony Spot Spot arr cial Line Features ams and Canais s ristate Highways Routes
 Soil Map Unit Polygons Soil Map Unit Lines Marsh or Swamp Miscellaneous Water Perential Water 	/ Stony Spot Spot ar cial Line Features ams and Canais arms and Canais fistate Highways Routes
Lines Points ssion Backgrounc	Spot er cial Line Features ams and Canats state Highways Routes
Points ssion mp Background	er cial Line Features ams and Canais s ristate Highways Routes
water Featu Water Featu Transportat Backgrounc	cial Line Features ams and Canais s state Highways Routes
Water Featu ssion mp Background v Water	ams and Canais s rstate Highways Routes
Borrow Pit Clay Spot Closed Depression Gravel Pit Gravelly Spot Landfill Lava Flow Marsh or swamp Mine or Quarry Miscellaneous Water Perennial Water	sams and Canals Please rely on th measurements. ristate Highways Routes Web Soil Survey
Clay Spot Clay Spot Gravel Pit Gravel Vit Gravely Spot Landfill Lava Flow Mine or Quarry Miscellaneous Water Perennial Water	Is Please rely on the measurements. In the measurements is source of Map: Routes Web Soil Survey
Closed Depression Gravel Pit Gravely Spot Landfill Lava Flow Marsh or swamp Mine or Quarry Miscellaneous Water Perennial Water	tate Highways Source of Map: Web Soil Survey
Closed Depression Gravel Pit Gravely Spot Landfill Lava Flow Marsh or swamp Mine or Quarry Miscellaneous Water Perennial Water	lighways Source of Map: Web Soil Survey
Gravel Pit Gravelly Spot Landfill Lava Flow Marsh or swamp Mine or Quarry Miscellaneous Water Perennial Water	Source of Map: Web Soil Survey
Gravelly Spot Landfill Lava Flow Background Marsh or swamp Mine or Quarry Miscellaneous Water Perennial Water	
Landfill Lava Flow Background Marsh or swamp Mine or Quarry Miscellaneous Water Perennial Water	Maior Roads Coordinate System: Web Mercator (EPSG:3857)
Lava Flow Background Marsh or swamp Mine or Quarry Miscellaneous Water Perennial Water	Local Roads Mans from the Web Soil Survey are based on the Web Marrietor
Marsh or swamp Mine or Quarry Miscellaneous Water Perennial Water	
	Aerial Photography Aerial Photography Albers equal-area conic projection, should be used if more
	accurate calculations of distance or area are required.
	This product is generated from the USDA-NRCS certified data as
	of the version date(s) listed below.
Rock Outcrop	Soil Survey Area: Orange County, New York
Saline Spot	
*** Sandy Spot	Soil map units are labeled (as space allows) for map scales
Severely Eroded Spot	1:50,000 or larger.
🔹 Sinkhole	Date(s) aerial imades were nhotomanhed: Oct 7 2013Eeh 26
Slide or Slip	
Sodic Spot	The orthophoto or other base map on which the soil lines were
	compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor

6

l

Map Unit Symbol	Map Unit Name	Acres in AOI	Percent of AOI
ESB	Erie extremely stony soils, gently sloping	3.2	23.7%
MdB	Mardin gravelly silt loam, 3 to 8 percent slopes	5.7	42.6%
MdC	Mardin gravelly silt loam, 8 to 15 percent slopes	2.9	21.6%
SXC	Swartswood and Mardin soils, sloping, very stony	1.6	12.1%
Totals for Area of Interest		13.4	100.0%

Map Unit Legend

Map Unit Descriptions

The map units delineated on the detailed soil maps in a soil survey represent the soils or miscellaneous areas in the survey area. The map unit descriptions, along with the maps, can be used to determine the composition and properties of a unit.

A map unit delineation on a soil map represents an area dominated by one or more major kinds of soil or miscellaneous areas. A map unit is identified and named according to the taxonomic classification of the dominant soils. Within a taxonomic class there are precisely defined limits for the properties of the soils. On the landscape, however, the soils are natural phenomena, and they have the characteristic variability of all natural phenomena. Thus, the range of some observed properties may extend beyond the limits defined for a taxonomic class. Areas of soils of a single taxonomic class rarely, if ever, can be mapped without including areas of other taxonomic classes. Consequently, every map unit is made up of the soils or miscellaneous areas for which it is named and some minor components that belong to taxonomic classes other than those of the major soils.

Most minor soils have properties similar to those of the dominant soil or soils in the map unit, and thus they do not affect use and management. These are called noncontrasting, or similar, components. They may or may not be mentioned in a particular map unit description. Other minor components, however, have properties and behavioral characteristics divergent enough to affect use or to require different management. These are called contrasting, or dissimilar, components. They generally are in small areas and could not be mapped separately because of the scale used. Some small areas of strongly contrasting soils or miscellaneous areas are identified by a special symbol on the maps. If included in the database for a given area, the contrasting minor components are identified in the map unit descriptions along with some characteristics of each. A few areas of minor components may not have been observed, and consequently they are not mentioned in the descriptions, especially where the pattern was so complex that it was impractical to make enough observations to identify all the soils and miscellaneous areas on the landscape.

The presence of minor components in a map unit in no way diminishes the usefulness or accuracy of the data. The objective of mapping is not to delineate pure taxonomic classes but rather to separate the landscape into landforms or landform segments that have similar use and management requirements. The delineation of such segments on the map provides sufficient information for the development of resource plans. If intensive use of small areas is planned, however, onsite investigation is needed to define and locate the soils and miscellaneous areas.

An identifying symbol precedes the map unit name in the map unit descriptions. Each description includes general facts about the unit and gives important soil properties and qualities.

Soils that have profiles that are almost alike make up a *soil series*. Except for differences in texture of the surface layer, all the soils of a series have major horizons that are similar in composition, thickness, and arrangement.

Soils of one series can differ in texture of the surface layer, slope, stoniness, salinity, degree of erosion, and other characteristics that affect their use. On the basis of such differences, a soil series is divided into *soil phases*. Most of the areas shown on the detailed soil maps are phases of soil series. The name of a soil phase commonly indicates a feature that affects use or management. For example, Alpha silt loam, 0 to 2 percent slopes, is a phase of the Alpha series.

Some map units are made up of two or more major soils or miscellaneous areas. These map units are complexes, associations, or undifferentiated groups.

A *complex* consists of two or more soils or miscellaneous areas in such an intricate pattern or in such small areas that they cannot be shown separately on the maps. The pattern and proportion of the soils or miscellaneous areas are somewhat similar in all areas. Alpha-Beta complex, 0 to 6 percent slopes, is an example.

An association is made up of two or more geographically associated soils or miscellaneous areas that are shown as one unit on the maps. Because of present or anticipated uses of the map units in the survey area, it was not considered practical or necessary to map the soils or miscellaneous areas separately. The pattern and relative proportion of the soils or miscellaneous areas are somewhat similar. Alpha-Beta association, 0 to 2 percent slopes, is an example.

An *undifferentiated group* is made up of two or more soils or miscellaneous areas that could be mapped individually but are mapped as one unit because similar interpretations can be made for use and management. The pattern and proportion of the soils or miscellaneous areas in a mapped area are not uniform. An area can be made up of only one of the major soils or miscellaneous areas, or it can be made up of all of them. Alpha and Beta soils, 0 to 2 percent slopes, is an example.

Some surveys include *miscellaneous areas*. Such areas have little or no soil material and support little or no vegetation. Rock outcrop is an example.

Orange County, New York

ESB—Erie extremely stony soils, gently sloping

Map Unit Setting

National map unit symbol: 9vvb Elevation: 180 to 1,460 feet Mean annual precipitation: 42 to 52 inches Mean annual air temperature: 46 to 52 degrees F Frost-free period: 135 to 215 days Farmland classification: Not prime farmland

Map Unit Composition

Erie, extremely stony, and similar soils: 80 percent *Minor components:* 20 percent *Estimates are based on observations, descriptions, and transects of the mapunit.*

Description of Erie, Extremely Stony

Setting

Landform: Drumlinoid ridges, hills, till plains Landform position (two-dimensional): Footslope, summit Landform position (three-dimensional): Base slope Down-slope shape: Concave Across-slope shape: Linear Parent material: Loamy till derived from siltstone, sandstone, shale, and limestone

Typical profile

H1 - 0 to 4 inches: gravely silt loam

H2 - 4 to 18 inches: channery silt loam

H3 - 18 to 50 inches: channery silt loam

H4 - 50 to 70 inches: channery silt loam

Properties and qualities

Slope: 3 to 8 percent

Surface area covered with cobbles, stones or boulders: 9.0 percent Depth to restrictive feature: 10 to 21 inches to fragipan Drainage class: Somewhat poorly drained Capacity of the most limiting layer to transmit water (Ksat): Moderately low to

moderately high (0.06 to 0.20 in/hr)

Depth to water table: About 6 to 18 inches

Frequency of flooding: None

Frequency of ponding: None

Calcium carbonate, maximum content: 15 percent Available water capacity: Very low (about 2.4 inches)

Interpretive groups

Land capability classification (irrigated): None specified Land capability classification (nonirrigated): 7s Hydrologic Soil Group: D Ecological site: F144AY037MA - Moist Dense Till Uplands Hydric soil rating: No

Minor Components

Mardin

Percent of map unit: 5 percent Hydric soil rating: No

Alden

Percent of map unit: 5 percent Landform: Depressions Hydric soil rating: Yes

Wurtsboro

Percent of map unit: 5 percent Hydric soil rating: No

Bath

Percent of map unit: 5 percent Hydric soil rating: No

MdB—Mardin gravelly silt loam, 3 to 8 percent slopes

Map Unit Setting

National map unit symbol: 2v30j Elevation: 330 to 2,460 feet Mean annual precipitation: 31 to 70 inches Mean annual air temperature: 39 to 52 degrees F Frost-free period: 105 to 180 days Farmland classification: Farmland of statewide importance

Map Unit Composition

Mardin and similar soils: 85 percent Minor components: 15 percent Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Mardin

Setting

Landform: Hills, mountains Landform position (two-dimensional): Summit, shoulder Landform position (three-dimensional): Interfluve, side slope Down-slope shape: Convex Across-slope shape: Convex Parent material: Loamy till

Typical profile

Ap - 0 to 8 inches: gravelly silt loam Bw - 8 to 15 inches: gravelly silt loam E - 15 to 20 inches: gravelly silt loam Bx - 20 to 72 inches: gravelly silt loam

Properties and qualities

Slope: 3 to 8 percent

Surface area covered with cobbles, stones or boulders: 0.0 percent Depth to restrictive feature: 14 to 26 inches to fragipan Drainage class: Moderately well drained Capacity of the most limiting layer to transmit water (Ksat): Very low to moderately low (0.00 to 0.14 in/hr) Depth to water table: About 13 to 24 inches Frequency of flooding: None

Frequency of ponding: None

Available water capacity: Low (about 3.6 inches)

Interpretive groups

Land capability classification (irrigated): None specified Land capability classification (nonirrigated): 2w Hydrologic Soil Group: D Ecological site: F144AY008CT - Moist Till Uplands Hydric soil rating: No

Minor Components

Volusia

Percent of map unit: 5 percent Landform: Hills, mountains Landform position (two-dimensional): Footslope, summit Landform position (three-dimensional): Base slope, interfluve, side slope Down-slope shape: Concave Across-slope shape: Linear Hydric soil rating: No

Bath

Percent of map unit: 5 percent Landform: Hills, mountains Landform position (two-dimensional): Backslope, shoulder Landform position (three-dimensional): Interfluve, side slope Down-slope shape: Concave Across-slope shape: Linear Hydric soil rating: No

Lordstown

Percent of map unit: 5 percent Landform: Mountains, hills Landform position (two-dimensional): Summit, shoulder Landform position (three-dimensional): Mountaintop, interfluve, crest Down-slope shape: Convex Across-slope shape: Convex Hydric soil rating: No

MdC—Mardin gravelly silt loam, 8 to 15 percent slopes

Map Unit Setting

National map unit symbol: 2v30l Elevation: 330 to 2,460 feet Mean annual precipitation: 31 to 70 inches Mean annual air temperature: 39 to 52 degrees F Frost-free period: 105 to 180 days Farmland classification: Farmland of statewide importance

Map Unit Composition

Mardin and similar soils: 85 percent Minor components: 15 percent Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Mardin

Setting

Landform: Hills, mountains Landform position (two-dimensional): Shoulder, backslope Landform position (three-dimensional): Interfluve, side slope Down-slope shape: Linear Across-slope shape: Linear Parent material: Loamy till

Typical profile

Ap - 0 to 8 inches: gravelly silt loam Bw - 8 to 15 inches: gravelly silt loam E - 15 to 20 inches: gravely silt loam Bx - 20 to 72 inches: gravelly silt loam

Properties and qualities

Slope: 8 to 15 percent Surface area covered with cobbles, stones or boulders: 0.0 percent Depth to restrictive feature: 14 to 26 inches to fragipan Drainage class: Moderately well drained Capacity of the most limiting layer to transmit water (Ksat): Very low to moderately low (0.00 to 0.14 in/hr) Depth to water table: About 13 to 24 inches Frequency of flooding: None

Frequency of ponding: None

Available water capacity: Low (about 3.6 inches)

Interpretive groups

Land capability classification (irrigated): None specified Land capability classification (nonirrigated): 3e Hydrologic Soil Group: D Ecological site: F144AY008CT - Moist Till Uplands Hydric soil rating: No

Minor Components

Volusia

Percent of map unit: 5 percent Landform: Hills, mountains Landform position (two-dimensional): Footslope, summit Landform position (three-dimensional): Base slope, interfluve, side slope Down-slope shape: Concave Across-slope shape: Linear Hydric soil rating: No

Lordstown

Percent of map unit: 5 percent Landform: Mountains, hills Landform position (two-dimensional): Backslope Landform position (three-dimensional): Mountainflank, side slope, nose slope Down-slope shape: Linear Across-slope shape: Linear Hydric soil rating: No

Bath

Percent of map unit: 5 percent Landform: Hills, mountains Landform position (two-dimensional): Backslope Landform position (three-dimensional): Nose slope, side slope Down-slope shape: Linear Across-slope shape: Linear Hydric soil rating: No

SXC-Swartswood and Mardin soils, sloping, very stony

Map Unit Setting

National map unit symbol: 2v30r Elevation: 330 to 2,460 feet Mean annual precipitation: 31 to 70 inches Mean annual air temperature: 39 to 52 degrees F Frost-free period: 105 to 180 days Farmland classification: Not prime farmland

Map Unit Composition

Swartswood, very stony, and similar soils: 41 percent Mardin, very stony, and similar soils: 39 percent Minor components: 20 percent Estimates are based on observations, descriptions, and transects of the mapunit.

Description of Swartswood, Very Stony

Setting

Landform: Hills, till plains

Landform position (two-dimensional): Shoulder

Landform position (three-dimensional): Crest

Down-slope shape: Convex

Across-slope shape: Convex

Parent material: Loamy till derived mainly from quartzite, conglomerate, and sandstone

Typical profile

H1 - 0 to 3 inches: gravelly loam

H2 - 3 to 31 inches: gravelly fine sandy loam

H3 - 31 to 60 inches: gravelly fine sandy loam

Properties and qualities

Slope: 8 to 15 percent

Surface area covered with cobbles, stones or boulders: 1.6 percent Depth to restrictive feature: 20 to 36 inches to fragipan

Drainage class: Well drained

Capacity of the most limiting layer to transmit water (Ksat): Moderately low to moderately high (0.06 to 0.57 in/hr)

Depth to water table: About 23 to 31 inches

Frequency of flooding: None

Frequency of ponding: None

Available water capacity: Low (about 3.1 inches)

Interpretive groups

Land capability classification (irrigated): None specified Land capability classification (nonirrigated): 6s Hydrologic Soil Group: C Ecological site: F140XY030NY - Well Drained Dense Till Hydric soil rating: No

Description of Mardin, Very Stony

Setting

Landform: Hills, mountains Landform position (two-dimensional): Shoulder, backslope Landform position (three-dimensional): Interfluve, side slope Down-slope shape: Linear Across-slope shape: Linear Parent material: Loamy till

Typical profile

A - 0 to 4 inches: gravelly silt loam Bw - 4 to 15 inches: gravelly silt loam E - 15 to 20 inches: gravelly silt loam Bx - 20 to 72 inches: gravelly silt loam

Properties and qualities

Slope: 8 to 15 percent
Surface area covered with cobbles, stones or boulders: 1.6 percent
Depth to restrictive feature: 14 to 26 inches to fragipan
Drainage class: Moderately well drained
Capacity of the most limiting layer to transmit water (Ksat): Very low to moderately low (0.00 to 0.14 in/hr)
Depth to water table: About 13 to 24 inches
Frequency of flooding: None
Frequency of ponding: None
Available water capacity: Low (about 3.6 inches)

Interpretive groups

Land capability classification (irrigated): None specified Land capability classification (nonirrigated): 6s Hydrologic Soil Group: D Ecological site: F144AY008CT - Moist Till Uplands Hydric soil rating: No

Minor Components

Volusia, very stony

Percent of map unit: 5 percent Landform: Hills, mountains Landform position (two-dimensional): Footslope, summit Landform position (three-dimensional): Base slope, interfluve, side slope Down-slope shape: Concave Across-slope shape: Linear Hydric soil rating: No

Bath, very stony

Percent of map unit: 5 percent Landform: Hills, mountains Landform position (two-dimensional): Backslope Landform position (three-dimensional): Nose slope, side slope Down-slope shape: Linear Across-slope shape: Linear Hydric soil rating: No

Lordstown

Percent of map unit: 5 percent Landform: Mountains, hills Landform position (two-dimensional): Backslope Landform position (three-dimensional): Mountainflank, nose slope, side slope Down-slope shape: Linear Across-slope shape: Linear Hydric soil rating: No

Wurtsboro, very stony

Percent of map unit: 5 percent Landform: Hills, till plains Landform position (two-dimensional): Summit Landform position (three-dimensional): Crest Down-slope shape: Concave Across-slope shape: Convex Hydric soil rating: No

References

American Association of State Highway and Transportation Officials (AASHTO). 2004. Standard specifications for transportation materials and methods of sampling and testing. 24th edition.

American Society for Testing and Materials (ASTM). 2005. Standard classification of soils for engineering purposes. ASTM Standard D2487-00.

Cowardin, L.M., V. Carter, F.C. Golet, and E.T. LaRoe. 1979. Classification of wetlands and deep-water habitats of the United States. U.S. Fish and Wildlife Service FWS/OBS-79/31.

Federal Register. July 13, 1994. Changes in hydric soils of the United States.

Federal Register. September 18, 2002. Hydric soils of the United States.

Hurt, G.W., and L.M. Vasilas, editors. Version 6.0, 2006. Field indicators of hydric soils in the United States.

National Research Council. 1995. Wetlands: Characteristics and boundaries.

Soil Survey Division Staff. 1993. Soil survey manual. Soil Conservation Service. U.S. Department of Agriculture Handbook 18. http://www.nrcs.usda.gov/wps/portal/ nrcs/detail/national/soils/?cid=nrcs142p2_054262

Soil Survey Staff. 1999. Soil taxonomy: A basic system of soil classification for making and interpreting soil surveys. 2nd edition. Natural Resources Conservation Service, U.S. Department of Agriculture Handbook 436. http://www.nrcs.usda.gov/wps/portal/nrcs/detail/national/soils/?cid=nrcs142p2_053577

Soil Survey Staff. 2010. Keys to soil taxonomy. 11th edition. U.S. Department of Agriculture, Natural Resources Conservation Service. http://www.nrcs.usda.gov/wps/portal/nrcs/detail/national/soils/?cid=nrcs142p2 053580

Tiner, R.W., Jr. 1985. Wetlands of Delaware. U.S. Fish and Wildlife Service and Delaware Department of Natural Resources and Environmental Control, Wetlands Section.

United States Army Corps of Engineers, Environmental Laboratory. 1987. Corps of Engineers wetlands delineation manual. Waterways Experiment Station Technical Report Y-87-1.

United States Department of Agriculture, Natural Resources Conservation Service. National forestry manual. http://www.nrcs.usda.gov/wps/portal/nrcs/detail/soils/ home/?cid=nrcs142p2_053374

United States Department of Agriculture, Natural Resources Conservation Service. National range and pasture handbook. http://www.nrcs.usda.gov/wps/portal/nrcs/ detail/national/landuse/rangepasture/?cid=stelprdb1043084

United States Department of Agriculture, Natural Resources Conservation Service. National soil survey handbook, title 430-VI. http://www.nrcs.usda.gov/wps/portal/nrcs/detail/soils/scientists/?cid=nrcs142p2_054242

United States Department of Agriculture, Natural Resources Conservation Service. 2006. Land resource regions and major land resource areas of the United States, the Caribbean, and the Pacific Basin. U.S. Department of Agriculture Handbook 296. http://www.nrcs.usda.gov/wps/portal/nrcs/detail/national/soils/? cid=nrcs142p2_053624

United States Department of Agriculture, Soil Conservation Service. 1961. Land capability classification. U.S. Department of Agriculture Handbook 210. http://www.nrcs.usda.gov/Internet/FSE_DOCUMENTS/nrcs142p2_052290.pdf

APPENDIX 8

ſ

L

Stormwater Quality and Runoff Reduction – Calculations & Supporting Data

Water Quality Volume (WQ_v) Calculation for Entire Project Drainage Area

Utilize 90% Rule:

 $WQ_{v} = [(P)(R_{v})(A)] / 12$

WQ_v = Water Quality Volume (acre-feet) R_v = 0.05 + 0.009 (I) I = Impervious Cover (Percent) P = 90% Rainfall Event Number = 1.39 inches A = Drainage Area in acres

Calculate Impervious Cover (%):

Drainage Area (A) = 13.56 acres New Impervious area within Site Area = 1.49 acres

Impervious Cover (I) = 11.0 %

Calculate Volumetric Runoff Coefficient (R_v) :

 $R_v = 0.05 + 0.009 (I)$ $R_v = 0.15$

Use $R_v \rightarrow 0.15$

90% Rainfall Event Number Utilized:

P = 1.39 inches

Calculate Water Quality Volume:

$$WQ_v = [(P)(R_v)(A)] / 12$$

 $WQ_v = 0.234 \text{ acre-fect}$ $= 10187 \text{ ft}^3$

Minimum Runoff Reduction Volume (RRv) Calculation

 $RRv = [(P)(R_v)(Ai)] / 12$

RRv = Runoff Reduction Volume (acre-feet) $R_v = 0.05 + 0.009 (I)$

(Where I = 100%)

I = Impervious Cover (Percent)

P = 90% Rainfall Event Number = 1.39 inches Ai = Impervious Cover Targeted for Runoff Reduction = (S) (Aic) Aic = Total Area of New Impervious Cover S = Hydrologic Soil Group (HSG) Specific Reduction Factor S for HSG A =0.55 S for HSG B =0.40 S for HSG C =0.30

S for HSG D =0.20

Calculate Specific Reduction Factor (S)

Total Drainage Area (A) =	13.56	acres
Total Area of HSG A	0.00	acres
Total Area of HSG B	0.00	acres
Total Area of HSG C	1.60	acres
Total Area of HSG D	11.96	acres

S = [(HSG A)(0.55) + (HSG B)(0.40) + (HSG C)(0.30) + (HSG D)(0.20)] / AS = 0.2118

Calculate Impervious Cover Targeted for Runoff Reduction (Ai)

Ai = (S) (Aic)Aic = Total Area of New Impervious Cover = 1.49 acres

Ai = 0.32 acres

Calculate Volumetric Runoff Coefficient (R_v):

 $R_v = 0.05 + 0.009 (I)$ $R_v = 0.95$

90% Rainfall Event Number Utilized:

P = 1.39 inches

Calculate Minimum Runoff Reduction Volume:

$RRv = [(P)(R_v)(Ai)] / 12$

RRv = 0.035 acre-feet RRv = 1513 ft³

l.,

Ł

L

Ł

L

Rain Garden Design

Step 1: Calculate the Water Quality Volume (WQ_v)

$$WQ_v = [(P)(R_v)(A)] / 12$$

$$\begin{split} & WQ_v = \text{Water Quality Volume (acre-feet)} \\ & P = 90\% \text{ Rainfall Event Number} = 1.39 \text{ inches} \\ & R_v = 0.05 + 0.009 \text{ (I)} = 0.05 + 0.009 (100) = 0.95 \\ & I = \text{Impervious Cover (Percent)} = 100\% \\ & A = \text{Drainage Area} = 750 \quad \text{ft}^2 \end{split}$$

 $WQ_v = 83 \text{ ft}^3$

Step 2: Solve for Drainage Layer and Soil Media Storage Volume:

 $V_{SM} = A_{RG} \times D_{SM} \times P_{SM}$ $V_{DL} = A_{RG} \times D_{DL} \times P_{DL}$

A_{RG} = Proposed Rain Garden Surface Area =	200	ft^2
$D_{SM} = Depth of Soil Media =$	1.0	ft
D_{DL} = Depth of Drainage Layer =	0.5	ft
P_{SM} = Porosity of Soil Media =	0.20	
$P_{DL} =$ Porosity of Drainage Layer =	0.40	

$V_{SM} =$	40	fť
$V_{DL} =$	40	ft^3

Step 3: Calculate the Provided Water Quality:

$$WQ_{v} = V_{SM} + V_{DL} + (D_{P} \times A_{RG})$$
$$D_{P} = Ponding Depth = 0.50 \text{ ft}$$
$$WQ_{v} = 180 \text{ ft}^{3}$$

180 ft³ is added to the Runoff Reduction Volume (RRv)

Total Runoff Reduction Volume (RRv) Contributed by Rain Gardens on Lots 1, 2, 3, 4, 5, 6, 7, 8, and 10

Total RRv = 40% of the sum of Rain Garden WQ_v

=

Total Rain Garden $RRv = 0.40 \times (180 \text{ ft}^3 \times 18)$

 1296 ft^3

Total Rain Garden RRv = 0.030 acre-feet

Remaining Required RRv:

Previous Calculated WQv = 0.035 acre-feet

Remaining RRv = Previous Calculated WQv - Rain Garden RRv

Remaining RRv =	0.005	acre-feet	
	229	ft ³	

Bioretention Basin Design (Pond 3P)

Step 1: Calculate the Water Quality Volume (WQ_v)

 $WQ_v = [(P)(R_v)(A)] / 12$

$$\begin{split} WQ_v &= \text{Water Quality Volume (acre-feet)} \\ P &= 90\% \text{ Rainfall Event Number} = 1.39 \text{ inches} \\ R_v &= 0.05 + 0.009 \text{ (I)} \\ I &= \text{Impervious Cover (Percent)} = 27.1 \% \\ A &= \text{Drainage Area} = 3.9 \text{ acres} \end{split}$$

 $WQ_v = 0.13$ ac-ft $WQ_v = 5738.94$ ft³

Step 2: Determine size of bioretention filter area:

 $A_{f} = (WQ_{v}) (d_{f}) / [(k) (h_{f} + d_{f}) (t_{f})]$

$d_f = filter bed depth =$	2.5	ft
k = coefficient of permeability of filter media =	0.5	ft/day
h_f = average height of water above filter bed =	0.5	ft
$t_f = design filter bed drain time =$	2.00	days

A_f (Required) =	4782.45
A_{f} (Provided) =	5940.00

Step 3: Determine size of pretreatment:

Pretreatment size = $1/4$ of the WQ _v =	1435	ft^3
Pretreatment Provided =	184 1	ft ³ Sedimentation Basin

Step 4: Determine size of underdrain area:

Underdrain size = $(10\% \text{ of } A_f)/3'$ zone of influence =	159	ft
Provided =	165	ft

Step 4: Calculate Stream Channel Protection Volume (Cp_v):

Stream Channel Protection Volume (Cp_v) Calculated using HydroCAD Software:

 $Cp_v = 0.305$ acre-feet

Step 6: Calculate Stream Channel Protection Volume (Cp_v) Release Rate: Release Cp_v over a 24 hour period:

 $(Cp_v \text{ acre-feet } * 43560 \text{ ft}^2 / \text{ acre}) / (24 \text{ hours } * 3600 \text{ sec } / \text{ hour})$

Required Release Rate =0.15 ft^3 / secProvided Release Rate =0.14 ft^3 / sec

Lands of Zazon

Total Runoff Reduction Volume (RRv) Contributed by Bioretention Utilization:

Total RRv = 40% of the WQ_v provided by the practice (HSG C & D)

Total Bioretention RRv =	0.065	acre-feet	
Total Bioretention RRv:	2851.20	ft ³	
Bioretention WQv Required= Bioretention WQv Provided=	5738.94 7128.00	ft ³ ft ³	

Runoff Reduction Volume (RRv) Summary:

Total RRv Calculated =

Total RRv Required per Calculation =	10,187 ft ³
Minimum RRv Required per Calculation =	1,513 ft ³
RRv Provided Utilizing Rain Gardens = RRv Provided Utilizing Bioretention Pond = Total RRv Provided =	1296 ft ³ 2,851 ft ³ 4,147 ft ³

:. Meets Minimum RRv Required, Utilized SMP for remaining RRv:

Remaining Required RRv:

Total RRv Required - RRv Provided Utilizing $GI = 6,040 \text{ ft}^3$

APPENDIX 9

Ì

 $\left[\right]$

 $\left[\right]$

L

Soil Restoration

. . · · · .

New York State Stormwater Management Design Manual

Chapter 5: Green Infrastructure Practices

Section 5.1 Planning for Green Infrastructure: Preservation of Natural Features and Conservation Design

5.1.6 Soil Restoration

Description

Soil Restoration is a required practice applied across areas of a development site where soils have been disturbed and will be vegetated in order to recover the original properties and porosity of the soil. Healthy soil is vital to a sustainable environment and landscape. A deep, well drained soil, rich in organic matter, absorbs rainwater, helps prevent flooding and soil erosion, filters out water pollutants, and promotes vigorous plant growth that requires less irrigation, pesticides, and fertilizer.

Soil Restoration is applied in the cleanup, restoration, and landscaping phase of construction followed by the permanent establishment of an appropriate, deep-rooted groundcover to help maintain the restored soil structure. Soil restoration includes mechanical decompaction, compost amendment, or both.

Many runoff reduction practices need Soil Restoration measures applied over and adjacent to the practice to achieve runoff reduction performance. (See typical compacted soil in Figure 5.15). Consult individual profile sheets for specific design criteria.

Key Benefits

- More marketable buildings and landscapes
- Less stormwater runoff, better water quality
- Healthier, aesthetically pleasing landscapes
- Increased porosity on redevelopment sites where impervious cover is converted to pervious
- Achieves performance standards on runoff reduction practices
- Decreases runoff volume generated and lowers the demand on runoff control structures
- Enhances direct groundwater recharge
- Promotes successful long-term revegetation by restoring soil organic matter, permeability, drainage and water holding capacity for healthy root system development of trees, shrubs and deep-rooted ground covers, minimizing lawn chemical requirements, plant drowning during wet periods, and burnout during dry periods

Typical Perceived Obstacles and Realities





New York State Stormwater Management Design Manual

Chapter 5: Green Infrastructure PracticesSection 5.1 Planning for Green Infrastructure: Preservation of Natural Features and Conservation Design

- Higher cost due to soil restoration- application of soil de-compaction and enhancement may have additional initial cost; however, they provide benefit in reducing the need for conveyance structures.
- Space constraints and obstruction for use of equipment post construction space may limit the ability of some of the de-compaction equipment, however, alternative equipment and sensible planning help overcome this obstacle.

Discussion

Tilling exposes compacted soil devoid of oxygen to air and recreates temporary air space. In addition, research has shown that the incorporation of organic compost, can greatly improve temporary water storage in the soil and subsequent runoff reduction through infiltration and evapotranspiration.

Soils that have a permanent high water table close to the surface (0-12 inches), either influenced by a clay or other highly impervious layer of material, may have bulk densities so naturally high that compaction has little added impact on infiltration (Lacey 2008). However, these soils will still benefit from the addition of compost. The water holding capacity, penetration, structural stability, and fertility of clay soils were improved with compost mixing (Avnimelech and Cohen 1988).

Table 5.3 describes various soil disturbance activities related to land development, soil types and the requirements for soil restoration for each activity. Soil Restoration or modification of curve numbers is a <u>required</u> practice. Restoration is applied across areas of a development site where soils have been compacted and will be vegetated according to the criteria defined in Table 5.3. If Soil Restoration is not applied according to these criteria, designers are required to:

- a) Increase the calculated WQv by factoring in the compacted areas that have not been kept as impervious cover (including areas of cut or fill, heavy traffic areas on site, or Impervious Cover reduction in redevelopment projects unless aeration or full soil restoration is applied, per Table 5.3).
- b) Change by one level the post-construction hydrologic soil group (HSG) to a less permeable group than the original condition. This is applied to all volumetric and discharge rate control computations.

L
Chapter 5: Green Infrastructure Practices

Section 5.1 Planning for Green Infrastructure: Preservation of Natural Features and Conservation Design

	Table 5.3 S	oil Restoration Requiren	nents
Type of Soil Disturbance	Soil Restora	ation Requirement	Comments/Examples
No soil disturbance	Restoration not	permitted	Preservation of Natural Features
Minimal soil disturbance	Restoration not	required	Clearing and grubbing
Areas where topsoil is	HSG A &B	HSG C&D	Protect area from any ongoing
stripped only - no change in grade	apply 6 inches of topsoil	Aerate* and apply 6 inches of topsoil	construction activities.
	HSG A &B	HSG C & D	
Areas of cut or fill	Aerate and apply 6 inches of topsoil	Apply full Soil Restoration **	
Heavy traffic areas on site (especially in a zone 5-25 feet around buildings but not within a 5 foot perimeter around foundation walls)	Apply full Soil compaction and enhancement)	Restoration (de- l compost	
Areas where Runoff Reduction and/or Infiltration practices are applied	applied to enhai	required, but may be nee the reduction propriate practices.	Keep construction equipment from crossing these areas. To protect newly installed practice from any ongoing construction activities construct a single phase operation fence area
Redevelopment projects		projects in areas impervious area will	

*Aeration includes the use of machines such as tractor-drawn implements with coulters making a narrow slit in the soil, a roller with many spikes making indentations in the soil, or prongs which function like a mini-subsoiler.

** Per "Deep Ripping and De-compaction, DEC 2008".

Using this Practice

During periods of relatively low to moderate subsoil moisture, the disturbed subsoils are returned to rough grade and the following Soil Restoration steps applied:

1) Apply 3 inches of compost over subsoil

Chapter 5: Green Infrastructure Practices

Section 5.1 Planning for Green Infrastructure: Preservation of Natural Features and Conservation Design

- Till compost into subsoil to a depth of at least 12 inches using a cat-mounted ripper, tractor-mounted disc, or tiller, mixing, and circulating air and compost into subsoils
- Rock-pick until uplifted stone/rock materials of four inches and larger size are cleaned off the site
- 4) Apply topsoil to a depth of 6 inches
- 5) Vegetate as required by approved plan.

At the end of the project an inspector should be able to push a 3/8" metal bar 12 inches into the

Figure 5.15 Soil aerator implement

soil just with body weight. Figures 5.16 and 5.17 show two attachments used for soil decompaction. Tilling (step 2 above) should not be performed within the drip line of any existing trees or over utility installations that are within 24 inches of the surface.

COMPOST SPECIFICATIONS

Compost shall be aged, from plant derived materials, free of viable weed seeds, have no visible free water or dust produced when handling, pass through a half inch screen and have a pH suitable to grow desired plants.

Maintenance

A simple maintenance agreement should identify where Soil Restoration is applied, where newly restored areas are/cannot be cleared, who the responsible parties are to ensure that routine vegetation improvements

are made (i.e., thinning, invasive plant removal, etc.). Soil compost amendments within a filter strip or grass channel should be located in public right of way, or within a dedicated stormwater or drainage easement.

First year maintenance operations includes:

• Initial inspections for the first six months (once after each storm greater than half- inch)



Figure 5.16 Soil aerator implement

Chapter 5: Green Infrastructure Practices

Section 5.1 Planning for Green Infrastructure: Preservation of Natural Features and Conservation Design

- Reseeding to repair bare or eroding areas to assure grass stabilization
- Water once every three days for first month, and then provide a half inch of water per week during first year. Irrigation plan may be adjusted according to the rain event.
- Fertilization may be needed in the fall after the first growing season to increase plant vigor
- Ongoing Maintenance:

Two points help ensure lasting results of decompaction:

- 1) Planting the appropriate ground cover with deep roots to maintain the soil structure
- 2) Keeping the site free of vehicular and foot traffic or other weight loads. Consider pedestrian footpaths. (Sometimes it may be necessary to de-thatch the turf every few years)

References/Further Resources

- Avnimelech, Y. and M. Kochva, Y. Yotal, D. Shkedy. 1988. THE USE OF COMPOST AS A SOIL AMENDMENT
- Balusek. 2003. Quantifying decreases in stormwater runoff from deep-tilling, chisel-planting and *compost amendments*. Dane County Land Conservation Department. Madison, Wisconsin. http://www.countyofdane.com/lwrd/landconservation/papers/quantifyingdecreasesinswrunoff.pdf
- Chollak, T. and P. Rosenfeld. 1998. Guidelines for Landscaping with Compost-Amended Soils City of Redmond Public Works. http://www.ci.redmond.wa.us/insidecityhall/publicworks/environment/pdfs/compostamendedsoils. pdf
- City of Portland. 2008. Soil Specification for Vegetated Stormwater Facilities. Portland Stormwater Management Manual. Portland, Oregon
- Composting Council (TCC). 1997. Development of a landscape architect specification for *compost utilization*. Alexandria, VA. http://www.cwc.org/organics/org972rpt.pdf
- DRAFT VA DCR STORMWATER DESIGN SPECIFICATION No. 4, SOIL COMPOST AMENDMENT, VERSION 1.5 June 22, 2009
- http://www.chesapeakestormwater.net/storage/first-draft-baywide-designspecificationsi/BAYWIDE%20No%204%20SOIL%20AMENDMENT%20SPECIFICATION.pdf
- Holman-Dodds, L. 2004. Chapter 6. Assessing infiltration-based stormwater practices. PhD Dissertation. Department of Hydroscience and Engineering. University of Iowa. Iowa City, IA.
- King County Department of Development & Environmental Services, Achieving the Post-construction Soil Standard, January 1, 2005. http://www.metrokc.gov/DDES/forms/ls-inf-SoilPost-ConStd.pdf

5-21

Chapter 5: Green Infrastructure Practices

Section 5.1 Planning for Green Infrastructure: Preservation of Natural Features and Conservation Design

- Lacey, John. 2008. NYSDEC Deep-Ripping and Decompaction, *Guidelines for Infiltration and Decompaction*, New York State Department of Environmental Conservation.
- Lenhart, J. 2007. Compost as a soil amendment for water quality treatment facilities. Proceedings 2007 LID Conference. Wilmington, NC
- Low Impact Development Center. *Guideline for Soil Amendments*. http://www.lowimpactdevelopment.org/epa03/soilamend.htm

NYS Dept. of Ag & Markets http://www.agmkt.state.ny.us/AP/agservices/constructG8.html

- Roa-Espinosa. 2006. An introduction to soil compaction and the subsoiling practice. technical note. Dane County Land Conservation Department. Madison, Wisconsin.
- Schueler, T. 2000. "The Compaction of Urban Soils" The Practice of Watershed Protection. P. 210-214. Center for Watershed Protection
- SERAIEG, Southern Extension and Research Activity Information Exchange Group, Interpreting Soil Organic Matter Tests, (2005). http://www.clemson.edu/agsrvlb/sera6/SERA6-ORGANIC_doc.pdf
- Soils for Salmon. 2003. Soil Restoration and compost amendments. http://www.soilsforsalmon.org/pdf/SoilsforSalmonLIDrev9-16-04.pdf

US Composting Council, www.compostingcouncil.org

APPENDIX 10

[

Γ

 $\left[\right]$

.

Ì.,

L

نيا ا

Outlet Protection Calculations



New York Standards and Specifications For Erosion and Sediment Control

APPENDIX 11

Lange and the second se

 $\left[\right]$

Pipe Sizing Calculations

						POND OUTLET		FROM	PIPE RUN				ASSUMED VALUES				
RD. CULVERT	CULVERT	DRIVEWAY 4 CULVERT	DRIVEWAY 13 CULVERT	CULVERT		SWALE		σ			KAINE	TIME OF CO					FROF USED CONDITIONS OFF SITE COLVERTS
	1	1		•	0.60	I	AREA	PERVIOUS	CONTRIBUTING AREAS (ac)		KAINFALL INTENSITY:	IME OF CONCENTRATION:					NO OFF OF
,	1	1	•	•	0.01	•	AREA				6.5 III/hr						
,	-	•	r	1	0.89	,	CVALUE	VEIGHTED			nvnr						
21.50	t	,	e.	1	0.61	7.11		SUBBASIN	AR				RU		ORAN		
21.50	7.72	7.72	7.72	7.72	7.72	7.11		WEIGHTED SUBBASIN CUMULATIVE PEAK FLOW CUMULATIVE S	AREAS (ac)		PERVIOUS:	IMPERVIOUS:	RUNOFF COEFFICIENTS	23153.01	ORANGE COUNTY, NEW YORK	NFWBURG	ANDS OF ZAZON
21,50	3.53	3.53	3.53	3.53	3.53	7.11	(SB ONLY)	PEAK FLOW	STORM F		0.3	0.9	IENTS		NEW YORK	Ť	AZON
21.50	10.64	10,64	10.64	10.64	10.64	7.11	FLOW	CUMULATIVE	STORM FLOWS (cfs)								
to a	5	5	ц,	.	15	18	(in)	SIZE	PIPE DE			CALCU	GENE				
0.0477	0.0500	0.0520	0.0420	0.0860	0.1100	0.0172	(ft/ft)	SLOPE	PIPE DESIGN INFORMATION		CKED BY:	CALCULATED BY: TBE	RAL INFOR				
24.80	17.00	17.40	15.60	22.30	23.30	14.90	(cfs)	MAXQ	RMATION	CA0711 17	2000	TBE	MATION				
21.50	10.64	10.64	10.64	10,64	10.64	7.11	(cfs)	CALC Q			ļ						
24,80	17.00	17.40	15.60	22.30	23.30	14_90	(cfs)	MAX Q	PIPE ANALYSIS								
86.59%	62.59%	61.15%	68.21%	47.71%	45.66%	47.72%	CAP (%)	UTILIZED	SIS								

.

•

.

[

[]

EXISTING	EXISTING CUNULIUNS OFF SITE CULVERTS			Ū									
					L L	ANDO OF A	ACON						
						NEWBURGH	¥						
					ORANG	E COUNTY,	NEW YORK			-			
						23153.01							
ASSUMED VALUES					RUN	RUNOFF COEFFICIENTS	IENTS		GENE	RAL INFOR	MATION		
	TIME OF CO	TIME OF CONCENTRATION:	10 min	nin		IMPERVIOUS:			CALCU	CALCULATED BY: TBE	TBE		
	RAINF	RAINFALL INTENSITY:		in/hr		PERVIOUS:	0.3		CHE	CHECKED BY:			
		n FACTOR:	0.01					/0	DATE OF CON	OF COMPLETION:	5/20/2005		
	PIPE RUN	CONTRIBUTING AREAS (ac)	G AREAS (ac)		ARE	AREAS (ac)	STORM F	STORM FLOWS (cfs)	PIPE DE	SIGN INFO	RMATION	-0	PIPE ANALYSIS
		PERVIOUS	IMPERVIOUS WEIGHTED SUBBASIN (CUMULATIVE) PEAK FLOW (CUMULATIVE SIZ	WEIGHTED	SUBBASIN		PEAK FLOW	CUMULATIVE	SIZE	TE SLOPE MAX Q	MAX Q	CALC. Q	MAXQ
		AREA	AREA				(SB ONLY)	FLOW	(in)	(fVft)	(cfs)		(cfs)
	DRIVEWAY 1 CULVERT	0.60	0.01	68.0	0.61	19.0	3 53	3.53	12	0.1100	12.80	3 53	12 80
	DRIVEWAY 2												
	CULVERT	•	•	ŀ	1	0.61	3.53	3.53	12	0.0850	11.30	3.53	11.30
	DRIVEWAY 13												
	CULVERT	,	r	1	1	0.61	3.53	3.53	12	0.0420	7.90	3.53	7.90
	DRIVEWAY 4					2	3 3	د د د	5	2	2	2	
								0.00	ł	0,000	0.00	0.40	0.00
	CULVERT	r	1	1	1	0.61	3.53	3,53	12	0.0500	8.60	3.53	8,60
	FROZEN RIDGE												
	RD, CULVERT	1	1	•	23.83	23.83	23,83	23.83	18	0.0477	24.80	23.83	24.80

.

.

ZAZON SWALE-PROP(min. slope)

PROPOSED CONDITIONS FOSTER TOWN ROAD SWALE CALCULATIONS(min. slope)

Given Input Data:

Į

ĺ

Γ

Shape Solving for Flowrate(25 YEAR STORM) Slope Manning's n Height Bottom width Left radius Right radius Left slope Right slope	Depth of Flow 10.6400 cfs 0.0400 ft/ft 0.0350 18.0000 in 36.0000 in 0.0000 in 0.0000 in 0.5000 ft/ft (V/H)
Computed Results: Depth Velocity Full Flowrate Flow area Flow perimeter Hydraulic radius Top width Area Perimeter Percent full	4.7121 fps 72.6595 cfs 2.2580 ft2 65.5471 in 4.9606 in 62.4278 in 9.0000 ft2 116.4984 in
Critical Inform Critical depth Critical slope Critical velocity Critical area Critical perimeter Critical hydraulic radius Critical top width Specific energy Minimum energy Froude number Flow condition	7.5705 in 0.0243 ft/ft 3.9574 fps 2.6886 ft2 69.8563 in 5.5423 in 66.2820 in 0.8956 ft 0.9463 ft 1.2610

ZAZON SWALE-PROP(max slope)

PROPOSED CONDITIONS FOSTER TOWN ROAD SWALE CALCULATIONS(max. slope)

Given Input Data: Advanced Shape Depth of Flow 10.6400 cfs 0.1100 ft/ft Solving for Flowrate(25 YEAR STORM) slope 0.0350 18.0000 in 36.0000 in Bottom width 0.0000 in Left radius 0.0000 in Right radius 0.5000 ft/ft (V/H) Left slope Right slope 0.5000 ft/ft (V/H) Computed Results: 4.9826 in 6.6899 fps Depth Velocity 120.4921 cfs Full Flowrate 1.5905 ft2 Flow area 58.2828 in Flow perimeter 3.9295 in Hydraulic radius Top width 55.9303 in 9.0000 ft2 Area 116.4984 in Perimeter Percent full 27.6810 % Critical Information 7.5705 in Critical depth Critical slope Critical velocity Critical area Critical perimeter Critical hydraulic radius 0.0243 ft/ft 3.9574 fps 2.6886 ft2 69.8563 in 5.5423 in 66.2820 in Critical top width 1.1107 ft Specific energy Minimum energy 0.9463 ft Froude number Flow condition 2.0190 Supercritical

ZAZON SWALE-EX

[EXISTING CONDITIONS FOSTER TOWN ROAD SW	ALE CALCULATIONS
	Given Input Data: Shape Solving for Flowrate(25 YEAR STORM) Slope Manning's n Height Bottom width Left radius Right radius Left slope Right slope	Advanced Depth of Flow 3.5300 cfs 0.0400 ft/ft 0.0350 18.0000 in 36.0000 in 0.0000 in 0.0000 in 0.5000 ft/ft (V/H) 0.5000 ft/ft (V/H)
	Computed Results: Depth Velocity Full Flowrate Flow area Flow perimeter Hydraulic radius Top width Area Perimeter Percent full	3.5441 in 3.3287 fps 72.6595 cfs 1.0605 ft2 51.8497 in 2.9452 in 50.1764 in 9.0000 ft2 116.4984 in 19.6895 %
التي من التي التي التي التي التي التي التي التي	Critical Information Critical depth Critical slope Critical velocity Critical area Critical perimeter Critical perimeter Critical hydraulic radius Critical top width Specific energy Minimum energy Froude number Flow condition	3.8972 in 0.0287 ft/ft 2.9783 fps 1.1852 ft2 53.4287 in 3.1944 in 51.5887 in 0.4675 ft

Ĺ

L

Page 1

		ASSUMED VALUES				 PIPE RUN	FROM		CB-1A	68-1	CB-2A	CB-2	CB-5A	CB-5	68-4	CB-3A	CB-3
			TIME OF C	RAIN			70		CB-1	CB-2	CB-2	CB-3	CB-5	GB-4	CB-3	<u> </u>	DET. POND
	:		TIME OF CONCENTRATION:	RAINFALL INTENSITY:	n FACTOR:	 CONTRIBUTIN	PERVIOUS	AREA	1.43	0.51	0.11	0.00	0,00	0.00	0.00	0.46	0.00
					. 0.01	CONTRIBUTING AREAS (ac)	IMPERVIOUS	AREA	0.49	0.25	0.07	0.12	0.02	0.03	0.01	0.26	0.16
			10:min	6.5 in/hr			١Ü	CVALUE	0.75	0.70	0.67	0,30	0.30	0.30	0.30	0.68	0.30
ORANG		RUN				ARE	SUBBASIN		1.92	0.76	0.18	0.12	0.02	0.03	0.01	0.72	0.16
NEWBURGH	23153.01	RUNOFF COEFFICIENTS	IMPERVIOUS:	PERVIOUS:		AREAS (ac)	CUMULATIVE		1.92	2,68	0,18	2.98	0_02	0.05	0.06	0.72	3.92
NEWBURGH		JENTS		0.3		STORM F	PEAK FLOW	(SB ONLY)	9.32	3.47	0.78	0.23	0_04	0.06	0.02	3.20	0.31
					DA	STORM FLOWS (cfs)	SUBBASIN CUMULATIVE PEAK FLOW CUMULATIVE	FLOW	9.32	12,79	0.78	13.81	0.04	0,10	0.12	3.20	17.43
		GENE	CALCUL	CHE		PIPE DE	SIZE	(iii)	18	18	15	18	15	15	15	15	18
		GENERAL INFORMATION	CALCULATED BY:	CHECKED BY:	TE OF COMPLETION:	PIPE DESIGN INFORMATION	SLOPE	(11/11)	0.0100	0.0580	0,0100	0.0170	0.0100	0.0100	0.0160	0.0240	0,1830
		MATION	TBE		2/22/2005	RMATION	MAXQ	(cfs)	11.37	27.40	6.90	15.00	6,99	6.99	8.85	10.80	48.60
							CALC. Q	(cfs)	9.32	12.79	0.78	13.81	0,04	0.10	0.12	3.20	17.43
						PIPE ANALYSIS	MAX Q	(cfs)	11.37	27.40	06.3	15,00	6.99	6.99	<u>8.85</u>	10.80	48.60
						YSIS	עדונוצ∈ס	CAP (%)	81,98%	46,69%	11.30%	92.04%	0.56%	1.39%	1.32%	29.61%	35.87%

,

.

APPENDIX 12

Γ

{

Ĺ

State Pollutant Discharge Elimination System for Construction Activities Construction Site Log Book

APPENDIX F CONSTRUCTION SITE INSPECTION AND MAINTENANCE LOG BOOK

STATE POLLUTANT DISCHARGE ELIMINATION SYSTEM FOR CONSTRUCTION ACTIVITIES

SAMPLE CONSTRUCTION SITE LOG BOOK

Table of Contents

I. Pre-Construction Meeting Documents

- a. Preamble to Site Assessment and Inspections
- b. Pre-Construction Site Assessment Checklist

II. Construction Duration Inspections

- a. Directions
- b. Modification to the SWPPP

I.)	PRE-CONSTRUCTION MEETING DOCUMENTS
------	------------------------------------

Project Name	
Permit No.	Date of Authorization
Name of Operator	
Prime Contractor	

a. Preamble to Site Assessment and Inspections

The Following Information To Be Read By All Person's Involved in The Construction of Stormwater Related Activities:

The Operator agrees to have a qualified inspector¹ conduct an assessment of the site prior to the commencement of construction² and certify in this inspection report that the appropriate erosion and sediment controls described in the SWPPP have been adequately installed or implemented to ensure overall preparedness of the site for the commencement of construction.

Prior to the commencement of construction, the Operator shall certify in this site logbook that the SWPPP has been prepared in accordance with the State's standards and meets all Federal, State and local erosion and sediment control requirements. A preconstruction meeting should be held to review all of the SWPPP requirements with construction personnel.

When construction starts, site inspections shall be conducted by the qualified inspector at least every 7 calendar days. The Operator shall maintain a record of all inspection reports in this site logbook. The site logbook shall be maintained on site and be made available to the permitting authorities upon request.

Prior to filing the Notice of Termination or the end of permit term, the Operator shall have a qualified inspector perform a final site inspection. The qualified inspector shall certify that the site has undergone final stabilization³ using either vegetative or structural stabilization methods and that all temporary erosion and sediment controls (such as silt fencing) not needed for long-term erosion control have been removed. In addition, the Operator must identify and certify that all permanent structures described in the SWPPP have been constructed and provide the owner(s) with an operation and maintenance plan that ensures the structure(s) continuously functions as designed.

1 Refer to "Qualified Inspector" inspection requirements in the current SPDES General Permit for Stormwater Discharges from Construction Activity for complete list of inspection requirements.

2 "Commencement of construction" means the initial removal of vegetation and disturbance of soils associated with clearing, grading or excavating activities or other construction activities.

3 "Final stabilization" means that all soil-disturbing activities at the site have been completed and a uniform, perennial vegetative cover with a density of eighty (80) percent has been established or equivalent stabilization measures (such as the use of mulches or geotextiles) have been employed on all unpaved areas and areas not covered by permanent structures.

November 2016

b. Pre-construction Site Assessment Checklist (NOTE: Provide comments below as necessary)
1. Notice of Intent, SWPPP, and Contractors Certification:
Yes No NA [] [] Has a Notice of Intent been filed with the NYS Department of Conservation? [] [] Is the SWPPP on-site? Where?
 2. Resource Protection Yes No NA [] [] [] Are construction limits clearly flagged or fenced? [] [] [] Important trees and associated rooting zones, on-site septic system absorption fields, existing vegetated areas suitable for filter strips, especially in perimeter areas, have been flagged for protection. [] [] [] Creek crossings installed prior to land-disturbing activity, including clearing and blasting.
3. Surface Water Protection Yes No NA
 [] [] [] Clean stormwater runoff has been diverted from areas to be disturbed. [] [] Bodies of water located either on site or in the vicinity of the site have been identified and protected. [] [] [] Appropriate practices to protect on-site or downstream surface water are installed. [] [] [] Are clearing and grading operations divided into areas <5 acres?
 4. Stabilized Construction Access Yes No NA
 [] [] [] A temporary construction entrance to capture mud and debris from construction vehicles before they enter the public highway has been installed. [] [] [] Other access areas (entrances, construction routes, equipment parking areas) are stabilized immediately as work takes place with gravel or other cover. [] [] [] Sediment tracked onto public streets is removed or cleaned on a regular basis.
5. Sediment Controls
Yes No NA [] [] [] Silt fence material and installation comply with the standard drawing and specifications. [] [] [] Silt fences are installed at appropriate spacing intervals [] [] [] Sediment/detention basin was installed as first land disturbing activity. [] [] [] Sediment traps and barriers are installed.
 6. Pollution Prevention for Waste and Hazardous Materials Yes No NA [] [] The Operator or designated representative has been assigned to implement the spill prevention avoidance and response plan. [] [] The plan is contained in the SWPPP on page

l

U

II. CONSTRUCTION DURATION INSPECTIONS

a. Directions:

Inspection Forms will be filled out during the entire construction phase of the project.

Required Elements:

- 1) On a site map, indicate the extent of all disturbed site areas and drainage pathways. Indicate site areas that are expected to undergo initial disturbance or significant site work within the next 14-day period;
- 2) Indicate on a site map all areas of the site that have undergone temporary or permanent stabilization;
- 3) Indicate all disturbed site areas that have not undergone active site work during the previous 14-day period;
- 4) Inspect all sediment control practices and record the approximate degree of sediment accumulation as a percentage of sediment storage volume (for example, 10 percent, 20 percent, 50 percent);
- 5) Inspect all erosion and sediment control practices and record all maintenance requirements such as verifying the integrity of barrier or diversion systems (earthen berms or silt fencing) and containment systems (sediment basins and sediment traps). Identify any evidence of rill or gully erosion occurring on slopes and any loss of stabilizing vegetation or seeding/mulching. Document any excessive deposition of sediment or ponding water along barrier or diversion systems. Record the depth of sediment within containment structures, any erosion near outlet and overflow structures, and verify the ability of rock filters around perforated riser pipes to pass water; and
- 6) Immediately report to the Operator any deficiencies that are identified with the implementation of the SWPPP.

CONSTRUCTION DURATION INSPECTIONS

Page 1 of _____

SITE PLAN/SKETCH

Inspector (print name)

Date of Inspection

Qualified Inspector (print name)

Qualified Inspector Signature

The above signed acknowledges that, to the best of his/her knowledge, all information provided on the forms is accurate and complete.

November 2016

L

CONSTRUCTION DURATION INSPECTIONS

Page 2 of _____

Maintaining Water Quality

Yes No NA

- [] [] [] Is there an increase in turbidity causing a substantial visible contrast to natural conditions at the outfalls?
- [] [] [] Is there residue from oil and floating substances, visible oil film, or globules or grease at the outfalls?
- [] [] [] All disturbance is within the limits of the approved plans.
- [] [] Have receiving lake/bay, stream, and/or wetland been impacted by silt from project?

Housekeeping

1. General Site Conditions

Yes No NA

- [] [] [] Is construction site litter, debris and spoils appropriately managed?
- [] [] Are facilities and equipment necessary for implementation of erosion and sediment control in working order and/or properly maintained?
- [] [] [] Is construction impacting the adjacent property?
- [] [] [] Is dust adequately controlled?
- 2. Temporary Stream Crossing

Yes No NA

- [] [] [] Maximum diameter pipes necessary to span creek without dredging are installed.
- [] [] [] Installed non-woven geotextile fabric beneath approaches.
- [] [] [] Is fill composed of aggregate (no earth or soil)?
- [] [] [] Rock on approaches is clean enough to remove mud from vehicles & prevent sediment from entering stream during high flow.

3. Stabilized Construction Access

Yes No NA

- [] [] [] Stone is clean enough to effectively remove mud from vehicles.
- [] [] Installed per standards and specifications?
- [] [] Does all traffic use the stabilized entrance to enter and leave site?
- [] [] [] Is adequate drainage provided to prevent ponding at entrance?

Runoff Control Practices

1. Excavation Dewatering

Yes No NA

i

- [] [] Upstream and downstream berms (sandbags, inflatable dams, etc.) are installed per plan.
- [] [] Clean water from upstream pool is being pumped to the downstream pool.
- [] [] Sediment laden water from work area is being discharged to a silt-trapping device.
- [] [] [] Constructed upstream berm with one-foot minimum freeboard.

CONSTRUCTION DURATION INSPECTIONS	Page 3 of
Runoff Control Practices (continued)	
2. Flow Spreader	
Yes No NA	
[] [] [] Installed per plan.	
[] [] [] Constructed on undisturbed soil, not on fill, receiving only cle	ar, non-sediment laden flow.
[] [] Flow sheets out of level spreader without erosion on downstre	am edge.
3. Interceptor Dikes and Swales	
Yes No NA	
[] [] [] Installed per plan with minimum side slopes 2H:1V or flatter.	
[] [] [] Stabilized by geotextile fabric, seed, or mulch with no erosion	occurring.

[] [] [] Sediment-laden runoff directed to sediment trapping structure

4. Stone Check Dam

Yes No NA

- [] [] [] Is channel stable? (flow is not eroding soil underneath or around the structure).
- [] [] Check is in good condition (rocks in place and no permanent pools behind the structure).

[] [] [] Has accumulated sediment been removed?.

5. Rock Outlet Protection

Yes No NA

[] [] [] Installed per plan.

[] [] Installed concurrently with pipe installation.

Soil Stabilization

1. Topsoil and Spoil Stockpiles

Yes No NA

- [] [] [] Stockpiles are stabilized with vegetation and/or mulch.
- [] [] Sediment control is installed at the toe of the slope. []

2. Revegetation

Yes No NA

- [] [] [] Temporary seedings and mulch have been applied to idle areas.
- [] [] []4 inches minimum of topsoil has been applied under permanent seedings

Sediment Control Practices

1. Silt Fence and Linear Barriers

Yes No NA

- [] [] Installed on Contour, 10 feet from toe of slope (not across conveyance channels).
- [] [] Joints constructed by wrapping the two ends together for continuous support.
- [] [] Fabric buried 6 inches minimum.
- [] [] Posts are stable, fabric is tight and without rips or frayed areas.

Sediment accumulation is ____% of design capacity.

CONSTRUCTION DURATION INSPECTIONS

Page 4 of _____

Sediment Control Practices (continued)

2. Storm Drain Inlet Protection (Use for Stone & Block; Filter Fabric; Curb; or, Excavated; Filter Sock or Manufactured practices)

Yes No NA

- [] [] Installed concrete blocks lengthwise so open ends face outward, not upward.
- [] [] [] Placed wire screen between No. 3 crushed stone and concrete blocks.
- [] [] Drainage area is lacre or less.
- [] [] Excavated area is 900 cubic feet.
- $\begin{bmatrix} j \end{bmatrix} \begin{bmatrix} j \end{bmatrix} \end{bmatrix}$ Excavated side slopes should be 2:1.
- [] [] [] 2" x 4" frame is constructed and structurally sound.
- [] [] [] Posts 3-foot maximum spacing between posts.
- [] [] Fabric is embedded 1 to 1.5 feet below ground and secured to frame/posts with staples at max 8inch spacing.
- [] [] Posts are stable, fabric is tight and without rips or frayed areas.
- [] [] [] Manufactured insert fabric is free of tears and punctures.
- $\begin{bmatrix} 1 \\ 1 \end{bmatrix} \begin{bmatrix} 1 \\ 1 \end{bmatrix}$ Filter Sock is not torn or flattened and fill material is contained within the mesh sock.

Sediment accumulation % of design capacity.

3. Temporary Sediment Trap

Yes No NA

- [] [] [] Outlet structure is constructed per the approved plan or drawing.
- [] [] [] Geotextile fabric has been placed beneath rock fill.
- [] [] [] Sediment trap slopes and disturbed areas are stabilized.

Sediment accumulation is ___% of design capacity.

4. Temporary Sediment Basin

Yes No NA

- [] [] [] Basin and outlet structure constructed per the approved plan.
- [] [] Basin side slopes are stabilized with seed/mulch.
- [] [] Drainage structure flushed and basin surface restored upon removal of sediment basin facility.
- [] [] [] Sediment basin dewatering pool is dewatering at appropriate rate.

Sediment accumulation is ___% of design capacity.

<u>Note</u>: Not all erosion and sediment control practices are included in this listing. Add additional pages to this list as required by site specific design. All practices shall be maintained in accordance with their respective standards.

Construction inspection checklists for post-development stormwater management practices can be found in Appendix F of the New York Stormwater Management Design Manual.

CONSTRUCTION DURATION INSPECTIONS

b.	Modifications to the SWPPP (To be completed as described below)	
----	------------------------------	-------------------------------------	--

The Operator shall amend the SWPPP whenever:

- 1. There is a significant change in design, construction, operation, or maintenance which may have a significant effect on the potential for the discharge of pollutants to the waters of the United States and which has not otherwise been addressed in the SWPPP; or
- 2. The SWPPP proves to be ineffective in:
 - a. Eliminating or significantly minimizing pollutants from sources identified in the SWPPP and as required by this permit; or
 - b. Achieving the general objectives of controlling pollutants in stormwater discharges from permitted construction activity; and
- 3. Additionally, the SWPPP shall be amended to identify any new contractor or subcontractor that will implement any measure of the SWPPP.

Modification & Reason:

	·····
	·····
· · · · · · · · · · · · · · · · · · ·	
	· · · · · · · · · · · · · · · · · · ·
	······································
	······································
3	
·····	
<u> </u>	
	· · · · · · · · · · · · · · · · · · ·

APPENDIX 13

-

New York State Department of Environmental Conservation Permit No. GP-0-20-002



Department of Environmental Conservation

NEW YORK STATE DEPARTMENT OF ENVIRONMENTAL CONSERVATION

SPDES GENERAL PERMIT FOR STORMWATER DISCHARGES

From

CONSTRUCTION ACTIVITY

Permit No. GP- 0-20-001

Issued Pursuant to Article 17, Titles 7, 8 and Article 70

of the Environmental Conservation Law

Effective Date: January 29, 2020

Expiration Date: January 28, 2025

John J. Ferguson

Chief Permit Administrator

Authorized Signature

1-23-20

Date

Address: NYS DEC Division of Environmental Permits 625 Broadway, 4th Floor Albany, N.Y. 12233-1750

PREFACE

Pursuant to Section 402 of the Clean Water Act ("CWA"), stormwater *discharges* from certain *construction activities* are unlawful unless they are authorized by a *National Pollutant Discharge Elimination System ("NPDES")* permit or by a state permit program. New York administers the approved State Pollutant Discharge Elimination System (SPDES) program with permits issued in accordance with the New York State Environmental Conservation Law (ECL) Article 17, Titles 7, 8 and Article 70.

An owner or operator of a construction activity that is eligible for coverage under this permit must obtain coverage prior to the *commencement of construction activity*. Activities that fit the definition of "*construction activity*", as defined under 40 CFR 122.26(b)(14)(x), (15)(i), and (15)(ii), constitute construction of a *point source* and therefore, pursuant to ECL section 17-0505 and 17-0701, the *owner or operator* must have coverage under a SPDES permit prior to *commencing construction activity*. The *owner or operator* cannot wait until there is an actual *discharge* from the *construction site* to obtain permit coverage.

*Note: The italicized words/phrases within this permit are defined in Appendix A.

NEW YORK STATE DEPARTMENT OF ENVIRONMENTAL CONSERVATION SPDES GENERAL PERMIT FOR STORMWATER DISCHARGES FROM CONSTRUCTION ACTIVITIES

Table of Contents

Part 1.	PERMIT COVERAGE AND LIMITATIONS	1
Α.	Permit Application	
В.	Effluent Limitations Applicable to Discharges from Construction Activities	1
C.	Post-construction Stormwater Management Practice Requirements	
D.	Maintaining Water Quality	8
E.	Maintaining Water Quality Eligibility Under This General Permit	9
F.	Activities Which Are Ineligible for Coverage Under This General Permit	9
Part II.	PERMIT COVERAGE	12
Α.	How to Obtain Coverage	12
В.	Notice of Intent (NOI) Submittal	13
C.	Permit Authorization	
D.	General Requirements For Owners or Operators With Permit Coverage	15
E.	Permit Coverage for Discharges Authorized Under GP-0-15-002	17
F.	Change of Owner or Operator	17
Part III.	STORMWATER POLLUTION PREVENTION PLAN (SWPPP)	18
Α.	General SWPPP Requirements	18
В.	Required SWPPP Contents	
C.	Required SWPPP Components by Project Type	
Part IV	. INSPECTION AND MAINTENANCE REQUIREMENTS	24
Α.	General Construction Site Inspection and Maintenance Requirements	24
В.	Contractor Maintenance Inspection Requirements	24
С.	Qualified Inspector Inspection Requirements	
Part V.	TERMINATION OF PERMIT COVERAGE	29
Α.	Termination of Permit Coverage	
Part VI.	REPORTING AND RETENTION RECORDS	
Α.	Record Retention	31
Β.	Addresses	
Part VI	I. STANDARD PERMIT CONDITIONS	31
Α.	Duty to Comply	
В.	Continuation of the Expired General Permit	
C.	Enforcement	
D.	Need to Halt or Reduce Activity Not a Defense	32
E.	Duty to Mitigate	33
F.	Duty to Provide Information	
G.	Other Information	
Η.	Signatory Requirements	33
1.	Property Rights	35
J.	Severability	

Li

Ł

	Κ.	Requirement to Obtain Coverage Under an Alternative Permit	35
	L.	Proper Operation and Maintenance	
	Μ.	Inspection and Entry	
	N.	Permit Actions	
	О.	Definitions	37
	Ρ.	Re-Opener Clause	37
	Q.	Penalties for Falsification of Forms and Reports	37
	R.	Other Permits	
	APPEN	IDIX A – Acronyms and Definitions	
	Acro	nyms	
	Defir	nitions	40
	APPEN	IDIX B – Required SWPPP Components by Project Type	48
		e 1	
	Table	e 2	50
	APPEN	IDIX C – Watersheds Requiring Enhanced Phosphorus Removal	52
•	APPEN	IDIX D – Watersheds with Lower Disturbance Threshold	58
	APPEN	IDIX E – 303(d) Segments Impaired by Construction Related Pollutant(s)	59
	APPEN	IDIX F – List of NYS DEC Regional Offices	65
		-	

· ·

Part 1. PERMIT COVERAGE AND LIMITATIONS

A. Permit Application

This permit authorizes stormwater *discharges* to *surface waters of the State* from the following *construction activities* identified within 40 CFR Parts 122.26(b)(14)(x), 122.26(b)(15)(i) and 122.26(b)(15)(ii), provided all of the eligibility provisions of this permit are met:

- 1. Construction activities involving soil disturbances of one (1) or more acres; including disturbances of less than one acre that are part of a *larger common plan of development or sale* that will ultimately disturb one or more acres of land; excluding *routine maintenance activity* that is performed to maintain the original line and grade, hydraulic capacity or original purpose of a facility;
- 2. Construction activities involving soil disturbances of less than one (1) acre where the Department has determined that a *SPDES* permit is required for stormwater *discharges* based on the potential for contribution to a violation of a *water quality standard* or for significant contribution of *pollutants* to *surface waters of the State.*
- 3. Construction activities located in the watershed(s) identified in Appendix D that involve soil disturbances between five thousand (5,000) square feet and one (1) acre of land.

B. Effluent Limitations Applicable to Discharges from Construction Activities

Discharges authorized by this permit must achieve, at a minimum, the effluent limitations in Part I.B.1. (a) – (f) of this permit. These limitations represent the degree of effluent reduction attainable by the application of best practicable technology currently available.

1. Erosion and Sediment Control Requirements - The owner or operator must select, design, install, implement and maintain control measures to minimize the discharge of pollutants and prevent a violation of the water quality standards. The selection, design, installation, implementation, and maintenance of these control measures must meet the non-numeric effluent limitations in Part I.B.1.(a) – (f) of this permit and be in accordance with the New York State Standards and Specifications for Erosion and Sediment Control, dated November 2016, using sound engineering judgment. Where control measures are not designed in conformance with the design criteria included in the technical standard, the owner or operator must include in the Stormwater Pollution Prevention Plan ("SWPPP") the reason(s) for the

(Part I.B.1)

deviation or alternative design and provide information which demonstrates that the deviation or alternative design is *equivalent* to the technical standard.

- a. **Erosion and Sediment Controls.** Design, install and maintain effective erosion and sediment controls to *minimize* the *discharge* of *pollutants* and prevent a violation of the *water quality standards*. At a minimum, such controls must be designed, installed and maintained to:
 - (i) *Minimize* soil erosion through application of runoff control and soil stabilization control measure to *minimize pollutant discharges*;
 - (ii) Control stormwater *discharges*, including both peak flowrates and total stormwater volume, to *minimize* channel and *streambank* erosion and scour in the immediate vicinity of the *discharge* points;
 - (iii) *Minimize* the amount of soil exposed during *construction activity*;
 - (iv) *Minimize* the disturbance of *steep slopes*;
 - (v) *Minimize* sediment *discharges* from the site;
 - (vi) Provide and maintain *natural buffers* around surface waters, direct stormwater to vegetated areas and maximize stormwater infiltration to reduce *pollutant discharges*, unless *infeasible*;
 - (vii) Minimize soil compaction. Minimizing soil compaction is not required where the intended function of a specific area of the site dictates that it be compacted;
 - (viii) Unless *infeasible*, preserve a sufficient amount of topsoil to complete soil restoration and establish a uniform, dense vegetative cover; and
 - (ix) *Minimize* dust. On areas of exposed soil, *minimize* dust through the appropriate application of water or other dust suppression techniques to control the generation of pollutants that could be discharged from the site.
- b. Soil Stabilization. In areas where soil disturbance activity has temporarily or permanently ceased, the application of soil stabilization measures must be initiated by the end of the next business day and completed within fourteen (14) days from the date the current soil disturbance activity ceased. For construction sites that *directly discharge* to one of the 303(d) segments

listed in Appendix E or is located in one of the watersheds listed in Appendix C, the application of soil stabilization measures must be initiated by the end of the next business day and completed within seven (7) days from the date the current soil disturbance activity ceased. See Appendix A for definition of *Temporarily Ceased*.

- c. **Dewatering**. *Discharges* from *dewatering* activities, including *discharges* from *dewatering* of trenches and excavations, must be managed by appropriate control measures.
- d. **Pollution Prevention Measures**. Design, install, implement, and maintain effective pollution prevention measures to *minimize* the *discharge* of *pollutants* and prevent a violation of the *water quality standards*. At a minimum, such measures must be designed, installed, implemented and maintained to:
 - (i) Minimize the discharge of pollutants from equipment and vehicle washing, wheel wash water, and other wash waters. This applies to washing operations that use clean water only. Soaps, detergents and solvents cannot be used;
 - (ii) Minimize the exposure of building materials, building products, construction wastes, trash, landscape materials, fertilizers, pesticides, herbicides, detergents, sanitary waste, hazardous and toxic waste, and other materials present on the site to precipitation and to stormwater. Minimization of exposure is not required in cases where the exposure to precipitation and to stormwater will not result in a *discharge* of *pollutants*, or where exposure of a specific material or product poses little risk of stormwater contamination (such as final products and materials intended for outdoor use); and
 - (iii) Prevent the *discharge* of *pollutants* from spills and leaks and implement chemical spill and leak prevention and response procedures.
- e. **Prohibited** *Discharges*. The following *discharges* are prohibited:
 - (i) Wastewater from washout of concrete;
 - (ii) Wastewater from washout and cleanout of stucco, paint, form release oils, curing compounds and other construction materials;

- (iii) Fuels, oils, or other *pollutants* used in vehicle and equipment operation and maintenance;
- (iv) Soaps or solvents used in vehicle and equipment washing; and
- (v) Toxic or hazardous substances from a spill or other release.
- f. Surface Outlets. When discharging from basins and impoundments, the outlets shall be designed, constructed and maintained in such a manner that sediment does not leave the basin or impoundment and that erosion at or below the outlet does not occur.

C. Post-construction Stormwater Management Practice Requirements

- The owner or operator of a construction activity that requires post-construction stormwater management practices pursuant to Part III.C. of this permit must select, design, install, and maintain the practices to meet the *performance criteria* in the New York State Stormwater Management Design Manual ("Design Manual"), dated January 2015, using sound engineering judgment. Where post-construction stormwater management practices ("SMPs") are not designed in conformance with the *performance criteria* in the Design Manual, the *owner or operator* must include in the SWPPP the reason(s) for the deviation or alternative design and provide information which demonstrates that the deviation or alternative design is *equivalent* to the technical standard.
- 2. The owner or operator of a construction activity that requires post-construction stormwater management practices pursuant to Part III.C. of this permit must design the practices to meet the applicable *sizing criteria* in Part I.C.2.a., b., c. or d. of this permit.

a. Sizing Criteria for New Development

- (i) Runoff Reduction Volume ("RRv"): Reduce the total Water Quality Volume ("WQv") by application of RR techniques and standard SMPs with RRv capacity. The total WQv shall be calculated in accordance with the criteria in Section 4.2 of the Design Manual.
- (ii) Minimum RRv and Treatment of Remaining Total WQv: Construction activities that cannot meet the criteria in Part I.C.2.a.(i) of this permit due to site limitations shall direct runoff from all newly constructed impervious areas to a RR technique or standard SMP with RRv capacity unless infeasible. The specific site limitations that prevent the reduction of 100% of the WQv shall be documented in the SWPPP.

For each impervious area that is not directed to a RR technique or standard SMP with RRv capacity, the SWPPP must include documentation which demonstrates that all options were considered and for each option explains why it is considered infeasible.

In no case shall the runoff reduction achieved from the newly constructed impervious areas be less than the Minimum RRv as calculated using the criteria in Section 4.3 of the Design Manual. The remaining portion of the total WQv that cannot be reduced shall be treated by application of standard SMPs.

- (iii) Channel Protection Volume ("Cpv"): Provide 24 hour extended detention of the post-developed 1-year, 24-hour storm event; remaining after runoff reduction. The Cpv requirement does not apply when:
 - (1) Reduction of the entire Cpv is achieved by application of runoff reduction techniques or infiltration systems, or
 - (2) The site discharges directly to tidal waters, or fifth order or larger streams.
- (iv) Overbank Flood Control Criteria ("Qp"): Requires storage to attenuate the post-development 10-year, 24-hour peak discharge rate (Qp) to predevelopment rates. The Qp requirement does not apply when:
 (1) the site discharges directly to tidal waters or fifth order or larger
 - streams, or
 - (2) A downstream analysis reveals that *overbank* control is not required.
- (v) Extreme Flood Control Criteria ("Qf"): Requires storage to attenuate the post-development 100-year, 24-hour peak discharge rate (Qf) to predevelopment rates. The Qf requirement does not apply when:
 - (1) the site discharges directly to tidal waters or fifth order or larger streams, or
 - (2) A downstream analysis reveals that *overbank* control is not required.

b. Sizing Criteria for New Development in Enhanced Phosphorus Removal Watershed

Runoff Reduction Volume (RRv): Reduce the total Water Quality
 Volume (WQv) by application of RR techniques and standard SMPs
 with RRv capacity. The total WQv is the runoff volume from the 1-year,
 24 hour design storm over the post-developed watershed and shall be

(Part I.C.2.b.i)

calculated in accordance with the criteria in Section 10.3 of the Design Manual.

(ii) Minimum RRv and Treatment of Remaining Total WQv: Construction activities that cannot meet the criteria in Part I.C.2.b.(i) of this permit due to site limitations shall direct runoff from all newly constructed impervious areas to a RR technique or standard SMP with RRv capacity unless infeasible. The specific site limitations that prevent the reduction of 100% of the WQv shall be documented in the SWPPP. For each impervious area that is not directed to a RR technique or standard SMP with RRv capacity, the SWPPP must include documentation which demonstrates that all options were considered and for each option explains why it is considered infeasible.

In no case shall the runoff reduction achieved from the newly constructed *impervious areas* be less than the Minimum RRv as calculated using the criteria in Section 10.3 of the Design Manual. The remaining portion of the total WQv that cannot be reduced shall be treated by application of standard SMPs.

- (iii) Channel Protection Volume (Cpv): Provide 24 hour extended detention of the post-developed 1-year, 24-hour storm event; remaining after runoff reduction. The Cpv requirement does not apply when:
 - (1) Reduction of the entire Cpv is achieved by application of runoff reduction techniques or infiltration systems, or
 - (2) The site *discharges* directly to tidal waters, or fifth order or larger streams.
- (iv) Overbank Flood Control Criteria (Qp): Requires storage to attenuate the post-development 10-year, 24-hour peak discharge rate (Qp) to predevelopment rates. The Qp requirement does not apply when:
 - (1) the site *discharges* directly to tidal waters or fifth order or larger streams, or
 - (2) A downstream analysis reveals that *overbank* control is not required.
- (v) Extreme Flood Control Criteria (Qf): Requires storage to attenuate the post-development 100-year, 24-hour peak *discharge* rate (Qf) to predevelopment rates. The Qf requirement does not apply when:
 - (1) the site *discharges* directly to tidal waters or fifth order or larger streams, or
 - (2) A downstream analysis reveals that *overbank* control is not required.

c. Sizing Criteria for Redevelopment Activity

- (i) Water Quality Volume (WQv): The WQv treatment objective for redevelopment activity shall be addressed by one of the following options. Redevelopment activities located in an Enhanced Phosphorus Removal Watershed (see Part III.B.3. and Appendix C of this permit) shall calculate the WQv in accordance with Section 10.3 of the Design Manual. All other redevelopment activities shall calculate the WQv in accordance with Section 4.2 of the Design Manual.
 - (1) Reduce the existing *impervious cover* by a minimum of 25% of the total disturbed, *impervious area*. The Soil Restoration criteria in Section 5.1.6 of the Design Manual must be applied to all newly created pervious areas, or
 - (2) Capture and treat a minimum of 25% of the WQv from the disturbed, impervious area by the application of standard SMPs; or reduce 25% of the WQv from the disturbed, impervious area by the application of RR techniques or standard SMPs with RRv capacity., or
 - (3) Capture and treat a minimum of 75% of the WQv from the disturbed, impervious area as well as any additional runoff from tributary areas by application of the alternative practices discussed in Sections 9.3 and 9.4 of the Design Manual., or
 - (4) Application of a combination of 1, 2 and 3 above that provide a weighted average of at least two of the above methods. Application of this method shall be in accordance with the criteria in Section 9.2.1(B) (IV) of the Design Manual.

If there is an existing post-construction stormwater management practice located on the site that captures and treats runoff from the *impervious area* that is being disturbed, the WQv treatment option selected must, at a minimum, provide treatment equal to the treatment that was being provided by the existing practice(s) if that treatment is greater than the treatment required by options 1 - 4 above.

- (ii) Channel Protection Volume (Cpv): Not required if there are no changes to hydrology that increase the *discharge* rate from the project site.
- (iii) *Overbank* Flood Control Criteria (Qp): Not required if there are no changes to hydrology that increase the *discharge* rate from the project site.
- (iv) Extreme Flood Control Criteria (Qf): Not required if there are no changes to hydrology that increase the *discharge* rate from the project site

(Part I.C.2.d)

d. Sizing Criteria for Combination of Redevelopment Activity and New Development

Construction projects that include both New Development and Redevelopment Activity shall provide post-construction stormwater management controls that meet the sizing criteria calculated as an aggregate of the Sizing Criteria in Part I.C.2.a. or b. of this permit for the New Development portion of the project and Part I.C.2.c of this permit for Redevelopment Activity portion of the project.

D. Maintaining Water Quality

The Department expects that compliance with the conditions of this permit will control *discharges* necessary to meet applicable *water quality standards*. It shall be a violation of the *ECL* for any discharge to either cause or contribute to a violation of *water quality standards* as contained in Parts 700 through 705 of Title 6 of the Official Compilation of Codes, Rules and Regulations of the State of New York, such as:

- 1. There shall be no increase in turbidity that will cause a substantial visible contrast to natural conditions;
- 2. There shall be no increase in suspended, colloidal or settleable solids that will cause deposition or impair the waters for their best usages; and
- 3. There shall be no residue from oil and floating substances, nor visible oil film, nor globules of grease.

If there is evidence indicating that the stormwater *discharges* authorized by this permit are causing, have the reasonable potential to cause, or are contributing to a violation of the *water quality standards*; the *owner or operator* must take appropriate corrective action in accordance with Part IV.C.5. of this general permit and document in accordance with Part IV.C.4. of this general permit. To address the *water quality standard* violation the *owner or operator* may need to provide additional information, include and implement appropriate controls in the SWPPP to correct the problem, or obtain an individual SPDES permit.

If there is evidence indicating that despite compliance with the terms and conditions of this general permit it is demonstrated that the stormwater *discharges* authorized by this permit are causing or contributing to a violation of *water quality standards*, or if the Department determines that a modification of the permit is necessary to prevent a violation of *water quality standards*, the authorized *discharges* will no longer be eligible for coverage under this permit. The Department may require the *owner or operator* to obtain an individual SPDES permit to continue discharging.
E. Eligibility Under This General Permit

- 1. This permit may authorize all *discharges* of stormwater from *construction activity* to *surface waters of the State* and *groundwaters* except for ineligible *discharges* identified under subparagraph F. of this Part.
- 2. Except for non-stormwater *discharges* explicitly listed in the next paragraph, this permit only authorizes stormwater *discharges*; including stormwater runoff, snowmelt runoff, and surface runoff and drainage, from *construction activities*.
- 3. Notwithstanding paragraphs E.1 and E.2 above, the following non-stormwater discharges are authorized by this permit: those listed in 6 NYCRR 750-1.2(a)(29)(vi), with the following exception: "Discharges from firefighting activities are authorized only when the firefighting activities are emergencies/unplanned"; waters to which other components have not been added that are used to control dust in accordance with the SWPPP; and uncontaminated *discharges* from *construction site* de-watering operations. All non-stormwater discharges must be identified in the SWPPP. Under all circumstances, the *owner or operator* must still comply with *water quality standards* in Part I.D of this permit.
- 4. The *owner or operator* must maintain permit eligibility to *discharge* under this permit. Any *discharges* that are not compliant with the eligibility conditions of this permit are not authorized by the permit and the *owner or operator* must either apply for a separate permit to cover those ineligible *discharges* or take steps necessary to make the *discharge* eligible for coverage.

F. Activities Which Are Ineligible for Coverage Under This General Permit

All of the following are **not** authorized by this permit:

- 1. *Discharges* after *construction activities* have been completed and the site has undergone *final stabilization*;
- 2. *Discharges* that are mixed with sources of non-stormwater other than those expressly authorized under subsection E.3. of this Part and identified in the SWPPP required by this permit;
- 3. *Discharges* that are required to obtain an individual SPDES permit or another SPDES general permit pursuant to Part VII.K. of this permit;
- 4. Construction activities or discharges from construction activities that may adversely affect an endangered or threatened species unless the owner or

(Part I.F.4)

operator has obtained a permit issued pursuant to 6 NYCRR Part 182 for the project or the Department has issued a letter of non-jurisdiction for the project. All documentation necessary to demonstrate eligibility shall be maintained on site in accordance with Part II.D.2 of this permit;

- 5. *Discharges* which either cause or contribute to a violation of *water quality standards* adopted pursuant to the *ECL* and its accompanying regulations;
- 6. Construction activities for residential, commercial and institutional projects:
 - a. Where the *discharges* from the *construction activities* are tributary to waters of the state classified as AA or AA-s; and
 - b. Which are undertaken on land with no existing *impervious cover*; and
 - c. Which disturb one (1) or more acres of land designated on the current United States Department of Agriculture ("USDA") Soil Survey as Soil Slope Phase "D", (provided the map unit name is inclusive of slopes greater than 25%), or Soil Slope Phase "E" or "F" (regardless of the map unit name), or a combination of the three designations.
- 7. Construction activities for linear transportation projects and linear utility projects:
 - a. Where the *discharges* from the *construction activities* are tributary to waters of the state classified as AA or AA-s; and
 - b. Which are undertaken on land with no existing impervious cover; and

c. Which disturb two (2) or more acres of land designated on the current USDA Soil Survey as Soil Slope Phase "D" (provided the map unit name is inclusive of slopes greater than 25%), or Soil Slope Phase "E" or "F" (regardless of the map unit name), or a combination of the three designations.

- 8. Construction activities that have the potential to affect an *historic property*, unless there is documentation that such impacts have been resolved. The following documentation necessary to demonstrate eligibility with this requirement shall be maintained on site in accordance with Part II.D.2 of this permit and made available to the Department in accordance with Part VII.F of this permit:
 - a. Documentation that the *construction activity* is not within an archeologically sensitive area indicated on the sensitivity map, and that the *construction activity* is not located on or immediately adjacent to a property listed or determined to be eligible for listing on the National or State Registers of Historic Places, and that there is no new permanent building on the *construction site* within the following distances from a building, structure, or object that is more than 50 years old, or if there is such a new permanent building on the *construction site* within those parameters that NYS Office of Parks, Recreation and Historic Preservation (OPRHP), a Historic Preservation Commission of a Certified Local Government, or a qualified preservation professional has determined that the building, structure, or object more than 50 years old is not historically/archeologically significant.
 - 1-5 acres of disturbance 20 feet
 - 5-20 acres of disturbance 50 feet
 - 20+ acres of disturbance 100 feet, or
 - b. DEC consultation form sent to OPRHP, and copied to the NYS DEC Agency Historic Preservation Officer (APO), and
 - the State Environmental Quality Review (SEQR) Environmental Assessment Form (EAF) with a negative declaration or the Findings Statement, with documentation of OPRHP's agreement with the resolution; or
 - (ii) documentation from OPRHP that the *construction activity* will result in No Impact; or
 - (iii) documentation from OPRHP providing a determination of No Adverse Impact; or
 - (iv) a Letter of Resolution signed by the owner/operator, OPRHP and the DEC APO which allows for this *construction activity* to be eligible for coverage under the general permit in terms of the State Historic Preservation Act (SHPA); or
 - c. Documentation of satisfactory compliance with Section 106 of the National Historic Preservation Act for a coterminous project area:

(Part I.F.8.c)

- (i) No Affect
- (ii) No Adverse Affect
- (iii) Executed Memorandum of Agreement, or
- d. Documentation that:
- SHPA Section 14.09 has been completed by NYS DEC or another state agency.
- 9. Discharges from construction activities that are subject to an existing SPDES individual or general permit where a SPDES permit for *construction activity* has been terminated or denied; or where the *owner or operator* has failed to renew an expired individual permit.

Part II. PERMIT COVERAGE

A. How to Obtain Coverage

- An owner or operator of a construction activity that is not subject to the requirements of a regulated, traditional land use control MS4 must first prepare a SWPPP in accordance with all applicable requirements of this permit and then submit a completed Notice of Intent (NOI) to the Department to be authorized to discharge under this permit.
- 2. An owner or operator of a construction activity that is subject to the requirements of a regulated, traditional land use control MS4 must first prepare a SWPPP in accordance with all applicable requirements of this permit and then have the SWPPP reviewed and accepted by the regulated, traditional land use control MS4 prior to submitting the NOI to the Department. The owner or operator shall have the "MS4 SWPPP Acceptance" form signed in accordance with Part VII.H., and then submit that form along with a completed NOI to the Department.
- 3. The requirement for an owner or operator to have its SWPPP reviewed and accepted by the regulated, traditional land use control MS4 prior to submitting the NOI to the Department does not apply to an owner or operator that is obtaining permit coverage in accordance with the requirements in Part II.F. (Change of Owner or Operator) or where the owner or operator of the construction activity is the regulated, traditional land use control MS4. This exemption does not apply to construction activities subject to the New York City Administrative Code.

B. Notice of Intent (NOI) Submittal

 Prior to December 21, 2020, an owner or operator shall use either the electronic (eNOI) or paper version of the NOI that the Department prepared. Both versions of the NOI are located on the Department's website (http://www.dec.ny.gov/). The paper version of the NOI shall be signed in accordance with Part VII.H. of this permit and submitted to the following address:

NOTICE OF INTENT NYS DEC, Bureau of Water Permits 625 Broadway, 4th Floor Albany, New York 12233-3505

- 2. Beginning December 21, 2020 and in accordance with EPA's 2015 NPDES Electronic Reporting Rule (40 CFR Part 127), the *owner or operator* must submit the NOI electronically using the *Department's* online NOI.
- 3. The *owner or operator* shall have the SWPPP preparer sign the "SWPPP Preparer Certification" statement on the NOI prior to submitting the form to the Department.
- 4. As of the date the NOI is submitted to the Department, the *owner or operator* shall make the NOI and SWPPP available for review and copying in accordance with the requirements in Part VII.F. of this permit.

C. Permit Authorization

- 1. An owner or operator shall not commence construction activity until their authorization to discharge under this permit goes into effect.
- 2. Authorization to *discharge* under this permit will be effective when the *owner* or *operator* has satisfied <u>all</u> of the following criteria:
 - a. project review pursuant to the State Environmental Quality Review Act ("SEQRA") have been satisfied, when SEQRA is applicable. See the Department's website (<u>http://www.dec.ny.gov/</u>) for more information,
 - b. where required, all necessary Department permits subject to the Uniform Procedures Act ("UPA") (see 6 NYCRR Part 621), or the equivalent from another New York State agency, have been obtained, unless otherwise notified by the Department pursuant to 6 NYCRR 621.3(a)(4). Owners or operators of construction activities that are required to obtain UPA permits

must submit a preliminary SWPPP to the appropriate DEC Permit Administrator at the Regional Office listed in Appendix F at the time all other necessary UPA permit applications are submitted. The preliminary SWPPP must include sufficient information to demonstrate that the *construction activity* qualifies for authorization under this permit,

- c. the final SWPPP has been prepared, and
- d. a complete NOI has been submitted to the Department in accordance with the requirements of this permit.
- 3. An *owner or operator* that has satisfied the requirements of Part II.C.2 above will be authorized to *discharge* stormwater from their *construction activity* in accordance with the following schedule:
 - a. For *construction activities* that are <u>not</u> subject to the requirements of a *regulated, traditional land use control MS4*:
 - (i) Five (5) business days from the date the Department receives a complete electronic version of the NOI (eNOI) for *construction activities* with a SWPPP that has been prepared in conformance with the design criteria in the technical standard referenced in Part III.B.1 and the *performance criteria* in the technical standard referenced in Parts III.B., 2 or 3, for *construction activities* that require post-construction stormwater management practices pursuant to Part III.C.; or
 - (ii) Sixty (60) business days from the date the Department receives a complete NOI (electronic or paper version) for *construction activities* with a SWPPP that has <u>not</u> been prepared in conformance with the design criteria in technical standard referenced in Part III.B.1. or, for *construction activities* that require post-construction stormwater management practices pursuant to Part III.C., the *performance criteria* in the technical standard referenced in Parts III.B., 2 or 3, or;
 - (iii) Ten (10) business days from the date the Department receives a complete paper version of the NOI for *construction activities* with a SWPPP that has been prepared in conformance with the design criteria in the technical standard referenced in Part III.B.1 and the *performance criteria* in the technical standard referenced in Parts III.B., 2 or 3, for *construction activities* that require post-construction stormwater management practices pursuant to Part III.C.

- b. For *construction activities* that are subject to the requirements of a *regulated, traditional land use control MS4*:
 - (i) Five (5) business days from the date the Department receives both a complete electronic version of the NOI (eNOI) and signed "*MS4* SWPPP Acceptance" form, or
 - (ii) Ten (10) business days from the date the Department receives both a complete paper version of the NOI and signed "MS4 SWPPP Acceptance" form.
- 4. Coverage under this permit authorizes stormwater *discharges* from only those areas of disturbance that are identified in the NOI. If an *owner or operator* wishes to have stormwater *discharges* from future or additional areas of disturbance authorized, they must submit a new NOI that addresses that phase of the development, unless otherwise notified by the Department. The *owner or operator* shall not *commence construction activity* on the future or additional areas until their authorization to *discharge* under this permit goes into effect in accordance with Part II.C. of this permit.

D. General Requirements For Owners or Operators With Permit Coverage

- The owner or operator shall ensure that the provisions of the SWPPP are implemented from the commencement of construction activity until all areas of disturbance have achieved final stabilization and the Notice of Termination ("NOT") has been submitted to the Department in accordance with Part V. of this permit. This includes any changes made to the SWPPP pursuant to Part III.A.4. of this permit.
- 2. The owner or operator shall maintain a copy of the General Permit (GP-0-20-001), NOI, NOI Acknowledgment Letter, SWPPP, MS4 SWPPP Acceptance form, inspection reports, responsible contractor's or subcontractor's certification statement (see Part III.A.6.), and all documentation necessary to demonstrate eligibility with this permit at the *construction site* until all disturbed areas have achieved *final stabilization* and the NOT has been submitted to the Department. The documents must be maintained in a secure location, such as a job trailer, on-site construction office, or mailbox with lock. The secure location must be accessible during normal business hours to an individual performing a compliance inspection.
- 3. The owner or operator of a construction activity shall not disturb greater than five (5) acres of soil at any one time without prior written authorization from the Department or, in areas under the jurisdiction of a *regulated, traditional land*

(Part II.D.3)

use control MS4, the regulated, traditional land use control MS4 (provided the regulated, traditional land use control MS4 is not the owner or operator of the construction activity). At a minimum, the owner or operator must comply with the following requirements in order to be authorized to disturb greater than five (5) acres of soil at any one time:

- a. The owner or operator shall have a qualified inspector conduct at least two (2) site inspections in accordance with Part IV.C. of this permit every seven (7) calendar days, for as long as greater than five (5) acres of soil remain disturbed. The two (2) inspections shall be separated by a minimum of two (2) full calendar days.
- b. In areas where soil disturbance activity has temporarily or permanently ceased, the application of soil stabilization measures must be initiated by the end of the next business day and completed within seven (7) days from the date the current soil disturbance activity ceased. The soil stabilization measures selected shall be in conformance with the technical standard, New York State Standards and Specifications for Erosion and Sediment Control, dated November 2016.
- c. The *owner or operator* shall prepare a phasing plan that defines maximum disturbed area per phase and shows required cuts and fills.
- d. The *owner or operator* shall install any additional site-specific practices needed to protect water quality.
- e. The *owner or operator* shall include the requirements above in their SWPPP.
- 4. In accordance with statute, regulations, and the terms and conditions of this permit, the Department may suspend or revoke an *owner's or operator's* coverage under this permit at any time if the Department determines that the SWPPP does not meet the permit requirements or consistent with Part VII.K..
- 5. Upon a finding of significant non-compliance with the practices described in the SWPPP or violation of this permit, the Department may order an immediate stop to all activity at the site until the non-compliance is remedied. The stop work order shall be in writing, describe the non-compliance in detail, and be sent to the *owner or operator*.
- 6. For *construction activities* that are subject to the requirements of a *regulated, traditional land use control MS4*, the *owner or operator* shall notify the

regulated, traditional land use control MS4 in writing of any planned amendments or modifications to the post-construction stormwater management practice component of the SWPPP required by Part III.A. 4. and 5. of this permit. Unless otherwise notified by the *regulated, traditional land use control MS4*, the owner or operator shall have the SWPPP amendments or modifications reviewed and accepted by the *regulated, traditional land use control MS4* prior to commencing construction of the post-construction stormwater management practice.

E. Permit Coverage for Discharges Authorized Under GP-0-15-002

 Upon renewal of SPDES General Permit for Stormwater Discharges from *Construction Activity* (Permit No. GP-0-15-002), an *owner or operator* of *a construction activity* with coverage under GP-0-15-002, as of the effective date of GP- 0-20-001, shall be authorized to *discharge* in accordance with GP- 0-20-001, unless otherwise notified by the Department.

An *owner or operator* may continue to implement the technical/design components of the post-construction stormwater management controls provided that such design was done in conformance with the technical standards in place at the time of initial project authorization. However, they must comply with the other, non-design provisions of GP-0-20-001.

F. Change of Owner or Operator

- 1. When property ownership changes or when there is a change in operational control over the construction plans and specifications, the original *owner or operator* must notify the new *owner or operator*, <u>in writing</u>, of the requirement to obtain permit coverage by submitting a NOI with the Department. For *construction activities* subject to the requirements of a *regulated*, *traditional land use control MS4*, the original *owner or operator* must also notify the MS4, in writing, of the change in ownership at least 30 calendar days prior to the change in ownership.
- 2. Once the new *owner or operator* obtains permit coverage, the original *owner or operator* shall then submit a completed NOT with the name and permit identification number of the new *owner or operator* to the Department at the address in Part II.B.1. of this permit. If the original *owner or operator* maintains ownership of a portion of the *construction activity* and will disturb soil, they must maintain their coverage under the permit.
- 3. Permit coverage for the new *owner or operator* will be effective as of the date the Department receives a complete NOI, provided the original *owner or*

(Part II.F.3)

operator was not subject to a sixty (60) business day authorization period that has not expired as of the date the Department receives the NOI from the new owner or operator.

Part III. STORMWATER POLLUTION PREVENTION PLAN (SWPPP)

A. General SWPPP Requirements

- A SWPPP shall be prepared and implemented by the owner or operator of each construction activity covered by this permit. The SWPPP must document the selection, design, installation, implementation and maintenance of the control measures and practices that will be used to meet the effluent limitations in Part I.B. of this permit and where applicable, the post-construction stormwater management practice requirements in Part I.C. of this permit. The SWPPP shall be prepared prior to the submittal of the NOI. The NOI shall be submitted to the Department prior to the commencement of construction activity. A copy of the completed, final NOI shall be included in the SWPPP.
- 2. The SWPPP shall describe the erosion and sediment control practices and where required, post-construction stormwater management practices that will be used and/or constructed to reduce the *pollutants* in stormwater *discharges* and to assure compliance with the terms and conditions of this permit. In addition, the SWPPP shall identify potential sources of pollution which may reasonably be expected to affect the quality of stormwater *discharges*.
- 3. All SWPPPs that require the post-construction stormwater management practice component shall be prepared by a *qualified professional* that is knowledgeable in the principles and practices of stormwater management and treatment.
- 4. The owner or operator must keep the SWPPP current so that it at all times accurately documents the erosion and sediment controls practices that are being used or will be used during construction, and all post-construction stormwater management practices that will be constructed on the site. At a minimum, the owner or operator shall amend the SWPPP, including construction drawings:
 - a. whenever the current provisions prove to be ineffective in minimizing *pollutants* in stormwater *discharges* from the site;

- b. whenever there is a change in design, construction, or operation at the *construction site* that has or could have an effect on the *discharge* of *pollutants*;
- c. to address issues or deficiencies identified during an inspection by the *qualified inspector*, the Department or other regulatory authority; and
- d. to document the final construction conditions.
- 5. The Department may notify the *owner or operator* at any time that the SWPPP does not meet one or more of the minimum requirements of this permit. The notification shall be in writing and identify the provisions of the SWPPP that require modification. Within fourteen (14) calendar days of such notification, or as otherwise indicated by the Department, the *owner or operator* shall make the required changes to the SWPPP and submit written notification to the Department that the changes have been made. If the *owner or operator* does not respond to the Department's comments in the specified time frame, the Department may suspend the *owner's or operator's* coverage under this permit or require the *owner or operator* to obtain coverage under an individual SPDES permit in accordance with Part II.D.4. of this permit.
- 6. Prior to the commencement of construction activity, the owner or operator must identify the contractor(s) and subcontractor(s) that will be responsible for installing, constructing, repairing, replacing, inspecting and maintaining the erosion and sediment control practices included in the SWPPP; and the contractor(s) and subcontractor(s) that will be responsible for constructing the post-construction stormwater management practices included in the SWPPP. The owner or operator shall have each of the contractors and subcontractors identify at least one person from their company that will be responsible for implementation of the SWPPP. This person shall be known as the *trained contractor*. The owner or operator shall ensure that at least one *trained contractor* is on site on a daily basis when soil disturbance activities are being performed.

The *owner or operator* shall have each of the contractors and subcontractors identified above sign a copy of the following certification statement below before they commence any *construction activity*:

"I hereby certify under penalty of law that I understand and agree to comply with the terms and conditions of the SWPPP and agree to implement any corrective actions identified by the *qualified inspector* during a site inspection. I also understand that the *owner or operator* must comply with

(Part III.A.6)

the terms and conditions of the most current version of the New York State Pollutant Discharge Elimination System ("SPDES") general permit for stormwater *discharges* from *construction activities* and that it is unlawful for any person to cause or contribute to a violation of *water quality standards*. Furthermore, I am aware that there are significant penalties for submitting false information, that I do not believe to be true, including the possibility of fine and imprisonment for knowing violations"

In addition to providing the certification statement above, the certification page must also identify the specific elements of the SWPPP that each contractor and subcontractor will be responsible for and include the name and title of the person providing the signature; the name and title of the *trained contractor* responsible for SWPPP implementation; the name, address and telephone number of the contracting firm; the address (or other identifying description) of the site; and the date the certification statement is signed. The *owner or operator* shall attach the certification statement(s) to the copy of the SWPPP that is maintained at the *construction site*. If new or additional contractors are hired to implement measures identified in the SWPPP after construction has commenced, they must also sign the certification statement and provide the information listed above.

 For projects where the Department requests a copy of the SWPPP or inspection reports, the owner or operator shall submit the documents in both electronic (PDF only) and paper format within five (5) business days, unless otherwise notified by the Department.

B. Required SWPPP Contents

- Erosion and sediment control component All SWPPPs prepared pursuant to this permit shall include erosion and sediment control practices designed in conformance with the technical standard, New York State Standards and Specifications for Erosion and Sediment Control, dated November 2016. Where erosion and sediment control practices are not designed in conformance with the design criteria included in the technical standard, the *owner or operator* must demonstrate *equivalence* to the technical standard. At a minimum, the erosion and sediment control component of the SWPPP shall include the following:
 - a. Background information about the scope of the project, including the location, type and size of project

- b. A site map/construction drawing(s) for the project, including a general location map. At a minimum, the site map shall show the total site area; all improvements; areas of disturbance; areas that will not be disturbed; existing vegetation; on-site and adjacent off-site surface water(s); floodplain/floodway boundaries; wetlands and drainage patterns that could be affected by the *construction activity*; existing and final contours; locations of different soil types with boundaries; material, waste, borrow or equipment storage areas located on adjacent properties; and location(s) of the stormwater *discharge*(s);
- c. A description of the soil(s) present at the site, including an identification of the Hydrologic Soil Group (HSG);
- d. A construction phasing plan and sequence of operations describing the intended order of *construction activities*, including clearing and grubbing, excavation and grading, utility and infrastructure installation and any other activity at the site that results in soil disturbance;
- e. A description of the minimum erosion and sediment control practices to be installed or implemented for each *construction activity* that will result in soil disturbance. Include a schedule that identifies the timing of initial placement or implementation of each erosion and sediment control practice and the minimum time frames that each practice should remain in place or be implemented;
- f. A temporary and permanent soil stabilization plan that meets the requirements of this general permit and the technical standard, New York State Standards and Specifications for Erosion and Sediment Control, dated November 2016, for each stage of the project, including initial land clearing and grubbing to project completion and achievement of *final stabilization*;
- g. A site map/construction drawing(s) showing the specific location(s), size(s), and length(s) of each erosion and sediment control practice;
- h. The dimensions, material specifications, installation details, and operation and maintenance requirements for all erosion and sediment control practices. Include the location and sizing of any temporary sediment basins and structural practices that will be used to divert flows from exposed soils;
- i. A maintenance inspection schedule for the contractor(s) identified in Part III.A.6. of this permit, to ensure continuous and effective operation of the erosion and sediment control practices. The maintenance inspection

(Part III.B.1.i)

schedule shall be in accordance with the requirements in the technical standard, New York State Standards and Specifications for Erosion and Sediment Control, dated November 2016;

- j. A description of the pollution prevention measures that will be used to control litter, construction chemicals and construction debris from becoming a *pollutant* source in the stormwater *discharges*;
- k. A description and location of any stormwater *discharges* associated with industrial activity other than construction at the site, including, but not limited to, stormwater *discharges* from asphalt plants and concrete plants located on the *construction site*; and
- Identification of any elements of the design that are not in conformance with the design criteria in the technical standard, New York State Standards and Specifications for Erosion and Sediment Control, dated November 2016. Include the reason for the deviation or alternative design and provide information which demonstrates that the deviation or alternative design is equivalent to the technical standard.
- 2. Post-construction stormwater management practice component The owner or operator of any construction project identified in Table 2 of Appendix B as needing post-construction stormwater management practices shall prepare a SWPPP that includes practices designed in conformance with the applicable sizing criteria in Part I.C.2.a., c. or d. of this permit and the performance criteria in the technical standard, New York State Stormwater Management Design Manual dated January 2015

Where post-construction stormwater management practices are not designed in conformance with the *performance criteria* in the technical standard, the *owner or operator* must include in the SWPPP the reason(s) for the deviation or alternative design and provide information which demonstrates that the deviation or alternative design is *equivalent* to the technical standard.

The post-construction stormwater management practice component of the SWPPP shall include the following:

 a. Identification of all post-construction stormwater management practices to be constructed as part of the project. Include the dimensions, material specifications and installation details for each post-construction stormwater management practice;

- b. A site map/construction drawing(s) showing the specific location and size of each post-construction stormwater management practice;
- c. A Stormwater Modeling and Analysis Report that includes:
 - Map(s) showing pre-development conditions, including watershed/subcatchments boundaries, flow paths/routing, and design points;
 - Map(s) showing post-development conditions, including watershed/subcatchments boundaries, flow paths/routing, design points and post-construction stormwater management practices;
 - (iii) Results of stormwater modeling (i.e. hydrology and hydraulic analysis) for the required storm events. Include supporting calculations (model runs), methodology, and a summary table that compares pre and postdevelopment runoff rates and volumes for the different storm events;
 - (iv) Summary table, with supporting calculations, which demonstrates that each post-construction stormwater management practice has been designed in conformance with the *sizing criteria* included in the Design Manual;
 - (v) Identification of any *sizing criteria* that is not required based on the requirements included in Part I.C. of this permit; and
 - (vi) Identification of any elements of the design that are not in conformance with the *performance criteria* in the Design Manual. Include the reason(s) for the deviation or alternative design and provide information which demonstrates that the deviation or alternative design is *equivalent* to the Design Manual;
- d. Soil testing results and locations (test pits, borings);
- e. Infiltration test results, when required; and
- f. An operations and maintenance plan that includes inspection and maintenance schedules and actions to ensure continuous and effective operation of each post-construction stormwater management practice. The plan shall identify the entity that will be responsible for the long term operation and maintenance of each practice.

(Part III.B.3)

3. Enhanced Phosphorus Removal Standards - All construction projects identified in Table 2 of Appendix B that are located in the watersheds identified in Appendix C shall prepare a SWPPP that includes post-construction stormwater management practices designed in conformance with the applicable *sizing criteria* in Part I.C.2. b., c. or d. of this permit and the *performance criteria*, Enhanced Phosphorus Removal Standards included in the Design Manual. At a minimum, the post-construction stormwater management practice component of the SWPPP shall include items 2.a - 2.f. above.

C. Required SWPPP Components by Project Type

Unless otherwise notified by the Department, *owners or operators* of *construction activities* identified in Table 1 of Appendix B are required to prepare a SWPPP that only includes erosion and sediment control practices designed in conformance with Part III.B.1 of this permit. *Owners or operators* of the *construction activities* identified in Table 2 of Appendix B shall prepare a SWPPP that also includes post-construction stormwater management practices designed in conformance with Part III.B.2 or 3 of this permit.

Part IV. INSPECTION AND MAINTENANCE REQUIREMENTS

A. General Construction Site Inspection and Maintenance Requirements

- 1. The *owner or operator* must ensure that all erosion and sediment control practices (including pollution prevention measures) and all post-construction stormwater management practices identified in the SWPPP are inspected and maintained in accordance with Part IV.B. and C. of this permit.
- 2. The terms of this permit shall not be construed to prohibit the State of New York from exercising any authority pursuant to the ECL, common law or federal law, or prohibit New York State from taking any measures, whether civil or criminal, to prevent violations of the laws of the State of New York or protect the public health and safety and/or the environment.

B. Contractor Maintenance Inspection Requirements

1. The owner or operator of each construction activity identified in Tables 1 and 2 of Appendix B shall have a *trained contractor* inspect the erosion and sediment control practices and pollution prevention measures being implemented within the active work area daily to ensure that they are being maintained in effective operating condition at all times. If deficiencies are identified, the contractor shall

begin implementing corrective actions within one business day and shall complete the corrective actions in a reasonable time frame.

- 2. For construction sites where soil disturbance activities have been temporarily suspended (e.g. winter shutdown) and *temporary stabilization* measures have been applied to all disturbed areas, the *trained contractor* can stop conducting the maintenance inspections. The *trained contractor* shall begin conducting the maintenance inspections in accordance with Part IV.B.1. of this permit as soon as soil disturbance activities resume.
- 3. For construction sites where soil disturbance activities have been shut down with partial project completion, the *trained contractor* can stop conducting the maintenance inspections if all areas disturbed as of the project shutdown date have achieved *final stabilization* and all post-construction stormwater management practices required for the completed portion of the project have been constructed in conformance with the SWPPP and are operational.

C. Qualified Inspector Inspection Requirements

The *owner or operator* shall have a *qualified inspector* conduct site inspections in conformance with the following requirements:

[Note: The *trained contractor* identified in Part III.A.6. and IV.B. of this permit **cannot** conduct the *qualified inspector* site inspections unless they meet the *qualified inspector* qualifications included in Appendix A. In order to perform these inspections, the *trained contractor* would have to be a:

- licensed Professional Engineer,
- Certified Professional in Erosion and Sediment Control (CPESC),
- New York State Erosion and Sediment Control Certificate Program holder
- Registered Landscape Architect, or
- someone working under the direct supervision of, and at the same company as, the licensed Professional Engineer or Registered Landscape Architect, provided they have received four (4) hours of Department endorsed training in proper erosion and sediment control principles from a Soil and Water Conservation District, or other Department endorsed entity].
- 1. A *qualified inspector* shall conduct site inspections for all *construction activities* identified in Tables 1 and 2 of Appendix B, <u>with the exception of</u>:
 - a. the construction of a single family residential subdivision with 25% or less *impervious cover* at total site build-out that involves a soil disturbance of one (1) or more acres of land but less than five (5) acres and is <u>not</u> located

in one of the watersheds listed in Appendix C and <u>not</u> directly discharging to one of the 303(d) segments listed in Appendix E;

- b. the construction of a single family home that involves a soil disturbance of one (1) or more acres of land but less than five (5) acres and is <u>not</u> located in one of the watersheds listed in Appendix C and <u>not</u> directly discharging to one of the 303(d) segments listed in Appendix E;
- c. construction on agricultural property that involves a soil disturbance of one (1) or more acres of land but less than five (5) acres; and
- d. *construction activities* located in the watersheds identified in Appendix D that involve soil disturbances between five thousand (5,000) square feet and one (1) acre of land.
- 2. Unless otherwise notified by the Department, the *qualified inspector* shall conduct site inspections in accordance with the following timetable:
 - a. For construction sites where soil disturbance activities are on-going, the *qualified inspector* shall conduct a site inspection at least once every seven (7) calendar days.
 - b. For construction sites where soil disturbance activities are on-going and the owner or operator has received authorization in accordance with Part II.D.3 to disturb greater than five (5) acres of soil at any one time, the *qualified inspector* shall conduct at least two (2) site inspections every seven (7) calendar days. The two (2) inspections shall be separated by a minimum of two (2) full calendar days.
 - c. For construction sites where soil disturbance activities have been temporarily suspended (e.g. winter shutdown) and *temporary stabilization* measures have been applied to all disturbed areas, the *qualified inspector* shall conduct a site inspection at least once every thirty (30) calendar days. The *owner or operator* shall notify the DOW Water (SPDES) Program contact at the Regional Office (see contact information in Appendix F) or, in areas under the jurisdiction of a *regulated, traditional land use control MS4*, the *regulated, traditional land use control MS4* (provided the *regulated, traditional land use control MS4* is not the *owner or operator* of the *construction activity*) in writing prior to reducing the frequency of inspections.

- d. For construction sites where soil disturbance activities have been shut down with partial project completion, the *gualified inspector* can stop conducting inspections if all areas disturbed as of the project shutdown date have achieved final stabilization and all post-construction stormwater management practices required for the completed portion of the project have been constructed in conformance with the SWPPP and are operational. The owner or operator shall notify the DOW Water (SPDES) Program contact at the Regional Office (see contact information in Appendix F) or, in areas under the jurisdiction of a regulated, traditional land use control MS4, the regulated, traditional land use control MS4 (provided the regulated, traditional land use control MS4 is not the owner or operator of the *construction activity*) in writing prior to the shutdown. If soil disturbance activities are not resumed within 2 years from the date of shutdown, the owner or operator shall have the qualified inspector perform a final inspection and certify that all disturbed areas have achieved final stabilization, and all temporary, structural erosion and sediment control measures have been removed; and that all post-construction stormwater management practices have been constructed in conformance with the SWPPP by signing the "Final Stabilization" and "Post-Construction Stormwater Management Practice" certification statements on the NOT. The owner or operator shall then submit the completed NOT form to the address in Part II.B.1 of this permit.
- e. For construction sites that directly *discharge* to one of the 303(d) segments listed in Appendix E or is located in one of the watersheds listed in Appendix C, the *qualified inspector* shall conduct at least two (2) site inspections every seven (7) calendar days. The two (2) inspections shall be separated by a minimum of two (2) full calendar days.
- 3. At a minimum, the *qualified inspector* shall inspect all erosion and sediment control practices and pollution prevention measures to ensure integrity and effectiveness, all post-construction stormwater management practices under construction to ensure that they are constructed in conformance with the SWPPP, all areas of disturbance that have not achieved *final stabilization*, all points of *discharge* to natural surface waterbodies located within, or immediately adjacent to, the property boundaries of the *construction site*, and all points of *discharge* from the *construction site*.
- 4. The *qualified inspector* shall prepare an inspection report subsequent to each and every inspection. At a minimum, the inspection report shall include and/or address the following:

- a. Date and time of inspection;
- b. Name and title of person(s) performing inspection;
- c. A description of the weather and soil conditions (e.g. dry, wet, saturated) at the time of the inspection;
- d. A description of the condition of the runoff at all points of *discharge* from the *construction site*. This shall include identification of any *discharges* of sediment from the *construction site*. Include *discharges* from conveyance systems (i.e. pipes, culverts, ditches, etc.) and overland flow;
- e. A description of the condition of all natural surface waterbodies located within, or immediately adjacent to, the property boundaries of the *construction site* which receive runoff from disturbed areas. This shall include identification of any *discharges* of sediment to the surface waterbody;
- f. Identification of all erosion and sediment control practices and pollution prevention measures that need repair or maintenance;
- Identification of all erosion and sediment control practices and pollution prevention measures that were not installed properly or are not functioning as designed and need to be reinstalled or replaced;
- Description and sketch of areas with active soil disturbance activity, areas that have been disturbed but are inactive at the time of the inspection, and areas that have been stabilized (temporary and/or final) since the last inspection;
- Current phase of construction of all post-construction stormwater management practices and identification of all construction that is not in conformance with the SWPPP and technical standards;
- j. Corrective action(s) that must be taken to install, repair, replace or maintain erosion and sediment control practices and pollution prevention measures; and to correct deficiencies identified with the construction of the postconstruction stormwater management practice(s);
- k. Identification and status of all corrective actions that were required by previous inspection; and

- I. Digital photographs, with date stamp, that clearly show the condition of all practices that have been identified as needing corrective actions. The *qualified inspector* shall attach paper color copies of the digital photographs to the inspection report being maintained onsite within seven (7) calendar days of the date of the inspection. The *qualified inspector* shall also take digital photographs, with date stamp, that clearly show the condition of the practice(s) after the corrective action has been completed. The *qualified inspector* shall attach paper color copies of the digital photographs to the inspection report that documents the completion of the corrective action work within seven (7) calendar days of that inspection.
- 5. Within one business day of the completion of an inspection, the *qualified inspector* shall notify the *owner or operator* and appropriate contractor or subcontractor identified in Part III.A.6. of this permit of any corrective actions that need to be taken. The contractor or subcontractor shall begin implementing the corrective actions within one business day of this notification and shall complete the corrective actions in a reasonable time frame.
- 6. All inspection reports shall be signed by the *qualified inspector*. Pursuant to Part II.D.2. of this permit, the inspection reports shall be maintained on site with the SWPPP.

Part V. TERMINATION OF PERMIT COVERAGE

A. Termination of Permit Coverage

- 1. An owner or operator that is eligible to terminate coverage under this permit must submit a completed NOT form to the address in Part II.B.1 of this permit. The NOT form shall be one which is associated with this permit, signed in accordance with Part VII.H of this permit.
- 2. An *owner or operator* may terminate coverage when one or more the following conditions have been met:
 - a. Total project completion All *construction activity* identified in the SWPPP has been completed; <u>and</u> all areas of disturbance have achieved *final stabilization*; <u>and</u> all temporary, structural erosion and sediment control measures have been removed; <u>and</u> all post-construction stormwater management practices have been constructed in conformance with the SWPPP and are operational;

- b. Planned shutdown with partial project completion All soil disturbance activities have ceased; and all areas disturbed as of the project shutdown date have achieved *final stabilization*; and all temporary, structural erosion and sediment control measures have been removed; and all postconstruction stormwater management practices required for the completed portion of the project have been constructed in conformance with the SWPPP and are operational;
- c. A new owner or operator has obtained coverage under this permit in accordance with Part II.F. of this permit.
- d. The *owner or operator* obtains coverage under an alternative SPDES general permit or an individual SPDES permit.
- 3. For *construction activities* meeting subdivision 2a. or 2b. of this Part, the *owner or operator* shall have the *qualified inspector* perform a final site inspection prior to submitting the NOT. The *qualified inspector* shall, by signing the "*Final Stabilization*" and "Post-Construction Stormwater Management Practice certification statements on the NOT, certify that all the requirements in Part V.A.2.a. or b. of this permit have been achieved.
- 4. For construction activities that are subject to the requirements of a regulated, traditional land use control MS4 and meet subdivision 2a. or 2b. of this Part, the owner or operator shall have the regulated, traditional land use control MS4 sign the "MS4 Acceptance" statement on the NOT in accordance with the requirements in Part VII.H. of this permit. The regulated, traditional land use control MS4 official, by signing this statement, has determined that it is acceptable for the owner or operator to submit the NOT in accordance with the requirements of this Part. The regulated, traditional land use control MS4 can make this determination by performing a final site inspection themselves or by accepting the qualified inspector's final site inspection certification(s) required in Part V.A.3. of this permit.
- 5. For *construction activities* that require post-construction stormwater management practices and meet subdivision 2a. of this Part, the *owner or operator* must, prior to submitting the NOT, ensure one of the following:
 - a. the post-construction stormwater management practice(s) and any right-ofway(s) needed to maintain such practice(s) have been deeded to the municipality in which the practice(s) is located,

- b. an executed maintenance agreement is in place with the municipality that will maintain the post-construction stormwater management practice(s),
- c. for post-construction stormwater management practices that are privately owned, the *owner or operator* has a mechanism in place that requires operation and maintenance of the practice(s) in accordance with the operation and maintenance plan, such as a deed covenant in the *owner or operator*'s deed of record,
- d. for post-construction stormwater management practices that are owned by a public or private institution (e.g. school, university, hospital), government agency or authority, or public utility; the *owner or operator* has policy and procedures in place that ensures operation and maintenance of the practices in accordance with the operation and maintenance plan.

Part VI. REPORTING AND RETENTION RECORDS

A. Record Retention

The owner or operator shall retain a copy of the NOI, NOI

Acknowledgment Letter, SWPPP, MS4 SWPPP Acceptance form and any inspection reports that were prepared in conjunction with this permit for a period of at least five (5) years from the date that the Department receives a complete NOT submitted in accordance with Part V. of this general permit.

B. Addresses

With the exception of the NOI, NOT, and MS4 SWPPP Acceptance form (which must be submitted to the address referenced in Part II.B.1 of this permit), all written correspondence requested by the Department, including individual permit applications, shall be sent to the address of the appropriate DOW Water (SPDES) Program contact at the Regional Office listed in Appendix F.

Part VII. STANDARD PERMIT CONDITIONS

A. Duty to Comply

The *owner or operator* must comply with all conditions of this permit. All contractors and subcontractors associated with the project must comply with the terms of the SWPPP. Any non-compliance with this permit constitutes a violation of the Clean Water

(Part VII.A)

Act (CWA) and the ECL and is grounds for an enforcement action against the *owner or operator* and/or the contractor/subcontractor; permit revocation, suspension or modification; or denial of a permit renewal application. Upon a finding of significant non-compliance with this permit or the applicable SWPPP, the Department may order an immediate stop to all *construction activity* at the site until the non-compliance is remedied. The stop work order shall be in writing, shall describe the non-compliance in detail, and shall be sent to the *owner or operator*.

If any human remains or archaeological remains are encountered during excavation, the *owner or operator* must immediately cease, or cause to cease, all *construction activity* in the area of the remains and notify the appropriate Regional Water Engineer (RWE). *Construction activity* shall not resume until written permission to do so has been received from the RWE.

B. Continuation of the Expired General Permit

This permit expires five (5) years from the effective date. If a new general permit is not issued prior to the expiration of this general permit, an *owner or operator* with coverage under this permit may continue to operate and *discharge* in accordance with the terms and conditions of this general permit, if it is extended pursuant to the State Administrative Procedure Act and 6 NYCRR Part 621, until a new general permit is issued.

C. Enforcement

Failure of the *owner or operator*, its contractors, subcontractors, agents and/or assigns to strictly adhere to any of the permit requirements contained herein shall constitute a violation of this permit. There are substantial criminal, civil, and administrative penalties associated with violating the provisions of this permit. Fines of up to \$37,500 per day for each violation and imprisonment for up to fifteen (15) years may be assessed depending upon the nature and degree of the offense.

D. Need to Halt or Reduce Activity Not a Defense

It shall not be a defense for an *owner or operator* in an enforcement action that it would have been necessary to halt or reduce the *construction activity* in order to maintain compliance with the conditions of this permit.

E. Duty to Mitigate

The *owner or operator* and its contractors and subcontractors shall take all reasonable steps to *minimize* or prevent any *discharge* in violation of this permit which has a reasonable likelihood of adversely affecting human health or the environment.

F. Duty to Provide Information

The *owner or operator* shall furnish to the Department, within a reasonable specified time period of a written request, all documentation necessary to demonstrate eligibility and any information to determine compliance with this permit or to determine whether cause exists for modifying or revoking this permit, or suspending or denying coverage under this permit, in accordance with the terms and conditions of this permit. The NOI, SWPPP and inspection reports required by this permit are public documents that the *owner or operator* must make available for review and copying by any person within five (5) business days of the *owner or operator* receiving a written request by any such person to review these documents. Copying of documents will be done at the requester's expense.

G. Other Information

When the *owner or operator* becomes aware that they failed to submit any relevant facts, or submitted incorrect information in the NOI or in any of the documents required by this permit, or have made substantive revisions to the SWPPP (e.g. the scope of the project changes significantly, the type of post-construction stormwater management practice(s) changes, there is a reduction in the sizing of the post-construction stormwater management practice, or there is an increase in the disturbance area or *impervious area*), which were not reflected in the original NOI submitted to the Department, they shall promptly submit such facts or information to the Department using the contact information in Part II.A. of this permit. Failure of the *owner or operator* to correct or supplement any relevant facts within five (5) business days of becoming aware of the deficiency shall constitute a violation of this permit.

H. Signatory Requirements

- 1. All NOIs and NOTs shall be signed as follows:
 - a. For a corporation these forms shall be signed by a responsible corporate officer. For the purpose of this section, a responsible corporate officer means:

33

(Part VII.H.1.a)

- a president, secretary, treasurer, or vice-president of the corporation in charge of a principal business function, or any other person who performs similar policy or decision-making functions for the corporation; or
- (ii) the manager of one or more manufacturing, production or operating facilities, provided the manager is authorized to make management decisions which govern the operation of the regulated facility including having the explicit or implicit duty of making major capital investment recommendations, and initiating and directing other comprehensive measures to assure long term environmental compliance with environmental laws and regulations; the manager can ensure that the necessary systems are established or actions taken to gather complete and accurate information for permit application requirements; and where authority to sign documents has been assigned or delegated to the manager in accordance with corporate procedures;
- b. For a partnership or sole proprietorship these forms shall be signed by a general partner or the proprietor, respectively; or
- c. For a municipality, State, Federal, or other public agency these forms shall be signed by either a principal executive officer or ranking elected official. For purposes of this section, a principal executive officer of a Federal agency includes:
 - (i) the chief executive officer of the agency, or
 - (ii) a senior executive officer having responsibility for the overall operations of a principal geographic unit of the agency (e.g., Regional Administrators of EPA).
- 2. The SWPPP and other information requested by the Department shall be signed by a person described in Part VII.H.1. of this permit or by a duly authorized representative of that person. A person is a duly authorized representative only if:
 - a. The authorization is made in writing by a person described in Part VII.H.1. of this permit;
 - b. The authorization specifies either an individual or a position having responsibility for the overall operation of the regulated facility or activity, such as the position of plant manager, operator of a well or a well field,

superintendent, position of *equivalent* responsibility, or an individual or position having overall responsibility for environmental matters for the company. (A duly authorized representative may thus be either a named individual or any individual occupying a named position) and,

- c. The written authorization shall include the name, title and signature of the authorized representative and be attached to the SWPPP.
- 3. All inspection reports shall be signed by the *qualified inspector* that performs the inspection.
- 4. The MS4 SWPPP Acceptance form shall be signed by the principal executive officer or ranking elected official from the *regulated, traditional land use control MS4*, or by a duly authorized representative of that person.

It shall constitute a permit violation if an incorrect and/or improper signatory authorizes any required forms, SWPPP and/or inspection reports.

I. Property Rights

The issuance of this permit does not convey any property rights of any sort, nor any exclusive privileges, nor does it authorize any injury to private property nor any invasion of personal rights, nor any infringement of Federal, State or local laws or regulations. *Owners or operators* must obtain any applicable conveyances, easements, licenses and/or access to real property prior to *commencing construction activity*.

J. Severability

The provisions of this permit are severable, and if any provision of this permit, or the application of any provision of this permit to any circumstance, is held invalid, the application of such provision to other circumstances, and the remainder of this permit shall not be affected thereby.

K. Requirement to Obtain Coverage Under an Alternative Permit

1. The Department may require any owner or operator authorized by this permit to apply for and/or obtain either an individual SPDES permit or another SPDES general permit. When the Department requires any discharger authorized by a general permit to apply for an individual SPDES permit, it shall notify the discharger in writing that a permit application is required. This notice shall

(Part VII.K.1)

include a brief statement of the reasons for this decision, an application form, a statement setting a time frame for the owner or operator to file the application for an individual SPDES permit, and a deadline, not sooner than 180 days from owner or operator receipt of the notification letter, whereby the authorization to discharge under this general permit shall be terminated. Applications must be submitted to the appropriate Permit Administrator at the Regional Office. The Department may grant additional time upon demonstration, to the satisfaction of the Department, that additional time to apply for an alternative authorization is necessary or where the Department has not provided a permit determination in accordance with Part 621 of this Title.

2. When an individual SPDES permit is issued to a discharger authorized to *discharge* under a general SPDES permit for the same *discharge*(s), the general permit authorization for outfalls authorized under the individual SPDES permit is automatically terminated on the effective date of the individual permit unless termination is earlier in accordance with 6 NYCRR Part 750.

L. Proper Operation and Maintenance

The owner or operator shall at all times properly operate and maintain all facilities and systems of treatment and control (and related appurtenances) which are installed or used by the owner or operator to achieve compliance with the conditions of this permit and with the requirements of the SWPPP.

M. Inspection and Entry

The owner or operator shall allow an authorized representative of the Department, EPA, applicable county health department, or, in the case of a *construction site* which *discharges* through an *MS4*, an authorized representative of the *MS4* receiving the discharge, upon the presentation of credentials and other documents as may be required by law, to:

- 1. Enter upon the owner's or operator's premises where a regulated facility or activity is located or conducted or where records must be kept under the conditions of this permit;
- 2. Have access to and copy at reasonable times, any records that must be kept under the conditions of this permit; and

- 3. Inspect at reasonable times any facilities or equipment (including monitoring and control equipment), practices or operations regulated or required by this permit.
- 4. Sample or monitor at reasonable times, for purposes of assuring permit compliance or as otherwise authorized by the Act or ECL, any substances or parameters at any location.

N. Permit Actions

This permit may, at any time, be modified, suspended, revoked, or renewed by the Department in accordance with 6 NYCRR Part 621. The filing of a request by the *owner or operator* for a permit modification, revocation and reissuance, termination, a notification of planned changes or anticipated noncompliance does not limit, diminish and/or stay compliance with any terms of this permit.

O. Definitions

Definitions of key terms are included in Appendix A of this permit.

P. Re-Opener Clause

- If there is evidence indicating potential or realized impacts on water quality due to any stormwater discharge associated with construction activity covered by this permit, the owner or operator of such discharge may be required to obtain an individual permit or alternative general permit in accordance with Part VII.K. of this permit or the permit may be modified to include different limitations and/or requirements.
- 2. Any Department initiated permit modification, suspension or revocation will be conducted in accordance with 6 NYCRR Part 621, 6 NYCRR 750-1.18, and 6 NYCRR 750-1.20.

Q. Penalties for Falsification of Forms and Reports

In accordance with 6NYCRR Part 750-2.4 and 750-2.5, any person who knowingly makes any false material statement, representation, or certification in any application, record, report or other document filed or required to be maintained under this permit, including reports of compliance or noncompliance shall, upon conviction, be punished in accordance with ECL §71-1933 and or Articles 175 and 210 of the New York State Penal Law.

(Part VII.R)

R. Other Permits

Nothing in this permit relieves the *owner or operator* from a requirement to obtain any other permits required by law.

APPENDIX A – Acronyms and Definitions

Acronyms

APO – Agency Preservation Officer

BMP – Best Management Practice

CPESC - Certified Professional in Erosion and Sediment Control

Cpv – Channel Protection Volume

CWA – Clean Water Act (or the Federal Water Pollution Control Act, 33 U.S.C. §1251 et seq)

DOW – Division of Water

EAF – Environmental Assessment Form

ECL - Environmental Conservation Law

EPA – U. S. Environmental Protection Agency

HSG – Hydrologic Soil Group

MS4 – Municipal Separate Storm Sewer System

NOI – Notice of Intent

NOT – Notice of Termination

NPDES – National Pollutant Discharge Elimination System

OPRHP – Office of Parks, Recreation and Historic Places

Qf – Extreme Flood

Qp – Overbank Flood

RRv – Runoff Reduction Volume

RWE -- Regional Water Engineer

SEQR – State Environmental Quality Review

SEQRA - State Environmental Quality Review Act

SHPA – State Historic Preservation Act

SPDES – State Pollutant Discharge Elimination System

SWPPP – Stormwater Pollution Prevention Plan

TMDL – Total Maximum Daily Load

UPA – Uniform Procedures Act

USDA – United States Department of Agriculture

WQv – Water Quality Volume

Definitions

<u>All definitions in this section are solely for the purposes of this permit.</u> <u>Agricultural Building</u> – a structure designed and constructed to house farm implements, hay, grain, poultry, livestock or other horticultural products; excluding any structure designed, constructed or used, in whole or in part, for human habitation, as a place of employment where agricultural products are processed, treated or packaged, or as a place used by the public.

Agricultural Property --means the land for construction of a barn, *agricultural building*, silo, stockyard, pen or other structural practices identified in Table II in the "Agricultural Management Practices Catalog for Nonpoint Source Pollution in New York State" prepared by the Department in cooperation with agencies of New York Nonpoint Source Coordinating Committee (dated June 2007).

Alter Hydrology from Pre to Post-Development Conditions - means the postdevelopment peak flow rate(s) has increased by more than 5% of the pre-developed condition for the design storm of interest (e.g. 10 yr and 100 yr).

Combined Sewer - means a sewer that is designed to collect and convey both "sewage" and "stormwater".

Commence (Commencement of) Construction Activities - means the initial disturbance of soils associated with clearing, grading or excavation activities; or other construction related activities that disturb or expose soils such as demolition, stockpiling of fill material, and the initial installation of erosion and sediment control practices required in the SWPPP. See definition for "*Construction Activity(ies)*" also.

Construction Activity(ies) - means any clearing, grading, excavation, filling, demolition or stockpiling activities that result in soil disturbance. Clearing activities can include, but are not limited to, logging equipment operation, the cutting and skidding of trees, stump removal and/or brush root removal. Construction activity does not include routine maintenance that is performed to maintain the original line and grade, hydraulic capacity, or original purpose of a facility.

Construction Site – means the land area where *construction activity(ies)* will occur. See definition for "*Commence (Commencement of) Construction Activities*" and "*Larger Common Plan of Development or Sale*" also.

Dewatering – means the act of draining rainwater and/or groundwater from building foundations, vaults or excavations/trenches.

Direct Discharge (to a specific surface waterbody) - means that runoff flows from a *construction site* by overland flow and the first point of discharge is the specific surface waterbody, or runoff flows from a *construction site* to a separate storm sewer system

Appendix A

and the first point of discharge from the separate storm sewer system is the specific surface waterbody.

Discharge(s) - means any addition of any pollutant to waters of the State through an outlet or *point source*.

Embankment --means an earthen or rock slope that supports a road/highway.

Endangered or Threatened Species – see 6 NYCRR Part 182 of the Department's rules and regulations for definition of terms and requirements.

Environmental Conservation Law (ECL) - means chapter 43-B of the Consolidated Laws of the State of New York, entitled the Environmental Conservation Law.

Equivalent (Equivalence) – means that the practice or measure meets all the performance, longevity, maintenance, and safety objectives of the technical standard and will provide an equal or greater degree of water quality protection.

Final Stabilization - means that all soil disturbance activities have ceased and a uniform, perennial vegetative cover with a density of eighty (80) percent over the entire pervious surface has been established; or other equivalent stabilization measures, such as permanent landscape mulches, rock rip-rap or washed/crushed stone have been applied on all disturbed areas that are not covered by permanent structures, concrete or pavement.

General SPDES permit - means a SPDES permit issued pursuant to 6 NYCRR Part 750-1.21 and Section 70-0117 of the ECL authorizing a category of discharges.

Groundwater(s) - means waters in the saturated zone. The saturated zone is a subsurface zone in which all the interstices are filled with water under pressure greater than that of the atmosphere. Although the zone may contain gas-filled interstices or interstices filled with fluids other than water, it is still considered saturated.

Historic Property – means any building, structure, site, object or district that is listed on the State or National Registers of Historic Places or is determined to be eligible for listing on the State or National Registers of Historic Places.

Impervious Area (Cover) - means all impermeable surfaces that cannot effectively infiltrate rainfall. This includes paved, concrete and gravel surfaces (i.e. parking lots, driveways, roads, runways and sidewalks); building rooftops and miscellaneous impermeable structures such as patios, pools, and sheds.

Infeasible – means not technologically possible, or not economically practicable and achievable in light of best industry practices.

41

Appendix A

Larger Common Plan of Development or Sale - means a contiguous area where multiple separate and distinct *construction activities* are occurring, or will occur, under one plan. The term "plan" in "larger common plan of development or sale" is broadly defined as any announcement or piece of documentation (including a sign, public notice or hearing, marketing plan, advertisement, drawing, permit application, State Environmental Quality Review Act (SEQRA) environmental assessment form or other documents, zoning request, computer design, etc.) or physical demarcation (including boundary signs, lot stakes, surveyor markings, etc.) indicating that *construction activities* may occur on a specific plot.

For discrete construction projects that are located within a larger common plan of development or sale that are at least 1/4 mile apart, each project can be treated as a separate plan of development or sale provided any interconnecting road, pipeline or utility project that is part of the same "common plan" is not concurrently being disturbed.

Minimize – means reduce and/or eliminate to the extent achievable using control measures (including best management practices) that are technologically available and economically practicable and achievable in light of best industry practices.

Municipal Separate Storm Sewer (MS4) - a conveyance or system of conveyances (including roads with drainage systems, municipal streets, catch basins, curbs, gutters, ditches, man-made channels, or storm drains):

- (i) Owned or operated by a State, city, town, borough, county, parish, district, association, or other public body (created by or pursuant to State law) having jurisdiction over disposal of sewage, industrial wastes, stormwater, or other wastes, including special districts under State law such as a sewer district, flood control district or drainage district, or similar entity, or an Indian tribe or an authorized Indian tribal organization, or a designated and approved management agency under section 208 of the CWA that discharges to surface waters of the State;
- (ii) Designed or used for collecting or conveying stormwater;
- (iii) Which is not a combined sewer, and
- (iv) Which is not part of a Publicly Owned Treatment Works (POTW) as defined at 40 CFR 122.2.

National Pollutant Discharge Elimination System (NPDES) - means the national system for the issuance of wastewater and stormwater permits under the Federal Water Pollution Control Act (Clean Water Act).

Natural Buffer – means an undisturbed area with natural cover running along a surface water (e.g. wetland, stream, river, lake, etc.).

New Development – means any land disturbance that does not meet the definition of Redevelopment Activity included in this appendix.

New York State Erosion and Sediment Control Certificate Program – a certificate program that establishes and maintains a process to identify and recognize individuals who are capable of developing, designing, inspecting and maintaining erosion and sediment control plans on projects that disturb soils in New York State. The certificate program is administered by the New York State Conservation District Employees Association.

NOI Acknowledgment Letter - means the letter that the Department sends to an owner or operator to acknowledge the Department's receipt and acceptance of a complete Notice of Intent. This letter documents the owner's or operator's authorization to discharge in accordance with the general permit for stormwater discharges from *construction activity*.

Nonpoint Source - means any source of water pollution or pollutants which is not a discrete conveyance or *point source* permitted pursuant to Title 7 or 8 of Article 17 of the Environmental Conservation Law (see ECL Section 17-1403).

Overbank –means flow events that exceed the capacity of the stream channel and spill out into the adjacent floodplain.

Owner or Operator - means the person, persons or legal entity which owns or leases the property on which the *construction activity* is occurring; an entity that has operational control over the construction plans and specifications, including the ability to make modifications to the plans and specifications; and/or an entity that has day-to-day operational control of those activities at a project that are necessary to ensure compliance with the permit conditions.

Performance Criteria – means the design criteria listed under the "Required Elements" sections in Chapters 5, 6 and 10 of the technical standard, New York State Stormwater Management Design Manual, dated January 2015. It does not include the Sizing Criteria (i.e. WQv, RRv, Cpv, Qp and Qf) in Part I.C.2. of the permit.

Point Source - means any discernible, confined and discrete conveyance, including but not limited to any pipe, ditch, channel, tunnel, conduit, well, discrete fissure, container, rolling stock, concentrated animal feeding operation, vessel or other floating craft, or landfill leachate collection system from which *pollutants* are or may be discharged.

Pollutant - means dredged spoil, filter backwash, solid waste, incinerator residue, sewage, garbage, sewage sludge, munitions, chemical wastes, biological materials, radioactive materials, heat, wrecked or discarded equipment, rock, sand and industrial, municipal, agricultural waste and ballast discharged into water; which may cause or might reasonably be expected to cause pollution of the waters of the state in contravention of the standards or guidance values adopted as provided in 6 NYCRR Parts 700 et seq.

Qualified Inspector - means a person that is knowledgeable in the principles and practices of erosion and sediment control, such as a licensed Professional Engineer, Certified Professional in Erosion and Sediment Control (CPESC), Registered Landscape Architect, New York State Erosion and Sediment Control Certificate Program holder or other Department endorsed individual(s).

It can also mean someone working under the direct supervision of, and at the same company as, the licensed Professional Engineer or Registered Landscape Architect, provided that person has training in the principles and practices of erosion and sediment control. Training in the principles and practices of erosion and sediment control means that the individual working under the direct supervision of the licensed Professional Engineer or Registered Landscape Architect has received four (4) hours of Department endorsed training in proper erosion and sediment control principles from a Soil and Water Conservation District, or other Department endorsed entity. After receiving the initial training, the individual working under the direct supervision of the licensed Professional Engineer or Registered Landscape Architect has received four (4) hours of the licensed water Conservation District, or other Department endorsed entity. After receiving the initial training, the individual working under the direct supervision of the licensed Professional Engineer or Registered Landscape Architect supervision of the licensed Professional Soil and Water Conservation District, or other Department endorsed entity. After receiving the initial training, the individual working under the direct supervision of the licensed Professional Engineer or Registered Landscape Architect shall receive four (4) hours of training every three (3) years.

It can also mean a person that meets the *Qualified Professional* qualifications in addition to the *Qualified Inspector* qualifications.

Note: Inspections of any post-construction stormwater management practices that include structural components, such as a dam for an impoundment, shall be performed by a licensed Professional Engineer.

Qualified Professional - means a person that is knowledgeable in the principles and practices of stormwater management and treatment, such as a licensed Professional Engineer, Registered Landscape Architect or other Department endorsed individual(s). Individuals preparing SWPPPs that require the post-construction stormwater management practice component must have an understanding of the principles of hydrology, water quality management practice design, water quantity control design, and, in many cases, the principles of hydraulics. All components of the SWPPP that involve the practice of engineering, as defined by the NYS Education Law (see Article 145), shall be prepared by, or under the direct supervision of, a professional engineer licensed to practice in the State of New York.

Redevelopment Activity(ies) – means the disturbance and reconstruction of existing impervious area, including impervious areas that were removed from a project site within five (5) years of preliminary project plan submission to the local government (i.e. site plan, subdivision, etc.).

Regulated, Traditional Land Use Control MS4 - means a city, town or village with land use control authority that is authorized to discharge under New York State DEC's
SPDES General Permit For Stormwater Discharges from Municipal Separate Stormwater Sewer Systems (MS4s) or the City of New York's Individual SPDES Permit for their Municipal Separate Storm Sewer Systems (NY-0287890).

Routine Maintenance Activity - means *construction activity* that is performed to maintain the original line and grade, hydraulic capacity, or original purpose of a facility, including, but not limited to:

- Re-grading of gravel roads or parking lots,
- Cleaning and shaping of existing roadside ditches and culverts that maintains the approximate original line and grade, and hydraulic capacity of the ditch,
- Cleaning and shaping of existing roadside ditches that does not maintain the approximate original grade, hydraulic capacity and purpose of the ditch if the changes to the line and grade, hydraulic capacity or purpose of the ditch are installed to improve water quality and quantity controls (e.g. installing grass lined ditch),
- Placement of aggregate shoulder backing that stabilizes the transition between the road shoulder and the ditch or *embankment*,
- Full depth milling and filling of existing asphalt pavements, replacement of concrete pavement slabs, and similar work that does not expose soil or disturb the bottom six (6) inches of subbase material,
- Long-term use of equipment storage areas at or near highway maintenance facilities,
- Removal of sediment from the edge of the highway to restore a previously existing sheet-flow drainage connection from the highway surface to the highway ditch or *embankment*,
- Existing use of Canal Corp owned upland disposal sites for the canal, and
- Replacement of curbs, gutters, sidewalks and guide rail posts.

Site limitations – means site conditions that prevent the use of an infiltration technique and or infiltration of the total WQv. Typical site limitations include: seasonal high groundwater, shallow depth to bedrock, and soils with an infiltration rate less than 0.5 inches/hour. The existence of site limitations shall be confirmed and documented using actual field testing (i.e. test pits, soil borings, and infiltration test) or using information from the most current United States Department of Agriculture (USDA) Soil Survey for the County where the project is located.

Sizing Criteria – means the criteria included in Part I.C.2 of the permit that are used to size post-construction stormwater management control practices. The criteria include; Water Quality Volume (WQv), Runoff Reduction Volume (RRv), Channel Protection Volume (Cpv), *Overbank* Flood (Qp), and Extreme Flood (Qf).

State Pollutant Discharge Elimination System (SPDES) - means the system established pursuant to Article 17 of the ECL and 6 NYCRR Part 750 for issuance of permits authorizing discharges to the waters of the state.

Appendix A

Steep Slope – means land area designated on the current United States Department of Agriculture ("USDA") Soil Survey as Soil Slope Phase "D", (provided the map unit name is inclusive of slopes greater than 25%), or Soil Slope Phase E or F, (regardless of the map unit name), or a combination of the three designations.

Streambank – as used in this permit, means the terrain alongside the bed of a creek or stream. The bank consists of the sides of the channel, between which the flow is confined.

Stormwater Pollution Prevention Plan (SWPPP) – means a project specific report, including construction drawings, that among other things: describes the construction activity(ies), identifies the potential sources of pollution at the *construction site*; describes and shows the stormwater controls that will be used to control the pollutants (i.e. erosion and sediment controls; for many projects, includes post-construction stormwater management controls); and identifies procedures the *owner or operator* will implement to comply with the terms and conditions of the permit. See Part III of the permit for a complete description of the information that must be included in the SWPPP.

Surface Waters of the State - shall be construed to include lakes, bays, sounds, ponds, impounding reservoirs, springs, rivers, streams, creeks, estuaries, marshes, inlets, canals, the Atlantic ocean within the territorial seas of the state of New York and all other bodies of surface water, natural or artificial, inland or coastal, fresh or salt, public or private (except those private waters that do not combine or effect a junction with natural surface waters), which are wholly or partially within or bordering the state or within its jurisdiction. Waters of the state are further defined in 6 NYCRR Parts 800 to 941.

Temporarily Ceased – means that an existing disturbed area will not be disturbed again within 14 calendar days of the previous soil disturbance.

Temporary Stabilization - means that exposed soil has been covered with material(s) as set forth in the technical standard, New York Standards and Specifications for Erosion and Sediment Control, to prevent the exposed soil from eroding. The materials can include, but are not limited to, mulch, seed and mulch, and erosion control mats (e.g. jute twisted yarn, excelsior wood fiber mats).

Total Maximum Daily Loads (TMDLs) - A TMDL is the sum of the allowable loads of a single pollutant from all contributing point and *nonpoint sources*. It is a calculation of the maximum amount of a pollutant that a waterbody can receive on a daily basis and still meet *water quality standards*, and an allocation of that amount to the pollutant's sources. A TMDL stipulates wasteload allocations (WLAs) for *point source* discharges, load allocations (LAs) for *nonpoint sources*, and a margin of safety (MOS).

Trained Contractor - means an employee from the contracting (construction) company, identified in Part III.A.6., that has received four (4) hours of Department endorsed

Appendix A

training in proper erosion and sediment control principles from a Soil and Water Conservation District, or other Department endorsed entity. After receiving the initial training, the *trained contractor* shall receive four (4) hours of training every three (3) years.

It can also mean an employee from the contracting (construction) company, identified in Part III.A.6., that meets the *qualified inspector* qualifications (e.g. licensed Professional Engineer, Certified Professional in Erosion and Sediment Control (CPESC), Registered Landscape Architect, New York State Erosion and Sediment Control Certificate Program holder, or someone working under the direct supervision of, and at the same company as, the licensed Professional Engineer or Registered Landscape Architect, provided they have received four (4) hours of Department endorsed training in proper erosion and sediment control principles from a Soil and Water Conservation District, or other Department endorsed entity).

The trained contractor is responsible for the day to day implementation of the SWPPP.

Uniform Procedures Act (UPA) Permit - means a permit required under 6 NYCRR Part 621 of the Environmental Conservation Law (ECL), Article 70.

Water Quality Standard - means such measures of purity or quality for any waters in relation to their reasonable and necessary use as promulgated in 6 NYCRR Part 700 et seq.

APPENDIX B – Required SWPPP Components by Project Type

Table 1

Construction Activities that Require the Preparation of a SWPPP That Only Includes Erosion and Sediment Controls

The following construction activities that involve soil disturbances of one (1) or more acres of land, but less than five (5) acres:
 Single family home <u>not</u> located in one of the watersheds listed in Appendix C or <u>not</u> directly discharging to one of the 303(d) segments listed in Appendix E. Single family residential subdivisions with 25% or less impervious cover at total site build-out and <u>not</u> located in one of the watersheds listed in Appendix C and <u>not</u> directly discharging to one of the 303(d) segments listed in Appendix E Construction of a barn or other agricultural building, silo, stock yard or pen.
The following construction activities that involve soil disturbances between five thousand (5000) square feet and one (1) acre of land:
All construction activities located in the watersheds identified in Appendix D that involve soil disturbances between five thousand (5,000) square feet and one (1) acre of land.
The following construction activities that involve soil disturbances of one (1) or more acres of land:
 Installation of underground, linear utilities; such as gas lines, fiber-optic cable, cable TV, electric, telephone, sewer mains, and water mains Environmental enhancement projects, such as wetland mitigation projects, stormwater retrofits and stream restoration projects Pond construction
 Linear bike paths running through areas with vegetative cover, including bike paths surfaced with an impervious cover Cross-country ski trails and walking/hiking trails
 Sidewalk, bike path or walking path projects, surfaced with an impervious cover, that are not part of residential, commercial or institutional development; Sidewalk, bike path or walking path projects, surfaced with an impervious cover, that include
incidental shoulder or curb work along an existing highway to support construction of the sidewalk, bike path or walking path.
 Slope stabilization projects Slope flattening that changes the grade of the site, but does not significantly change the runoff characteristics

Appendix B

Table 1 (Continued) CONSTRUCTION ACTIVITIES THAT REQUIRE THE PREPARATION OF A SWPPP

THAT ONLY INCLUDES EROSION AND SEDIMENT CONTROLS

The following construction activities that involve soil disturbances of one (1) or more acres of land:

- Spoil areas that will be covered with vegetation
- Vegetated open space projects (i.e. recreational parks, lawns, meadows, fields, downhill ski trails) excluding projects that alter hydrology from pre to post development conditions,
- Athletic fields (natural grass) that do not include the construction or reconstruction of *impervious* area and do not alter hydrology from pre to post development conditions
- Demolition project where vegetation will be established, and no redevelopment is planned
- Overhead electric transmission line project that does not include the construction of permanent access roads or parking areas surfaced with *impervious cover*
- Structural practices as identified in Table II in the "Agricultural Management Practices Catalog for Nonpoint Source Pollution in New York State", excluding projects that involve soil disturbances of greater than five acres and construction activities that include the construction or reconstruction of impervious area
- Temporary access roads, median crossovers, detour roads, lanes, or other temporary impervious areas that will be restored to pre-construction conditions once the construction activity is complete

Appendix B

Table 2

CONSTRUCTION ACTIVITIES THAT REQUIRE THE PREPARATION OF A SWPPP THAT INCLUDES POST-CONSTRUCTION STORMWATER MANAGEMENT PRACTICES

The following construction activities that involve soil disturbances of one (1) or more acres of land:

- Single family home located in one of the watersheds listed in Appendix C or *directly discharging* to one of the 303(d) segments listed in Appendix E
- Single family home that disturbs five (5) or more acres of land
- Single family residential subdivisions located in one of the watersheds listed in Appendix C or directly discharging to one of the 303(d) segments listed in Appendix E
- Single family residential subdivisions that involve soil disturbances of between one (1) and five (5) acres of land with greater than 25% impervious cover at total site build-out
- Single family residential subdivisions that involve soil disturbances of five (5) or more acres of land, and single family residential subdivisions that involve soil disturbances of less than five (5) acres that are part of a larger common plan of development or sale that will ultimately disturb five or more acres of land
- Multi-family residential developments; includes duplexes, townhomes, condominiums, senior housing complexes, apartment complexes, and mobile home parks
- Airports
- Amusement parks
- · Breweries, cideries, and wineries, including establishments constructed on agricultural land
- Campgrounds
- Cemeteries that include the construction or reconstruction of impervious area (>5% of disturbed area) or *alter the hydrology from pre to post development* conditions
- Commercial developments
- Churches and other places of worship
- Construction of a barn or other agricultural building (e.g. silo) and structural practices as identified in Table II in the "Agricultural Management Practices Catalog for Nonpoint Source Pollution in New York State" that include the construction or reconstruction of *impervious area*, excluding projects that involve soil disturbances of less than five acres.
- Golf courses
- Institutional development; includes hospitals, prisons, schools and colleges
- Industrial facilities; includes industrial parks
- Landfills
- Municipal facilities; includes highway garages, transfer stations, office buildings, POTW's, water treatment plants, and water storage tanks
- Office complexes
- Playgrounds that include the construction or reconstruction of impervious area
- Sports complexes
- · Racetracks; includes racetracks with earthen (dirt) surface
- Road construction or reconstruction, including roads constructed as part of the construction activities listed in Table 1

Appendix B

Table 2 (Continued)

CONSTRUCTION ACTIVITIES THAT REQUIRE THE PREPARATION OF A SWPPP THAT INCLUDES POST-CONSTRUCTION STORMWATER MANAGEMENT PRACTICES

The following construction activities that involve soil disturbances of one (1) or more acres of land:

- Parking lot construction or reconstruction, including parking lots constructed as part of the construction activities listed in Table 1
- Athletic fields (natural grass) that include the construction or reconstruction of impervious area (>5% of disturbed area) or alter the hydrology from pre to post development conditions
- Athletic fields with artificial turf
- Permanent access roads, parking areas, substations, compressor stations and well drilling pads, surfaced with *impervious cover*, and constructed as part of an over-head electric transmission line project, wind-power project, cell tower project, oil or gas well drilling project, sewer or water main project or other linear utility project
- Sidewalk, blke path or walking path projects, surfaced with an impervious cover, that are part of a residential, commercial or institutional development
- Sidewalk, bike path or walking path projects, surfaced with an impervious cover, that are part of a highway construction or reconstruction project
- All other construction activities that include the construction or reconstruction of *impervious area* or alter the hydrology from pre to post development conditions, and are not listed in Table 1

APPENDIX C – Watersheds Requiring Enhanced Phosphorus Removal

Watersheds where *owners or operators* of construction activities identified in Table 2 of Appendix B must prepare a SWPPP that includes post-construction stormwater management practices designed in conformance with the Enhanced Phosphorus Removal Standards included in the technical standard, New York State Stormwater Management Design Manual ("Design Manual").

- Entire New York City Watershed located east of the Hudson River Figure 1
- Onondaga Lake Watershed Figure 2
- Greenwood Lake Watershed -Figure 3
- Oscawana Lake Watershed Figure 4
- Kinderhook Lake Watershed Figure 5

Figure 1 - New York City Watershed East of the Hudson

L







Appendix C

Figure 3 - Greenwood Lake Watershed

L,

L,

Ŀ

L...

L

L

L



Appendix C

Figure 4 - Oscawana Lake Watershed



Figure 5 - Kinderhook Lake Watershed

L

L

1.

L



APPENDIX D – Watersheds with Lower Disturbance Threshold

Watersheds where *owners or operators* of construction activities that involve soil disturbances between five thousand (5000) square feet and one (1) acre of land must obtain coverage under this permit.

Entire New York City Watershed that is located east of the Hudson River - See Figure 1 in Appendix C

APPENDIX E – 303(d) Segments Impaired by Construction Related Pollutant(s)

List of 303(d) segments impaired by pollutants related to *construction activity* (e.g. silt, sediment or nutrients). The list was developed using "The Final New York State 2016 Section 303(d) List of Impaired Waters Requiring a TMDL/Other Strategy" dated November 2016. *Owners or operators* of single family home and single family residential subdivisions with 25% or less total impervious cover at total site build-out that involve soil disturbances of one or more acres of land, but less than 5 acres, and *directly discharge* to one of the listed segments below shall prepare a SWPPP that includes post-construction stormwater management practices designed in conformance with the New York State Stormwater Management Design Manual ("Design Manual"), dated January 2015.

COUNTY	WATERBODY	POLLUTANT
Albany	Ann Lee (Shakers) Pond, Stump Pond	Nutrients
Albany	Basic Creek Reservoir	Nutrients
Allegany	Amity Lake, Saunders Pond	Nutrients
Bronx	Long Island Sound, Bronx	Nutrients
Bronx	Van Cortlandt Lake	Nutrients
Broome	Fly Pond, Deer Lake, Sky Lake	Nutrients
Broome	Minor Tribs to Lower Susquehanna (north)	Nutrients
Broome	Whitney Point Lake/Reservoir	Nutrients
Cattaraugus	Allegheny River/Reservoir	Nutrients
Cattaraugus	Beaver (Alma) Lake	Nutrients
Cattaraugus	Case Lake	Nutrients
Cattaraugus	Linlyco/Club Pond	Nutrients
Cayuga	Duck Lake	Nutrients
Cayuga	Little Sodus Bay	Nutrients
Chautauqua	Bear Lake Nutrients	
Chautauqua	Chadakoin River and tribs	Nutrients
Chautauqua	Chautauqua Lake, North	Nutrients
Chautauqua,	Chautauqua Lake, South	Nutrients
Chautauqua	Findley Lake	Nutrients
Chautauqua	Hulburt/Clymer Pond	Nutrients
Clinton	Great Chazy River, Lower, Main Stem	Silt/Sediment
Clinton	Lake Champlain, Main Lake, Middle Nutrients	
Clinton	Lake Champlain, Main Lake, North	Nutrients
Columbia	Kinderhook Lake	Nutrients
Columbia	Robinson Pond	Nutrients
Cortland	Dean Pond	Nutrients

Dutchess	Fall Kill and tribs	Nutrients	
Dutchess	Hillside Lake	Nutrients	
Dutchess	Wappingers Lake	Nutrients	
Dutchess	Wappingers Lake	Silt/Sediment	
Erie	Beeman Creek and tribs	Nutrients	
Erie	Ellicott Creek, Lower, and tribs	Silt/Sediment	
Erie	Ellicott Creek, Lower, and tribs	Nutrients	
Erie	Green Lake	Nutrients	
Erie	Little Sister Creek, Lower, and tribs	Nutrients	
Erie	Murder Creek, Lower, and tribs	Nutrients	
Erie	Rush Creek and tribs	Nutrients	
Erie	Scajaquada Creek, Lower, and tribs	Nutrients	
Erie	Scajaquada Creek, Middle, and tribs	Nutrients	
Erie	Scajaquada Creek, Upper, and tribs	Nutrients	
Erie	South Branch Smoke Cr, Lower, and tribs	Silt/Sediment	
Erie	South Branch Smoke Cr, Lower, and tribs	Nutrients	
Essex	Lake Champlain, Main Lake, South	Nutrients	
Essex	Lake Champlain, South Lake	Nutrients	
Essex	Willsboro Bay	Nutrients	
Genesee	Bigelow Creek and tribs	Nutrients	
Genesee	Black Creek, Middle, and minor tribs	Nutrients	
Genesee	Black Creek, Upper, and minor tribs	Nutrients	
Genesee	Bowen Brook and tribs	Nutrients	
Genesee	LeRoy Reservoir	Nutrients	
Genesee	Oak Orchard Cr, Upper, and tribs	Nutrients	
Genesee	Tonawanda Creek, Middle, Main Stem	Nutrients	
Greene	Schoharie Reservoir	Silt/Sediment	
Greene	Sleepy Hollow Lake	Silt/Sediment	
Herkimer	Steele Creek tribs	Silt/Sediment	
Herkimer	Steele Creek tribs	Nutrients	
Jefferson	Moon Lake	Nutrients	
Kings	Hendrix Creek	Nutrients	
Kings	Prospect Park Lake	Nutrients	
Lewis	Mill Creek/South Branch, and tribs	Nutrients	
Livingston	Christie Creek and tribs	Nutrients	
Livingston	Conesus Lake	Nutrients	
Livingston	Mill Creek and minor tribs	Silt/Sediment	
Monroe	Black Creek, Lower, and minor tribs	Nutrients	
Monroe	Buck Pond	Nutrients	
Monroe	Cranberry Pond	Nutrients	

ſ

.

L

L

L

L

Ŀ

Monroe	Lake Ontario Shoreline, Western	Nutrients	
Monroe	Long Pond	Nutrients	
Monroe	Mill Creek and tribs	Nutrients	
Monroe	Mill Creek/Blue Pond Outlet and tribs	Nutrients	
Monroe	Minor Tribs to Irondequoit Bay	Nutrients	
Monroe	Rochester Embayment - East	Nutrients	
Monroe	Rochester Embayment - West	Nutrients	
Monroe	Shipbuilders Creek and tribs	Nutrients	
Monroe	Thomas Creek/White Brook and tribs	Nutrients	
Nassau	Beaver Lake	Nutrients	
Nassau	Camaans Pond	Nutrients	
Nassau	East Meadow Brook, Upper, and tribs	Silt/Sediment	
Nassau	East Rockaway Channel	Nutrients	
Nassau	Grant Park Pond	Nutrients	
Nassau	Hempstead Bay	Nutrients	
Nassau	Hempstead Lake	Nutrients	
Nassau	Hewlett Bay	Nutrients	
Nassau	Hog Island Channel	Nutrients	
Nassau	Long Island Sound, Nassau County Waters	Nutrients	
Nassau	Massapequa Creek and tribs	Nutrients	
Nassau	Milburn/Parsonage Creeks, Upp, and tribs	Nutrients	
Nassau	Reynolds Channel, west	Nutrients	
Nassau	Tidal Tribs to Hempstead Bay	Nutrients	
Nassau	Tribs (fresh) to East Bay	Nutrients	
Nassau	Tribs (fresh) to East Bay	Silt/Sediment	
Nassau	Tribs to Smith/Halls Ponds	Nutrients	
Nassau	Woodmere Channel	Nutrients	
New York	Harlem Meer	Nutrients	
New York	The Lake in Central Park	Nutrients	
Niagara	Bergholtz Creek and tribs	Nutrients	
Niagara	Hyde Park Lake	Nutrients	
Niagara	Lake Ontario Shoreline, Western	Nutrients	
Niagara	Lake Ontario Shoreline, Western	Nutrients	
Oneida	Ballou, Nail Creeks and tribs	Nutrients	
Onondaga	Harbor Brook, Lower, and tribs	Nutrients	
Onondaga	Ley Creek and tribs	Nutrients	
Onondaga	Minor Tribs to Onondaga Lake	Nutrients	
Onondaga	Ninemile Creek, Lower, and tribs	Nutrients	
Onondaga	Onondaga Creek, Lower, and tribs	Nutrients	
Onondaga	Onondaga Creek, Middle, and tribs	Nutrients	

61

Onondaga	Onondaga Lake, northern end	Nutrients	
Onondaga	Onondaga Lake, southern end	Nutrients	
Ontario	Great Brook and minor tribs	Silt/Sediment	
Ontario	Great Brook and minor tribs	Nutrients	
Ontario	Hemlock Lake Outlet and minor tribs	Nutrients	
Ontario	Honeoye Lake	Nutrients	
Orange	Greenwood Lake	Nutrients	
Orange	Monhagen Brook and tribs	Nutrients	
Orange	Orange Lake	Nutrients	
Orleans	Lake Ontario Shoreline, Western	Nutrients	
Orleans	Lake Ontario Shoreline, Western	Nutrients	
Oswego	Lake Neatahwanta	Nutrients	
Oswego	Pleasant Lake	Nutrients	
Putnam	Bog Brook Reservoir	Nutrients	
Putnam	Boyd Corners Reservoir	Nutrients	
Putnam	Croton Falls Reservoir	Nutrients	
Putnam	Diverting Reservoir	Nutrients	
Putnam	East Branch Reservoir	Nutrients	
Putnam	Lake Carmel	Nutrients	
Putnam	Middle Branch Reservoir	Nutrients	
Putnam	Oscawana Lake	Nutrients	
Putnam	Palmer Lake	Nutrients	
Putnam	West Branch Reservoir	Nutrients	
Queens	Bergen Basin	Nutrients	
Queens	Flushing Creek/Bay	Nutrients	
Queens	Jamaica Bay, Eastern, and tribs (Queens)	Nutrients	
Queens	Kissena Lake	Nutrients	
Queens	Meadow Lake	Nutrients	
Queens	Willow Lake	Nutrients	
Rensselaer	Nassau Lake	Nutrients	
Rensselaer	Snyders Lake	Nutrients	
Richmond	Grasmere Lake/Bradys Pond	Nutrients	
Rockland	Congers Lake, Swartout Lake	Nutrients	
Rockland	Rockland Lake	Nutrients	
Saratoga	Bailston Lake	Nutrients	
Saratoga	Dwaas Kill and tribs	Silt/Sediment	
Saratoga	Dwaas Kill and tribs	Nutrients	
Saratoga	Lake Lonely	Nutrients	
Saratoga	Round Lake	Nutrients	
Saratoga	Tribs to Lake Lonely	Nutrients	

 $\left[\right]$

t.

L

L

L

L

L

Schenectady	Collins Lake	Nutrients
Schenectady	Duane Lake	Nutrients
Schenectady	Mariaville Lake	Nutrients
Schoharie	Engleville Pond	Nutrients
Schoharie	Summit Lake	Nutrients
Seneca	Reeder Creek and tribs	Nutrients
St.Lawrence	Black Lake Outlet/Black Lake	Nutrients
St.Lawrence	Fish Creek and minor tribs	Nutrients
Steuben	Smith Pond	Nutrients
Suffolk	Agawam Lake	Nutrients
Suffolk	Big/Little Fresh Ponds	Nutrients
Suffolk	Canaan Lake	Silt/Sediment
Suffolk	Canaan Lake	Nutrients
Suffolk	Flanders Bay, West/Lower Sawmill Creek	Nutrients
Suffolk	Fresh Pond	Nutrients
Suffolk	Great South Bay, East	Nutrients
Suffolk	Great South Bay, Middle	Nutrients
Suffolk	Great South Bay, West	Nutrients
Suffolk	Lake Ronkonkoma	Nutrients
Suffolk	Long Island Sound, Suffolk County, West	Nutrients
Suffolk	Mattituck (Marratooka) Pond	Nutrients
Suffolk	Meetinghouse/Terrys Creeks and tribs	Nutrients
Suffolk	Mill and Seven Ponds	Nutrients
Suffolk	Millers Pond	Nutrients
Suffolk	Moriches Bay, East	Nutrients
Suffolk	Moriches Bay, West	Nutrients
Suffolk	Peconic River, Lower, and tidal tribs	Nutrients
Suffolk	Quantuck Bay	Nutrients
Suffolk	Shinnecock Bay and Inlet	Nutrients
Suffolk	Tidal tribs to West Moriches Bay	Nutrients
Sullivan	Bodine, Montgomery Lakes	Nutrients
Sullivan	Davies Lake	Nutrients
Sullivan	Evens Lake	Nutrients
Sullivan	Pleasure Lake	Nutrients
Tompkins	Cayuga Lake, Southern End	Nutrients
Tompkins	Cayuga Lake, Southern End	Silt/Sediment
Tompkins	Owasco Inlet, Upper, and tribs	Nutrients
Ulster	Ashokan Reservoir	Silt/Sediment
Ulster	Esopus Creek, Upper, and minor tribs	Silt/Sediment
Warren	Hague Brook and tribs	Silt/Sediment

63

Warren	Huddle/Finkle Brooks and tribs	Silt/Sediment
Warren	Indian Brook and tribs Silt/Sedin	
Warren	Lake George	Silt/Sediment
Warren	Tribs to L.George, Village of L George	Silt/Sediment
Washington	Cossayuna Lake	Nutrients
Washington	Lake Champlain, South Bay	Nutrients
Washington	Tribs to L.George, East Shore	Silt/Sediment
Washington	Wood Cr/Champlain Canal and minor tribs	Nutrients
Wayne	Port Bay	Nutrients
Westchester	Amawalk Reservoir	Nutrients
Westchester	Blind Brook, Upper, and tribs	Silt/Sediment
Westchester	Cross River Reservoir	Nutrients
Westchester	Lake Katonah	Nutrients
Westchester	Lake Lincolndale	Nutrients
Westchester	Lake Meahagh	Nutrients
Westchester	Lake Mohegan	Nutrients
Westchester	Lake Shenorock	Nutrients
Westchester	Long Island Sound, Westchester (East)	Nutrients
Westchester	Mamaroneck River, Lower	Silt/Sediment
Westchester	Mamaroneck River, Upper, and minor tribs	Silt/Sediment
Westchester	Muscoot/Upper New Croton Reservoir	Nutrients
Westchester	New Croton Reservoir	Nutrients
Westchester	Peach Lake	Nutrients
Westchester	Reservoir No.1 (Lake Isle)	Nutrients
Westchester	Saw Mill River, Lower, and tribs	Nutrients
Westchester	Saw Mill River, Middle, and tribs	Nutrients
Westchester	Sheldrake River and tribs	Silt/Sediment
Westchester	Sheldrake River and tribs	Nutrients
Westchester	Silver Lake	Nutrients
Westchester	Teatown Lake	Nutrients
Westchester	Titicus Reservoir	Nutrients
Westchester	Truesdale Lake	Nutrients
Westchester	Wallace Pond	Nutrients
Wyoming	Java Lake	Nutrients
Wyoming	Silver Lake	Nutrients

APPENDIX F – List of NYS DEC Regional Offices

/] }_____

1...

L

1.

L

Ĺ

L

L

<u>Region</u>	<u>COVERING THE</u> FOLLOWING COUNTIES!	DIVISION OF ENVIRONMENTAL PERMITS (DEP) <u>PERMITADWINISTRATORS</u>	DIVISION OF WATER (DOW) <u>WATER (SPDES) PROGRAM</u>
1	NASSAU AND SUFFOLK	50 CIRCLE ROAD STONY BROOK, NY 11790 TEL. (631) 444-0365	50 CIRCLE ROAD STONY BROOK, NY 11790-3409 TEL. (631) 444-0405
2	BRONX, KINGS, NEW YORK, Queens and Richmond	1 HUNTERS POINT PLAZA, 47-40 21ST ST. LONG ISLAND CITY, NY 11101-5407 TEL. (718) 482-4997	1 HUNTERS POINT PLAZA, 47-40 21st St. Long Island City, Ny 11101-5407 Tel. (718) 482-4933
3	DUTCHESS, ORANGE, PUTNAM, Rockland, Sullivan, Ulster and Westchester	21 SOUTH PUTT CORNERS ROAD NEW PALTZ, NY 12561-1696 Tel. (845) 256-3059	100 Hillside Avenue, Suite 1w White Plains, Ny 10603 Tel. (914) 428 - 2505
4	Albany, Columbia, Delaware, Greene, Montgomery, Otsego, Rensselaer, Schenectady and Schoharie	1150 North Westcott Road Schenectady, Ny 12306-2014 Tel. (518) 357-2069	1130 North Westcott Road Schenectady, Ny 12306-2014 Tel. (518) 357-2045
5	CLINTON, ESSEX, FRANKLIN, FULTON, HAMILTON, SARATOGA, WARREN AND WASHINGTON	1115 STATE ROUTE 86, Po Box 296 Ray Brook, Ny 12977-0296 Tel. (518) 897-1234	232 GOLF COURSE ROAD WARRENSBURG, NY 12885-1172 TEL. (518) 623-1200
6	HERKIMER, JEFFERSON, LEWIS, ONEIDA AND ST. LAWRENCE	STATE OFFICE BUILDING 317 WASHINGTON STREET WATERTOWN, NY 13601-3787 TEL. (315) 785-2245	STATE OFFICE BUILDING 207 GENESEE STREET UTICA, NY 13501-2885 TEL. (315) 793-2554
7	BROOME, CAYUGA, CHENANGO, CORTLAND, MADISON, ONONDAGA, OSWEGO, TIOGA AND TOMPKINS	615 ERIE BLVD. WEST SYRACUSE, NY 13204-2400 TEL. (315) 426-7438	615 ERIE BLVD. WEST SYRACUSE, NY 13204-2400 TEL. (315) 426-7500
8	CHEMUNG, GENESEE, LIVINGSTON, MONROE, ONTARIO, ORLEANS, SCHUYLER, SENECA, STEUBEN, WAYNE AND YATES	6274 EAST AVON-LIMA ROADAVON, NY 14414-9519 TEL. (585) 226-2466	6274 EAST AVON-LIMA RD. AVON, NY 14414-9519 TEL. (585) 226-2466
9	ALLEGANY, CATTARAUGUS, CHAUTAUQUA, ERIE, NIAGARA AND WYOMING	270 MICHIGAN AVENUE BUFFALO, NY 14203-2999 TEL. (716) 851-7165	270 MICHIGAN AVENUE BUFFALO, NY 14203-2999 TEL. (716) 851-7070





NOTES:

N/F

GRAPHIC SCALE

(IN FEET)

1 inch = 50 ft.

SCALE 1'' = 2000'

- 1. UNAUTHORIZED ALTERATION OR ADDITION TO A SURVEY MAP BEARING A LICENSED LAND SURVEYOR'S SEAL IS A VIOLATION OF SECTION 7209, SUBDIVISION 2 OF THE NEW YORK STATE EDUCATION LAW. 2. ONLY COPIES FROM THE ORIGINAL OF THIS SURVEY MARKED WITH AN ORIGINAL OF THE LAND SURVEYOR'S INKED SEAL OR HIS EMBOSSED SEAL SHALL BE CONSIDERED TO BE VALID TRUE COPIES.
- 3. CERTIFICATION INDICATED HEREON SIGNIFY THAT THIS SURVEY WAS PREPARED IN ACCORDANCE WITH THE EXISTING CODE OF PRACTICE FOR LAND SURVEYS ADOPTED BY THE NEW YORK STATE ASSOCIATION OF PROFESSIONAL LAND SURVEYORS. SAID CERTIFICATION SHALL RUN ONLY TO THE PERSON FOR WHOM THE SURVEY IS PREPARED, AND ON HIS BEHALF TO THE TITLE COMPANY, GOVERNMENTAL AGENCY, AND LENDING INSTITUTION LISTED HEREON, AND TO THE ASSIGNEES OF THE LENDING INSTITUTION. CERTIFICATIONS ARE NOT TRANSFERABLE TO ADDITIONAL INSTITUTIONS OR SUBSEQUENT OWNERS.
- 4. UNDERGROUND IMPROVEMENTS OR ENCROACHMENTS, IF ANY, ARE NOT SHOWN HEREON, 5. CONTACT UTILITY COMPANIES PRIOR TO ANY CONSTRUCTION AS UNDERGROUND IMPROVEMENTS MAY EXIST WHICH WERE NOT LOCATED AT THE TIME OF THIS SURVEY.
- 5. SUBJECT TO AN UP TO DATE AND ACCURATE TITLE REPORT. 7. SUBJECT TO ANY EASEMENTS OR AGREEMENTS IF ANY.
- 8. SUBJECT TO ANY RIGHTS, TITLE, OR INTEREST BY OTHERS TO PROPERTY IN AND/OR ALONG THE ROADWAYS SHOWN HEREON. 9. TOPOGRAPHY BASED ON A SURVEY PREPARED BY PIETRZAK & PFAU ENGINEERING & SURVEYING, PLLC. 10. ALL INTERIOR FEATURES MAY NOT BE SHOWN OR VISIBLE AT THE TIME OF THIS SURVEY.
- 11. TOTAL AREA OF PARCEL: 13.56± ACRES
- 12. PARCEL IS LOCATED IN THE R-2 ZONING DISTRICT.
- 13. TOTAL NUMBER OF PROPOSED RESIDENTIAL BUILDING LOTS: 11 14. ALL LOTS TO BE SERVED BY INDIVIDUAL SEPTIC.
- 15. TAX MAP DESIGNATION: SECTION 20 BLOCK 1 LOT 24. 16. PARCEL IS LOCATED WITHIN THE NEWBURGH CENTRAL SCHOOL DISTRICT.
- 17. PARCEL IS LOCATED OUTSIDE ANY 100 YEAR FLOOD PLAINS.
- 18. A ROAD AGREEMENT IS TO BE FILED W/ COUNTY CLERKS OFFICE PRIOR TO CONSTRUCTION.
- 19. ALL UTILITIES TO BE UNDER GROUND. 20. ALL SEWAGE DISPOSAL SYSTEMS AND WELLS WITHIN 200' OF THE PROJECT HAVE BEEN LOCATED AND ARE SHOWN ON THE PLANS. 21. INDIVIDUAL WELLS AND SEWAGE DISPOSAL SYSTEMS SHALL NO LONGER BE CONSTRUCTED OR USED FOR HOUSEHOLD DOMESTIC PURPOSES WHEN PUBLIC FACILITIES BECOME AVAILABLE. CONNECTION TO THE PUBLIC SEWER SYSTEM IS REQUIRED WITHIN ONE YEAR OF
- AVAILABILITY. 22. ORANGE COUNTY DEPARTMENT OF HEALTH PLAN APPROVAL IS LIMITED TO FIVE YEARS. TIME EXTENSIONS FOR PLAN APPROVAL MAY BE GRANTED BY THE ORANGE COUNTY DEPARTMENT OF HEALTH BASED UPON DEVELOPMENT FACTS AND THE REALTY SUBDIVISION
- GRANTED BT THE ORANGE COUNTY DEPARTMENT OF HEALTH BASED UPON DEVELOPMENT FACTS AND THE REALTY SUBDIVISION REGULATIONS IN EFFECT AT THAT TIME. A NEW PLAN SUBMISSION MAY BE REQUIRED TO OBTAIN A TIME EXTENSION.
 23. THE APPROVED PLANS MUST BE FILED WITH THE ORANGE COUNTY CLERKS OFFICE PRIOR TO OFFERING LOTS FOR SALE AND WITHIN 90 DAYS OF THE LAST APPROVAL OF FINAL PLANS.
 24. NO LOT IS TO BE FURTHER SUBDIVIDED WITHOUT ORANGE COUNTY DEPARTMENT OF HEALTH REVIEW AND APPROVAL.
 25. THE PURCHASER OF EACH LOT SHALL BE PROVIDED WITH A COPY OF THE APPROVED PLANS AND AN ACCURATE AS-BUILT DRAWING OF
- ANY EXISTING SANITARY FACILITIES. THE PURCHASER SHALL ALSO BE ADVISED OF ANY ROUTINE OR SPECIAL MAINTENANCE PROCEDURES THAT MAY BE NECESSARY (REFER TO PAGES 58-61 OF THE NYSDOH DESIGN HANDBOOK FOR RECOMMENDED ROUTINE OPERATION AND MAINTENANCE ITEMS).
- 26. THERE ARE NO N.Y.S.D.E.C. WETLANDS ON THE PARCEL. FEDERAL WETLANDS HAVE BEEN DELINEATED AND MAPPED IN ACCORDANCE WITH A.C.O.E. REGULATIONS. 27. THIS PROJECT DOES NOT FALL INTO A PUBLIC WATER SUPPLY WATER SHED.
- 28. FOR EACH RESIDENCE, PRIOR TO ISSUING A CERTIFICATE OF OCCUPANCY, THE SEPTIC SYSTEM SHALL BE INSPECTED AND A MAP SHALL BE PREPARED BY A LICENSED DESIGN PROFESSIONAL FOR SUBMISSION TO THE CODE ENFORCEMENT OFFICER.
- 29. THE CLEARING LIMIT AREAS SHALL BE CLEARLY MARKED IN THE FIELD (WITH PROTECTIVE FENCING) BEFORE COMMENCEMENT OF ANY SITE WORK. THE AREAS SO MARKED SHALL PROVIDE SUFFICIENT ARE TO PROTECT THE ROOT SYSTEMS OF THE TREES TO BE PROTECTED. IF THESE LIMITS ARE VIOLATED, THE DEVELOPER OR LOT OWNER SHALL BE REQUIRED TO PROVIDE ADDITIONAL OR REPLACEMENT LANDSCAPING OF EQUIVALENT BASAL AREA. 102/11/21 LIDDATE DOND DESIGN

LANDSCAPING UP EQUIVALENT DASAL AREA.	02/11/21 UPDATE POND DESIGN	CJP
수가 잘 많은 것 같아요. 이렇게 가지 않는 것 같아요. 이렇게 하는 것 같아요.	12/5/08 AS PER 12/4/08 PLANNING BOARD MEETING	TBE
	11/26/08 AS PER 11/25/08 WORKSHOP MEETING	KW
	11/14/08 AS PER TOWN ENGINEER COMMENTS	TBE
하는 것을 하는 것이 집에 있는 것이 있는 것이 있는 것이 있는 것이 있는 것이 없는 것이 없는 것이 없다.	6/23/08 AS PER OCHD COMMENTS DATED 5/8/08	TBE
	2/28/08 AS PER OCHD COMMENTS DATED 2/8/08	TBE
내 같은 가슴 가는 성장은 것은 것이 있는 것이 같이 같이 같이 있는 것이 같이	12/26/07 AS PER OCHD COMMENTS DATED 10/26/07	GNF
	9/4/07 GENERAL REVISIONS	MGF
그릇한 여섯 - 이 방송한 영화 옷이 있는 것은 것이 있는 것 같아. 것이 많이 있는 것이 없는	9/12/05 AS PER 07/6/05 DRAINAGE COMMENTS	TBE
이렇는 관련을 만들었다. 승규는 것이 있는 것은 것이 같이 많이 많이 많이 했다.	5/20/05 AS PER 09/29/04 CONSULTANT LETTERS	TBE
그는 것 같아요. 말한 것 같은 것 같아요. 것 것 같아요. 아이는 것 같아요. 것 같아요.	9/28/04 AS PER 08/04 CONSULTANT LETTERS	TBE
같은 것은 것 같은 사람이 있는 것이 같은 것은 것은 사람이 있는 것이 있다. 이 가지 않는 것은 것은 것은 것은 것은 것은 것은 것이 있는 것이 있는 것이 있다. 것은 것은 것은 것은 것은 것은 가 같은 것은	7/20/04 AS PER 03/04 CONSULTANT LETTERS	TBE
	4/1/04 ORIGINAL PREPARATION DATE	JS
이 방법은 관계에 가장 이 것을 수 있는 것이 같아요. 이 것이 가지는 것이 가지는 것이 가지?	DATE DESCRIPTION	INITIALS
이는 것이 같아요. 이는 것은 것은 것이 가지 않는 것이 가지 않는 것이 같아. 이는 것이 같아.	REVISIONS	
그는 것 같은 그는 것이 없는 것 같은 것 같은 것 같이 많이 많이 많이 했다.	MAP CHECK DATE: 00/00/00 INITIALED BY:	
사람이 가지 않는 것 같은 것을 가 많은 것이 같은 것이 없는 것이 없는 것이 없는 것이 없는 것이 없다.		
승규는 물질을 가 많은 것이 많은 것이 같이 다. 이렇게 많은 것이 많이 많이 많이 많이 했다.	J	
BULK REQUIREMENTS	PIETRZAK & PFA	TT
		. 14
ZONE R-2 (RESIDENTIAL DISTRICT) USE GROUP-ONE FAMILY DETACHED DWELLINGS	ENGINEERING & SURVEYING, F	
(WITHOUT PUBLIC SEWER)	262 GREENWICH AVENUE, SUITE A 2 HAMILTON AVENU	
승규는 지원 것 같아요. 그는 것은 것은 것이 있는 것이 가지 않는 것이 가지 않는 것이 없는 것이 같아.	GOSHEN, NEW YORK 10924 MONTICELLO, NEW YORK	
MIN. REQUIRED PROVIDED	(845) 294-0606 (845) 796-4646	
LOT AREA (SF) 17,500 >17,500		· · • •
LOT WIDTH (FT.) 100 >100	MICHARL P. BREEGAN/PNEW	
LOT DEPTH (FT.) 125 >125	LICENSE NO. 050823 DIA VIELS	
FRONT SETBACK (FT)* 40 >40	ALCENSE HON DESCENT A PIETRZAK P.	E. PLS
REAR SETBACK (FT.) 40 >40 SIDE SETBACK (FT.) 15 >15	LICENSE NO. 076	075
TOTAL SIDE SETBACK (FT.) 30 >30	IPLS UCENSE NO. 3	5396
	Windaws 55 1/1/4	
MAX. PERMITTED	SIGNATURE COS #00 DATE	4
BLDG. COVERAGE (%) 15 <15	THAT WE THE TATON	
LOT COVERAGE (%) 30 <30	TANDS OF ZAZON	
BLDG. HEIGHT (FT). 35 <35		
	TOWN OF NEWBURGH COUNTY OF ORANGE, NEW YORK	
* FRONT YARD SET BACK 60 <60 ALONG FOSTER TOWN ROAD	COUNTI OF ORANGE, NEW IORA	·]]
	PROJECT TITLE	
	CUDDINICION	
AREA=590,807± sq.ft.		
$=13.56\pm$ acres		
-10.001 ucres		
DEDADWINENT OF DUDUC WODER NOTES.		
	DRAWING TITLE SECTION 20 BLOCK 1 LO	T 24
DEPARTMENT OF PUBLIC WORKS NOTES:	UNAUTHORIZED ALTERATION OR ADDITION TO A PLAN BEARING A LICE	NSED
		NSED
CONSTRUCTION SHALL COMMENCE UNTIL A VALID	UNAUTHORIZED ALTERATION OR ADDITION TO A PLAN BEARING A LICE LAND SURVEYOR'S OR PROFESSIONAL ENGINEER'S SEAL IS A VIOLATIC SECTION 7209, SUBDIVISION 2 OF THE N.Y. STATE EDUCATION LAW. O.C.H.D. SHEET NO D.E.C. SHEET NO DRAWING NUM	NSED DN OF
DEFARINEINT OF PUBLIC WURKS NUTES: CONSTRUCTION SHALL COMMENCE UNTIL A VALID O FROM THE ORANGE COUNTY DEPARTMENT OF TION 136 OF THE HIGHWAY LAW	UNAUTHORIZED ALTERATION OR ADDITION TO A PLAN BEARING A LICE LAND SURVEYOR'S OR PROFESSIONAL ENGINEER'S SEAL IS A VIOLATIO SECTION 7209, SUBDIVISION 2 OF THE N.Y. STATE EDUCATION LAW,	NSED DN OF

SCALE CAD REFERENCE 1"=50" 23153.01

CAD REFERENCE PROJECT NUMBER

PERMIT HAS BEEN SECURED FROM PUBLIC WORKS UNDER SECTION 136 OF THE HIGHWAY LAW 2. NO PLANTING SHALL HAVE THE DRIP EDGE PROJECT WITHIN THE COUNTY R.O.W. 3. NO SIGNS SHALL BE PLACED OR PROJECT INTO THE COUNTY R.O.W.







		en fast er en stær finder og			이 것이 같아. 이 나는 것이 많이 많이 있어?
÷ n	THE TABLE	OF NOWDING	VIL WATCH CVCT	CLU DEALHDEC	A PERMIT FROM
10	THE ROWN	UF NEWBURU	H WALLE SIJ	EM REQUIRES	
			한 것 이 것 같은 것 같은 것 같은 것 같은 것 같은 것 같은 것 같은		THE LOOK OT TT
'NT	IS SHALL (ONFORM TO	HE REOLIREM	MIS OF THE N	EW YORK STATE
1.3.1		JOINT OF MILLING	(1.3 m > 1.3 m > K + 11.5 m + 1.1 m		

THRUST RESTRAINT OF THE PIPE SHALL BE THROUGH THE USE OF JOINT RESTRAINT. THRUST BLOCKS ARE NOT ACCEPTABLE. JOINT RESTRAINTS SHALL BE THROUGH THE USE OF MECHANICAL JOINT PIPE WITH RETAINER GLANDS. ALL FITTINGS AND VALVES SHALL ALSO BE INSTALLED WITH RETAINER GLANDS FOR JOINT RESTRAINT. RETAINER GLANDS SHALL BE EBBA IRON MEGALUG SERIES 1100 OR

4. ALL FITTINGS SHALL BE CAST IRON OR DUCTILE IRON, MECHANICAL JOINT, CLASS 250 AND CONFORM TO ANSI/AWWA C110/A21.10-87 OR LATEST REVISIONS FOR DUCTILE AND GRAY IRON FITTINGS OR ANSI/AWWA C153/A21.53-94 FOR LATEST REVISION FOR DUCTILE

GATE VALVES CONFORMING TO ANSI/AWWA C509 SUCH AS MUELLER MODEL T-2360-19 OR APPROVED EQUAL. ALL TAPPING SLEEVES

SHALL BE MUELLER H-15020 FOR % INCH AND 1 INCH, MUELLER H-15000 OR B-25000 FOR 1 % FOR 2 INCH SIZES. CURB VALVES SHALL BE MUELLER H-1501-2 FOR 14 AND 1 INCH AND MUELLER B-25204 FOR 1 1/2 AND 2 INCH SIZES. CURB BOXES

SHALL BE RESPONSIBLE FOR COORDINATING ALL INSPECTIONS AS REQUIRED WITH THE TOWN OF NEWBURGH WATER DEPARTMENT.

9. THE WATER MAIN SHALL BE TESTED, DISINFECTED AND FLUSHED IN ACCORDANCE WITH THE TOWN OF NEWBURGH REQUIREMENTS. ALL PUTTING THE WATER MAIN IN SERVICE, SATISFACTORY SANITARY RESULTS FROM A CERTIFIED LAB MUST BE SUBMITTED TO THE TOWN OF NEWBURGH WATER DEPARTMENT. THE TEST SAMPLES MUST BE COLLECTED BY A REPRESENTATIVE OF THE TESTING LABORATORY AND



------FD-----

2X2 GRATE

LEGEND

EXISTING WATER VALVE

A.C.O.E. WETLANDS RAIN GARDEN W/ CONVEYANCE SWALE PROPOSED FOOTING DRAIN W/ RODENT SCREEN PROPOSED ROOF LEADER W/ RODENT SCREEN EXISTING WELL

FOR HEALTH DEPARTMENT APPROVAL

THIS SHEET IS INCOMPLETE AND INVALID WITHOUT EACH AND EVERY OTHER SHEET LISTED IN THE TITLE BOX UNDER THE HEADING O.C.H.D. SHEET NO. - OF

		·	
	02/11/21 UPDATE POND DESIGN	CJP	
	2/5/08 AS PER 12/4/08 PLANNING BOARD MEETIN		
	1/26/08 AS PER 11/25/08 WORKSHOP MEETING	KW	
	1/14/08 AS PER TOWN ENGINEER COMMENTS	TBE	
	6/23/08 AS PER OCHD COMMENTS DATED 5/8/08	TBE	
	2/28/08 AS PER OCHD COMMENTS DATED 2/8/08	TBE	
	2/26/07 AS PER OCHD COMMENTS DATED 10/26/0		
	9/4/07 GENERAL REVISIONS	MGF	
	9/12/05 AS PER 07/6/05 DRAINAGE COMMENTS	TBE	
	5/20/05 AS PER 09/29/04 CONSULTANT LETTERS 9/28/04 AS PER 08/04 CONSULTANT LETTERS	TBE	
	7/20/04 AS PER 03/04 CONSULTANT LETTERS	TBE	
	4/1/04 ORIGINAL PREPARATION DATE	JS	
	DATE DESCRIPTION	INITIALS	
	REVISIONS		
	MAP CHECK DATE: 00/00/00 INITIALED BY: -		
	PIETRZAK & PFA ENGINEERING & SURVEYING,		
	262 GREENWICH AVENUE, SUITE A GOSHEN, NEW YORK 10924 (845) 294-0606 (845) 796-46 (845) 796-46	RK 12701	
CHORD 35	LICENSE NO. 00094	050075	
FROZEN RID	LANDS OF ZAZON		
366.9 65.3 65.4.1 1	TOWN OF NEWBURGH COUNTY OF ORANGE, NEW YOR	K	
	PROJECT TITLE UTILITY SHEET		
ATE 18" RCP 18" NUT=361.0±"	DRAWING TITLE SECTION 20 BLOCK 1 LOT 24 UNAUTHORIZED ALTERATION OR ADDITION TO A PLAN BEARING A LICENSED		
	LAND SURVEYOR'S OR PROFESSIONAL ENGINEER'S SEAL IS A VIOL SECTION 7209, SUBDIVISION 2 OF THE N.Y. STATE EDUCATION LA O.C.H.D. SHEET NO D.E.C. SHEET NO DRAWING I	ation of W.	
	OF OF OF	11	
	SCALE CAD REFERENCE PROJECT N 1"=50' 23153COV 23153.01		



國際 사람이 다 지난 사람은 영화 관계에 가지는 것이 같은 것은 것은 것이 가지 않는 것이 가지 않는 것이 같이 있다.	
PROPOSED 1250 GAL. SEPTIC TANK	02/11/21 UPDATE POND DESIGN CJP
	12/5/08 AS PER 12/4/08 PLANNING BOARD MEETING TBE
PROPOSED SDS	11/26/08 AS PER 11/25/08 WORKSHOP MEETING KW
	11/14/08 AS PER TOWN ENGINEER COMMENTS THE
50 % EXPANSION	6/23/08 AS PER OCHD COMMENTS DATED 5/8/08 TBE
사람이 있는 것은 <u>다 가장 가 있었</u> 는 것이 있는 것이 같이 있는 것이 있는 것이 있다. 것이 있는 것이 있는 것이 있는 것이 있는 것이 있는 것이 있는 것이 있는 것이 있는 것이 있다. 것이 있는 것이 없는 것이 있는 것이 없는 것이 없는 것이 있는 것이 있는 것이 없는 것이 없는 것이 있는 것이 없는 br>것이 않아, 것이 않아, 것이 않아, 것이 없는 것이 않아, 것이 없는 것이 없는 것이 없는 것이 없는 것이 없는 것이 없는 것이 없는 것이 없이 않아, 것이 않아, 것이 않아, 것이 없이 않아, 않아, 것이 않아, 않아, 것이 않아, 것이 않아, 않아, 것이 않아, 것이 않아, 않아 것이 것이 않아, 것이 않아, 것이 않아, 것이 않아, 것이 않아, 것이 않아, 것이 않아, 것이 않아, 것이 않아, 않아, 것이 않아, 것이 않아, 것이 않아, 것이 않아, 것이 않아, 것이 않아, 것이 않아, 것이 않아, 않아, 것이 않아, 것이 않아, 않아, 것이 않아, 것이 않아, 않아, 않아, 것이 않아, 것이 않아, 것이 않아, 것이 않아, 않아, 않아, 않이 않아, 것이 않아, 않이 않아, 않아, 것	2/28/08 AS PER OCHD COMMENTS DATED 2/8/08 TBE
	12/26/07 AS PER OCHD COMMENTS DATED 10/26/07 GNF
PROPOSED DWELLING	9/4/07 GENERAL REVISIONS MGF
	9/12/05 AS PER 07/6/05 DRAINAGE COMMENTS THE
PROP. 20'x30' SNOW	5/20/05 AS PER 09/29/04 CONSULTANT LETTERS THE
STORAGE AREA	9/28/04 AS PER 08/04 CONSULTANT LETTERS THE
STABILIZED W/ GRASS COVER	7/20/04 AS PER 03/04 CONSULTANT LETTERS THE
& CURB CUT.	4/1/04 ORIGINAL PREPARATION DATE JS
가장 이 가슴에 가지 않는 것이 있는 것이 있는 것이 있는 것이 있는 것이 있다. 이 가슴에 있는 것이 있는 것이 있는 것이 있는 것이 있는 것이 있는 것이 있는 것이 있는 것이 있는 것이 있는 것 이 같은 것은 같은 것이 같은 것이 있는 것이 있는 것이 있는 것이 있는 것이 있는 것이 있는 것이 있는 것이 있는 것이 있는 것이 있는 것이 있는 것이 있는 것이 있는 것이 있는 것이 있는 것이 있는	
PROPOSED STORM WATER CATCH BASIN	DATE DESCRIPTION INITIALS
	REVISIONS
So PROPOSED HYDRANT	MAP CHECK DATE: 00/00/00 INITIALED BY:
-ge PROPOSED HYDRANT	
물건이 가지 않는 것 같은 것 같은 것 같은 것이다. 이 가지 않는 것 같은 것 같은 것이다. 가지 않는 것 같은 것 같은 것 같은 것 같은 것 같은 것 같은 것 같은 것 같	
4 [%] PROPOSED WATER VALVE	DIDTDTAT 0. DEATT
	PIETRZAK, & PFAU
	ENGINEERING & SURVEYING, PLLC
A.C.O.E. WETLANDS	262 GREENWICH AVENUE, SUITE A 2 HAMILTON AVENUE
	GOSHEN, NEW YORK 10924 KONTICELLO, NEW YORK 12701
	(845) 294-0606 (845) 796-4646
	AND THE OWNER OF THE OWNER OWNER OF THE OWNER
EXISTING TREE TO REMAIN	MICHAEL F. CREEGAN PLS
1월 28일 전 28일 전 28일 전 20일 전 28일 - 11일 전 28일 전	LICENSE NO. 050323
EXISTING TREE TO BE REMOVED	JOSEPH I. PPAU PE
철학 사람은 것은 방법에서 적별한 것이 많은 것이 같이 많이 있는 것이 같이 많이 있는 것이 같이 많이 많이 했다.	LICENSE NO. 066945 P.E. LICENSE NO. 076936 PLS. LICENSE NO. 050075
에는 이 가지 않는 것 같은 것 같은 것 같은 것 같은 것 같은 것 같은 것 같은 것 같	NJPLS. LICENSE, NO, 35396
	1 1 State Si shinkar
그는 것은 것을 수 있는 것은 것을 수 있는 것을 하는 것을 가지 않는 것을 수 있다.	SIGNATURE TANK STOR
	SIGNATURE DATE /
	LANDS OF 7470N
이 같은 것은 것이 있는 것이 같은 것이 있는 것이 있는 것이 같은 것이 있는 것이 있는 것이 있는 것이 있는 것이 있는 것이 있는 것이 있는 것이 있는 것이 있는 것이 있는 것이 있는 것이 있는 것 같은 것이 같은 것이 같은 것이 같은 것이 있는 것이 같은 것이 있는 것이 같은 것이 있는 것이 같은 것이 있는 것이 같은 것이 있는 것이 같은 것이 있는 것이 같은 것이 있는 것이 같은 것이 있는 것	LANDS OF ZAZON
	TOWN OF NEWBURGH
	COUNTY OF ORANGE, NEW YORK
병 이 이 나는 밖에 앉아? 그는 것이 아이지 않는 것이 있는 것이 없는 것이 없다.	
	PROJECT TITLE
ROTECTION NOTES:	TREE LOCATION
Y AND MARK THE CLEARING LIMIT LINE.	
L ORANGE SAFETY FENCING ALONG THE CLEARING LINE.	CUPPT
VER INDIVIDUAL SPECIMEN TREES SHALL BE PRESERVED, INSTALL SAFETY FENCIN	
3 OR OUTSIDE OF THE DRIP LINE OF THE TREE CANOPY.	DRAWING TITLE SECTION 20 BLOCK 1 LOT 24
SHALL BE NO STORAGE OF EQUIPMENT OR MATERIALS INSIDE CLEARING LIMITS.	
UIPMENT IS PERMITTED WITHIN CLEARING LIMITS.	UNAUTHORIZED ALTERATION OR ADDITION TO A PLAN BEARING A LICENSED LAND SURVEYOR'S OR PROFESSIONAL ENGINEER'S SEAL IS A VIOLATION OF
ENCING SHOULD REMAIN IN GOOD CONDITION FOR THE DURATION OF THE TRUCTION PERIOD.	SECTION 7209, SUBDIVISION 2 OF THE N.Y. STATE EDUCATION LAW.
REES CLEARED WITHOUT AUTHORIZATION WITHIN PROTECTED AREAS SHALL BE	O.C.H.D. SHEET NO D.E.C. SHEET NO DRAWING NUMBER
CED WITH A TREE EQUAL TO THE BASEL AREA OF THE TREE PREVIOUSLY REMO	
S NECESSARY TO CLEAR OR REMOVE TREES WITHIN THE PROTECTED AREAS, WRI	TTEN SCALE CAD REFERENCE PROJECT NUMBER
ISSION FROM THE TOWN OF NEWBURGH MUST BE RECEIVED PRIOR TO THE REMO	DVAL. 1"=50' 23153COV 23153.01
	//
말했는 것 같아요. 저녁 지역에 가슴 가락 같은 것 같아. 이는 것 같아. 이는 것 같아.	

지수는 것 같아요. 지수는 것	
e de la composition de la composition de la composition de la composition de la composition de la composition d	EXISTING WATER VALVE
	EXISTING HYDRANT
and the second second	EXISTING MAIL BOX
	EXISTING SIGN
en la presenta de la composición de la composición de la composición de la composición de la composición de la	EXISTING HEAD WALL
	EXISTING UTILITY POLE
	EXISTING 10' CONTOUR
nden son en ser en ser en ser en ser en ser en ser en ser en ser en ser en ser en ser en ser en ser en ser en s En ser en ser	EXISTING 2' CONTOUR
	PROPOSED GRADE
	PROPOSED LOT LINE
	PROPOSED LUI LINE
	EXISTING PROPERTY LINE

LEGEND

* DENOTES TREE CLUSTER

TREES TO BE REMOVED P		TREE DESCRIPTION
TO BE REMOVED 2	and di di di di 🛔	28" DBH SUGAR MAPLE
	007	26" DBH SUGAR MAPLE
TO BE REMOVED 2		40" DBH NORWAY SPRUCE
TO BE REMOVED 2		30" DBH SUGAR MAPLE
TO BE REMOVED 2	. 2 j j	42" DBH SUGAR MAPLE
TO BE REMOVED 2	011	44" DBH WHITE PINE
TO BE REMOVED 2	012*	TWIN 30" DBH MAPLES
TO BE REMOVED 2		26" DBH NORWAY SPRUCE
	014	36" DBH SUGAR MAPLE
TO BE REMOVED 2		50" DBH WHITE OAK
「「「「」」「「」」」「「」」」「「「」」」「「「「」」」「「」」」「「」	016	24" DBH SUGAR MAPLE
TO BE REMOVED 2	017	18" DBH SUGAR MAPLE
TO BE REMOVED 2		14" DBH SUGAR MAPLE
TO BE REMOVED 2	019	14" DBH BLACK OAK
TO BE REMOVED 2		TWIN 24" DBH BASSWOOD
	021	17" DBH BLACK CHERRY
	022*	12", 14", AND 15" DBH SUGAR MAPLE
	023	17" DBH SUGAR MAPLE
TO BE REMOVED 2		19" DBH SUGAR MAPLE
	025*	12", 15", AND 16" DBH SUGAR MAPLE
이 같이 있는 것이 있는 것이 있는 것이 있는 것이 있는 것이 있는 것이 있다. 이 것이 있는 것이 있는 것이 있는 것이 있다. 이 것이 있는 것이 있는 것이 있는 것이 있는 것이 있는 것이 있는 것이 없다. 이 있는 것이 있는 것이 없는 것이 없는 것이 없다. 이 있는 것이 없는 것이 없는 것이 없는 것이 없는 것이 없다. 이 있는 것이 없는 이 없는 것이 없이 없는 것이 없이 없는 것이 없는 것이 없이 않이 않이 않이 않이 않이 않이 않이	2026*	TWIN 17" DBH SUGAR MAPLES
그는 그는 그는 것 같은 것을 가려왔다. 그는 것이 잘 못했다.	2027*	17" AND 19" DBH BLACK WALNUT
이 같은 것 같아요. 이 집 것 같아요. 이 밖에서 이 것은 것 같아요. 이 것 같아요. 나는 것 같아요.	028*	(6) 16-20" DBH SUGAR MAPLES
TO BE REMOVED 2	029	18" DBH BLACK WALNUT 17" AND 19" DBH BLACK WALNUT
・「「「「」、「」、「」、「」、「」、「」、「」、「」、「」、「」、「」、「」、「	031	17" AND 19" DBH BLACK WALNUT 20" DBH BLACK WALNUT
· 제품 · 제품 · 제품 · 제품 · 제품 · 제품 · 제품 · 제품	032*	16" AND 17" DBH BLACK CHERRY &
	002	19" DBH BLACK WALNUT
	033	18" DBH RED OAK
그래 김 씨는 지수는 문화적으로 가지 않는 것이 많이 있는 것이 없는 것이 같다.	2034	20" DBH RED OAK
아내는 것 같은 방법에서 가지 않는 것 같은 것 같이 가지 않는 것 같아.	035	15" DBH RED OAK
「うちょう」 しんがい しゃ おもなく かんじょう 三月之間 アリール しいれんしょうかい	036	15" DBH BLACK OAK
	037	18" DBH TULIP POPLAR
· 같이 가지 않는 것 같은 것 같은 것 같은 것 같은 것 같은 것 같은 것 같은 것 같	038*	15" AND 17" DBH RED OAK
「「「」「「」」「」」「」」「」」「」」「」」」」「」」「「」」「」」「「」」」「」」「」」」」	039*	18" AND 20" DBH RED OAK
時間 이렇는 김 요즘 전에 있는 것은 그렇게 한 것을 가지 않는 것을 만들었다. 이 나는 것을	040	16" DBH BLACK OAK
で働いたり マヤクト しかたきみぶんして 外外 からくしゃきから にかかみ ないし 事分務会	041	16" DBH RED OAK
이 같은 것 같은 것 같은 것 같은 것이 못 것 않는 것 같은 것 같이 많을 것 때 수가요?	042	19" DBH RED OAK
이 집에 가지 않는 것은 것을 많은 것을 하는 것을 많이 하는 것을 많이 없다. 전화적	043*	(3) 16 " DBH BLACK OAK
TO BE REMOVED 2		17" DBH RED OAK
TO BE REMOVED 2		17" DBH BLACK WALNUT
	046*	(2) 17" DBH SUGAR MAPLES
小手 かっか ロード ふかいがら 二十分 かっかう ようしゃ 靴の 夜川 しかど	047	24" DBH SUGAR MAPLE
그럼 그 그는 것 같은 것 같아요. 이 문화가 이 가운 것이라는 것 않는 것 같아요. 한 것	048	18" DBH BLACK OAK
이렇게 가지 않는 것을 가지 않는 것이 같이 있는 것이 같이 많이	049*	18" AND 23" DBH BLACK OAKS
2	050*	(5) BLACK OAK 14-24" DBH
1	051	(4) BLACK OAK 14-20" DBH
TO BE REMOVED 2	052	18" DBH BLACK WALNUT





1.		TEST RESULTS & SI	-PIIC SYSTEM DESIG	N	
LOT #	DEEP TEST PIT #1. RESULTS	DEEP TEST PIT #2 RESULTS	JSI DEEP. TEST PIT # 3 RESULTS	STABILIZED PERC RATE	S
1	0-12" TOPSOIL - ROOT MASS 12"-54" SANDY SILT W/ COBBLE & GRAVEL NO SEEPAGE BOULDERS @ 54"	0-11" TOPSOIL & ROOT MASS 11"-64" SANDY SILT W/ GRAVEL, COB & TRACE BOULDER 64"-74" SILT W/GRAVEL, COB & BOULDER NO SEEPAGE NO BEDROCK	0-8" TOPSOIL 8"-80" SILT LOAM W/ COBBLES AND STONES NO SEEPAGE NO BEDROCK	PT1: 5 MIN. DEPTH: 24" PT2: 4 MIN. DEPTH: 24"	EL DE # 4 SE
2	0-12" TOPSOIL 12"-32" LIGHT BROWN SILT LOAM W/ SOME SAND 32"-90" SANDY SILT W/ COBBLES, BOULDERS & GRAVEL NO SEEPAGE NO BEDROCK	0-12" TOPSOIL 12"-84" LIGHT BROWN SILT LOAM W/ COB., GRAVEL & BOULDERS NO SEEPAGE NO BEDROCK		PT1: 12 MIN. DEPTH: 24" PT2: 35 MIN. DEPTH: 24" JSI 16 MIN. DEPTH: 24"	EL DE # 5 SE
3	0-11" TOPSOIL-ROOT MASS 11"-88" SANDY SILT LOAM W/ COBBLES AND TRACE BOULDERS NO SEEPAGE NO BEDROCK	0-12" TOPSOIL-ROOT MASS 12"-70" GRAVELY SILT LOAM W/ COBS, BOULDER NO SEEPAGE NO BEDROCK		PT1: 3 MIN. DEPTH: 24" PT2: 10 MIN. DEPTH: 24"	EL DE # 4 SEI
4	0-12" TOPSOIL-ROOT MASS 12"-36" GRAVELY SILT LOAM W/ COBBLES 36"-49" FRACTURED SHALE W/ COB, BOULDER & LEDGE ROCK NO SEEPAGE BEDROCK @ 49"	0-12" TOPSOIL-ROOT MASS 12"-49" SILT LOAM W/ GRAVEL & COBS. NO SEEPAGE BEDROCK @ 49"	0-11" TOPSOIL 11"-42" SILTY LOAM 42"-89" SILT W/ SAND, COBBLES, AND STONES NO SEEPAGE NO BEDROCK	PT1: 10 MIN. DEPTH: 24* PT2: 8 MIN. DEPTH: 24*	EL DE # # 4
5	0-12" TOPSOIL 12"-64" BOULDERS W/ SILTY GRAVELY LOAM NO SEEPAGE REFUSAL @ 64"	0-12" TOPSOIL 12"-48" SILT LOAM W/ COBBLES & GRAVEL 48"-76" SILT LOAM W/ GRAVEL & COBBLES SEEPAGE @ 76"	NO BEDROCK	PT1: 4 MIN. DEPTH: 24" PT2: 7 MIN. DEPTH: 24" JSI 6 MIN. DEPTH: 24"	SE DE # 4
5	0-8" TOPSOIL 0-8" TOPSOIL 8"-26" LIGHT BROWN MOTTLED SILT LOAM W/ GRAVEL/SAND 26"-60" COBBLE, GRAVEL W/ SILT LOAM 60"-70" SANDY GRAVEL / SEEPAGE © 70" MOTTLING © 26" NO BEDROCK	BEDROCK @ 49" 0-9" TOPSOIL 9"-53" SILT LOAM W/ GRAVEL & COBS, BOULDERS MOTTLING @ 24" NO BEDROCK	0-9" TOPSOIL 9"-76" SILT LOAM W/ STONES, COBBLES, AND BOULDERS NO SEEPAGE NO BEDROCK	PT1: 3 MIN. DEPTH: 24" PT2: 14 MIN. DEPTH: 24"	SE DE # 4 SEI
7, 7,	0-9" TOPSOIL 9"-63" SILT LOAM W/ GRAVEL & COBS, BOULDERS NO SEEPAGE NO BEDROCK	0-10" TOPSOIL 10"-36" SILT LOAM W/ GRAVEL & COBS, 36"-68" SANDY SILTY W/ BOULDERS, GRAVEL & COBS, NO SEEPAGE NO BEDROCK		PT1: 2 MIN. DEPTH: 24" PT2: 8 MIN. DEPTH: 24" JSI 2 MIN. DEPTH: 24"	EL DE # # 4 SEI
3	0-10" TOPSOIL 10"73" SILT LOAM W/ GRAVEL & COBS, TRACE BOULDERS LIGHT MOT @ 50" NO BEDROCK	0-8" TOPSOIL 8"30" SILT LOAM W/ GRAVEL 30"77" GRAVELLY SILT LOAM SOME BOULDER NO SEEPAGE NO BEDROCK		PT1: 28 Min. DEPTH: 24" PT2: 18 Min. DEPTH: 24"	EL. DE: # # 4 L SEF
)	0-8" TOPSOIL 8"-36" SILT LOAM W/ GRAVEL & COBS 36"-68" GRAVELY LOAM W/ COBS 68"-80" GRAVELY LOAM W/ BOULDERS NO SEEPAGE NO BEDROCK	0-12" TOPSOIL 12"36" GRAVELY SILT LOAM W/ COBS 36"60" SANDY SILT LOAM NO SEEPAGE NO BEDROCK	•	PT1: 3 MIN. DEPTH: 24* PT2: 4 MIN. DEPTH: 24*	ELJ DES # 1 # 1 4 L SEP
0	0-8" TOPSOIL 8"-24" GRAVELY SILT LOAM 24"-72" GRAVELY LOAM W/ COBS AND SOME BOULDERS NO SEEPAGE NO BEDROCK	0-8" TOPSOIL 8"-24" SILT LOAM W/ GRAVEL 24"-70" FRACTURED SHALE SILTY SAND LOAM W/ BOULDERS NO SEEPAGE NO BEDROCK	0-5" TOPSOIL 5"-84" SILT W/ SAND W/ COBBLES AND STONE NO SEEPAGE NO BEDROCK	PT1: 3 Min. DEPTH: 24* PT2: 5 Min. DEPTH: 24*	ELJ DES # E # E 4 L SEP
1	0-8" TOPSOIL 8"-40" SANDY SILT LOAM W/COBBLE	0-7" TOPSOIL 7"-25" SILT LOAM W/ GRAVEL & COBS 25"-76" BROWN SILT LOAM W/ GRAVEL & COBS		PT1: 28 MIN. DEPTH: 24* PT2: 2 MIN. DEPTH: 24*	ELJI DES # E

NOTES:

1. PERCOLATION TESTS WERE PERFORMED ON 6/14,15,17&20/05 BY PIETRZAK AND PFAU, PLLC.

2. DEEP PIT TESTS WERE PERFORMED ON 6/22/05.

BY PIETRZAK AND PFAU, PLLC.

 ALL PERCOLATION TESTS WERE PERFORMED AT A DEPTH OF 24".
 JSI PERCOLATION AND DEEP PIT TESTS WERE PERFORMED ON 5/14/08 BY PIETRZAK AND PFAU, PLLC, AND WITNESSD BY THE ORANGE COUNTY DEPARTMENT OF HEALTH. ENGINEER'S CERTIFICATION THE PROPOSED SEWAGE DISPOSAL SYSTEM(S) AND WATER SUPPLY SYSTEM(S) SHOWN ARE DESIGNED IN ACCORDANCE WITH THE STANDARDS AND REQUIREMENTS ESTABLISHED BY THE NEW YORK STATE DEPARTMENT OF HEALTH AND THE NEW YORK STATE DEPARTMENT OF ENVIRONMENTAL CONSERVATION FOR RESIDENTIAL LOTS. THE DESIGN(S) ARE BASED UPON ACTUAL SOIL AND SITE CONDITIONS FOUND UPON THE LOT(S) AT THE DESIGN LOCATION AT THE TIME OF DESIGN.

The second second second second second second second second second second second second second second second se

PVI STA = 0+35.91 PVI ELEV = 426.41 A.D. = 1.00 |K = 50.00|337.50' VC 50.0' VC LOW POINT ELEV = 425.85 CB-2A LOW POINT STA = 1+17.16STATION 3+17 440 ······ PVI STA = 2+35.91 CB-3A RiM = 429.51STATION 1+17 PVI ELEV + 424.41 _." OF 15" INV.= 426,76 RIM = 425.51HDPE@5.6% A.D. = 6.75 CB-2 INV.= 422.76 27' OF 1 K = 50.00 STATION 3+17 CB-3 HDPE@3.7% RIM= 429.51 STATION 1+17 INV = 425.26 RIM = 425.51INV.= 421.76 CB-1A 2.00% STATION 4+76 -1.00%RIM= 437.88 201' OF 18" HDPE @ 1.7% INV.= 435.13 CB-1 26'0F15 HDPE@2.2 8"DIP WATER MAIN STATION 4+76 HDPE@1.0% 420 RIM= 437.88 INV.= 434.38 CB-4PROPOSED CB-5A STATION 0+59 WATER MAIN STATION 0+28 RIM= 425.84 TO CONNECT RIM= 426.26 INV.= 422.90 O EX. WATER , 35' OF 15" INV.= 423.51 MAIN. CONTRACTOR CB-5 HDPE@1.09 STATION 0+28 TO VERIFY RIM = 426.26OCATION & INV.= 423.16 <u>ATUM ELEV</u> 405.00 LEVATION. NOT FOR HEALTH DEPARTMENT REVIEW 0+00 1+00 4+00 5+00 ROAD A HORIZONTAL SCALE 1"=50' VERTICAL SCALE 1"=10'





• •

- ·

· • •

BIORETENTION FILTER CROSS SECTION DETAIL

NOTES: 1. THE FILTER MEDIA SHOULD FALL WITHIN THE SM, OR ML CLASSIFICATIONS OF THE USCS. 2. THE SOIL SHOULD BE FREE OF STONES, STUMPS, ROOTS, OR OTHER WOODY MATERIAL OVER 1" IN DIAMETER AND BRUSH OR SEEDS FROM NOXIOUS WEEDS. THE FILANTING SOIL SHOULD BE IN LIETS OF 12" TO 3. PLACEMENT OF THE PLANTING SOIL SHOULD BE IN LIFTS OF 12" TO 18", LOOSELY COMPACTED (TAMPED LIGHTLY WITH A DOZER OR BACKHOE BUCKET).
A DENSE AND VIGOROUS VEGETATIVE COVER SHALL BE ESTABLISHED OVER THE CONTRIBUTING PERVIOUS DRAINAGE AREAS BEFORE RUNOFF CAN BE ACCEPTED INTO THE FACILITY.

- FILTER

40° SCH. 40 PERFORATED PIPE

PLANTING SOILS SHALL WITH THE FOLLOWING S	
PH RANGE ORGANIC MATTER MAGNESIUM	5.2 - 7.0 1.5 - 4.0% 35 LBS/AC
PHOSPHORUS POTASSIUM	75 LBS/AC 85 LBS/AC
CLAY SILT	10-25% 30-55%
SAND	35-60%

	11/14/08							
	6/23/08	AS PER OCHD COMMENTS DATED 5/8/08 T						
	2/28/08	AS PER OCHD COMMENTS DATED 2/8/08						
	12/26/07	AS PER O	CHD COMMENTS DAT	ED 10/26/07	GNF			
	9/4/07	GENERAL I	MGF					
		AS PER 07/6/05 DRAINAGE COMMENTS						
			AS PER 09/29/04 CONSULTANT LETTERS TBE					
	9/28/04	AS PER 08/04 CONSULTANT LETTERS TBE						
	7/20/04		3/04 CONSULTANT	ETTERS	TBE			
í	4/1/04	ORIGINAL	PREPARATION DATE		JS			
	DATE		DESCRIPTION	· · · · · · · · · · · · · · · · · · ·	INITIALS			
		· · ·	REVISIONS					
	MAP CHEC	K DATE: 0	0/00/00 INITIAL	ED BY:	-			
•••								
	PI	TR	ZAK, &	PFA	TT			
			VG & SURV					
				<i>5111</i> 4 6 , 1				
		WICH AVENU		HAMILTON AVEN				
	1	I, NEW YORK		ELLO, NEW YOR				
		(845) 294-0606 (845) 796-4646						
	NUSTRABAR BAR							
	MICHAEL P	HAEL & CREECIN' PLS						
	LICEPS	5 HU LOUGS CEED AND AND AND AND AND AND AND AND AND AN						
		JOSEPH. J. PFAU. P.E. LICENSE NO. 068945						
	- F. R. LICENSE NO. 050075							
	JE NJPLS. LICENSE NO. 35396							
	11ng	MBARE 18 2/1/21						
	SIGNATURE: DATE							
	LANDS OF ZAZON							
	LANDS OF ZAZON							
	TOWN OF NEWBURGH COUNTY OF ORANGE, NEW YORK							
		PROJECT TITLE						
	STORMWATER							
		DETAIL SHEET						
		DEIAIL OREEI						
	DRAWING	ITTLE SE	ECTION 20 BI	OCK 1 LC	T 24			
		AUTHORIZED ALTERATION OR ADDITION TO A PLAN BEARING A LICENSED						
	LAND S	AND SURVEYOR'S OR PROFESSIONAL ENGINEER'S SEAL IS A VIOLATION OF ECTION 7209, SUBDIVISION 2 OF THE N.Y. STATE EDUCATION LAW.						
		· · · · · · · · · · · · · · · · · · ·	D.E.C. SHEET NO	· · · · · · · · · · · · · · · · · · ·	MRFR			
		F <u>6</u>	_#OF#	9 OF				
- 1								

CAD REFERENCE PROJECT NUMBER

1"=50' 23153COV 23153.01

SCALE

02/11/21 UPDATE POND DESIGN

12/5/08 AS PER 12/4/08 PLANNING BOARD MEETING THE

11/26/08 AS PER 11/25/08 WORKSHOP MEETING KW

CJP

JMN ING

OWNER.

1	R	AIN GARDEN PLAN	TING LIST	
	BOTANICAL NAME	COMMON NAME	SIZE	SEAS
	WINTERBERRY	ILEX VERTICILLATA	24" HIGH	AUTU
1.	ARROWWOOD	VIBURNUM DENTATUM	24" HIGH	SPRI
	SWEET PEPPERBUSH	CLETHRA ALNIFOLIA	24" HIGH	SUM
	7. THE TOP FEW INCHES OF	N. ACED 5 FEET ON CENTER. MINIMUM OF 24" IN HEIGHT. NTAINER GROWN WITH A WE JNED IF THEY START TO BE MAY INCLUDE OCCASIONAL I D THINNING TO MAINTAIN T PLANTING SOIL IS TO BE PONDS FOR MORE THAN 4	LL ESTABLISHED COME LEGGY AND F REPLACEMENT OF PI HE DESIRED APPEAF REMOVED AND 8 HOURS.	LANTS, RANCE.

and the second second

SEE PLANTING LIST



방법 그는 말했던 것 같은 것 같아. 것



RAIN GARDEN PLANTINGS, TYP.

• PLANTING LEGEND:

	Es.
an miskan benderininka (88) inderinder dets staten dets beför til find næts seder dir sin bendels versions tes A	

EXISITNG TREES TO REMAIN

EXISTING TREELINE



DECIDUOUS SHRUBS

EVERGREEN TREES PROPOSED TREELINE

NORTHEAST NATIVE WILDFLOWER MIX

RETENTION / DETENTION PLANTINGS:



LOWEST ZONE MIDDLE ZONE



HIGHEST ZONE

N/F HAMILTON

SCALE: |" = 5'



LANDSCAPE NOTES:

PLANTING SCHEDULE:

- I. REFER TO PLANTING DETAIL, PLANTING AND GROUNDCOVERS SCHEDULES FOR INSTALLATION OF NEW TREES AND SHRUBS.
- 2. THE CONTRACTOR SHALL TAG AND NUMBER ALL PLANT MATERIAL TO BE RELOCATED. THE CONTRACTOR SHALL FULLY ASSIST AND COORDINATE THIS WORK WITH THE LANDSCAPE ARCHITECT AND OWNER'S REPRESENTATIVE PRIOR TO INITIATING FIELD DIGGING THE TREES.
- 3. LOCATIONS OF RELOCATED PLANT MATERIAL ARE APROXIMATED ON THE DRAWINGS. EXACT LOCATIONS OF RELOCATED PLANT MATERIAL WILL BE CLARIFIED BY THE LANDSCAPE ARCHITECT OR OWNER'S REPRESENTATIVE ON SITE.
- 4. THE CONTRACTOR SHALL BEAR ALL COSTS OF TESTING OF SOILS, AMENDMENTS, ETC. ASSOCIATED WITH THE WORK AND INCLUDED IN THE SPECIFICATIONS.
- 5. NEW PLANTING ROOT BALLS REMOVED FROM CANS SHALL BE SCARIFIED PRIOR TO BACKFILLING THE PLANT.
- 6. TYPICALLY, SHRUB AND GROUNDCOVER PLANTINGS ARE SHOWN AS MASS PLANTINGS BEDS. PLANTS SHALL BE PLACED ON A TRIANGULAR SPACING CONFIGURATION (STAGGERED SPACING). PLANT CENTER TO CENTER DIMENSIONS (O.C.) ARE LISTED ON THE PLANT LIST.
- 7. CONTRACTOR SHALL FIELD STAKE THE LOCATIONS OF ALL PLANT MATERIAL PRIOR TO INITIATING INSTALLATION FOR THE REVIEW AND APPROVAL OF THE OWNER'S REPRESENTATIVE AND/OR LANDSCAPE ARCHITECT.
- 8. CONTRACTOR SHALL FIELD ADJUST LOCATION OF PLANT MATERIAL AS NECESSARY TO AVOID DAMAGE TO ALL EXISTING UNDERGROUND UTILITIES AND/OR EXISTING ABOVE GROUND ELEMENTS. ALL CHANGES REQUIRED SHALL BE COMPLETED AT THE CONTRACTOR'S EXPENSE AND SHALL BE COORDINATED WITH THE OWNER'S REPRESENTATIVE AND THE LANDSCAPE ARCHITECT.
- 9. SHRUB MASS PLANTINGS ALONG ROADWAYS AND ENTRY LANDSCAPING AREAS TO RECEIVE ADDITIONAL LANDSCAPE AND FLORAL PLANTING CHOSEN BY THE DEVELOPER AND/OR TENANT TO FURTHER ENHANCE AND CUSTOMIZE THE OVERALL APPEARANCE OF THE PROPERTY.
- 10. ALL TREES PLANTED IN LAWN AREA SHALL RECEIVE A 3 DIAMETER MULCH RING AT A DEPTH OF 3" AS PER THE SPECIFICATIONS.
- II. ALL EXISTING TREES PLANTED IN LAWN AREA SHALL RECEIVE A MINIMUM 5' DIAMETER MULCH RING OR TO THE LIMIT OF THE ADJACENT LAWN AREA AT A DEPTH OF 3" AS PER THE SPECIFICATIONS.
- 12. EXISTING VEGETATION AREAS DISTURBED BY CONSTRUCTION ACTIVITIES SHALL BE TOPSOILED AND SEEDED UPON COMPLETION OF WORK
- 13. PLANT LIST QUANTITIES ARE PROVIDED FOR CONVENIENCE IN THE EVENT OF QUANTITY DISCREPANCIES, THE DRAWING SHALL TAKE PRECEDENCE. ANY DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE LANDSCAPE ARCHITECT.
- 14. CONTRACTOR SHALL REFER TO THE LANDSCAPE PLANTING DETAILS, PLANT LIST, GENERAL NOTES AND SPECIFICATIONS FOR FURTHER AND COMPLETE INSTRUCTIONS.
- 15. ALL PLANTS SHALL BE WARRANTED FOR A PERIOD OF TWO YEARS. REPLACE, IN ACCORDANCE WITH THE DRAWINGS AND SPECIFICATIONS, ALL PLANTS THAT ARE MISSING, MORE THAN 25% DEAD, WHICH DO NOT DEVELOP FROM PLANTING STOCK, THAT APPEAR UNHEALTHY OR UNSIGHTLY AND/OR HAVE LOST THEIR NATURAL SHAPE DUE TO DEAD BRANCHES. ANY TREE THAT LOOSES THE MAIN LEADER SHALL BE REPLACED. PLANT MATERIAL SHALL BE INSPECTED BY THE LANDSCAPE ARCHITECTURAL CONSULTANT FOR THE TOWN OF NEWBURGH UPON COMPLETION OF WORK AND DURING EVERY GROWING SEASON FOR TWO YEARS. PLANTS THAT NEED REPLACEMENT SHALL BE NOTED ON AN INSPECTION REPORT AND MUST BE REPLACED WITHIN TWO MONTHS OF RECEIPT OF THE INSPECTION REPORT OR WITHIN TWO MONTHS FROM THE NEXT FOLLOWING GROWING SEASON.
- 16. ALL PLANTINGS SHOWN ON APPROVED SITE DEVELOPMENT PLAN OR SPECIAL PERMIT PLAN SHALL BE MAINTAINED IN A VIGOROUS GROWING CONDITION THROUGHOUT THE DURATION OF USE AND PLANTS NOT SO MAINTAINED SHALL BE REPLACED WITH NEW PLANTS AT THE BEGINNING OF THE NEXT IMMEDIATELY FOLLOWING THE GROWING SEASON.
- 17. ALL PLANT MATERIAL SHALL CONFORM TO THE STANDARDS FOR NURSERY STOCK SET FORTH BY THE AMERICAN ASSOCIATION OF NURSERYMEN IN THERE MOST RECENT EDITION.
- 18. ANY DETERMINATION OF "EQUAL" SUBSTITUTION SHALL BE MADE ONLY BY THE LANDSCAPE ARCHITECT.

TREES 4 SH EVERGR DECIDUC

1. TOPSOIL SHALL BE SPREAD TO A COMPACTED UNIFORM THICKNESS OF 6".

2. TOPSOIL SURFACE SHALL BE FINELY GRADED AND LOOSENED BY MECHANICAL RAKES TO ENSURE SEED ACCEPTANCE AND SEED TO SOIL CONTACT.

3. FERTILIZER SHALL BE APPLIED AT 6 LBS. OF 5-10-10 COMMERCIAL FERTILIZER/1000 SQ. FT.



RETENTION/DETENTION PLANTING SCHEDULE:

CENTER

3. THE SEEDING FOR EACH ZONE SHALL CONSIST OF EQUAL PARTS PER SPECIES AND BROADCAST OVER IT'S RESPECTIVE AREA AT A RATE OF I POUND PER 2,000 SQFT.

2. AN OVERLAP OF SPECIES FROM ONE ZONE TO THE NEXT WILL NOT EXCEED 3'-6', ALLOWING FOR INTERGRATION BETWEEN THE INDIVIDUAL ZONES.

3. NORTHEAST WETLAND DIVERSITY MIX SHALL BE BROADCAST OVER BASIN AREAS AT A RATE OF 2 POUNDS PER ACRE INTERMIXED WITH PLUGS AND/OR SEEDING SPECIFIED ABOVE AS MANUFACTURED BY:

SOUTHERN TIER CONSULTING, INC. 2701-A ROUTE 305, P.O. BOX 30 WEST CLARKSVILLE, NY 14786

HRUBS	SPRING PLANTING FALL PLANTING	
REEN	APRIL 1 - JUNE 30 SEPT 1 - OCT. IE	2
ous	MARCH 1 - JUNE 30 OCT. I - DEC.I	

PERMANENT SEEDING SCHEDULE:

4. SEEDING SHALL BE INSTALLED AT 5 LBS/1000 SQ. FT.

60% KENTUCKY BLUE GRASS

20% CHEWINGS FESCUE 20% PERENNIAL RYE

RETENTION/DETENTION POND PLANTINGS:

LOWEST ZONE:

PONTEDERIA CORDATA, PICKELWEED SCIRPUS PUNGENS, COMMON THREE-SQUARE TYPHA SPP., CATTAIL

VALLISMERA AMERICANA, WILD CELERY

MIDDLE ZONE:

CAREX LACUSTRIS, LAKEBANK SEDGE SAGGITTARA LATIFOLIA, DUCK POTATO SCIRPUS CYPERNUS, WOOL GRASS SCIRPUS VALIDUS, SOFT-STEM BULRUSH

HIGHEST ZONE:

DESCHAPSIA CAESPITOSA. TUFTED HAIRGRASS GLYCERIA STRIATA, FOWL MANNAGRASS JUNCUS EFFUSUS, SOFT RUSH PANICUM VIRGATUM, SWITCH GRASS

1. EACH ZONE CONSISTS OF FOUR SPECIES TO BE PLANTED USING EITHER I &" PLUGS AND/OR SEED.

2. THE PLUGS FOR EACH ZONE SHALL BE RANDOMLY MIXED AND PLANTED WITH AN APPROXIMATE SPACING OF 2'-6" ON

UNUSOUTHERNTIERCONSULTING.COM



PLANTING & GUYING DETAIL 8/9/ NTS.

	LANTS LIST:				
	IYPE	KEY	QTY.	BOTANICAL NAME	COMMON NAME
	이러와 화장되면 물건이 가졌다. 아이들에게 가려면 운동	Ar	8	Acer rubrum "Red Sunset"	Red Sunset Map
	DeciduousTrees		6	Betula nigra	River Birch
n de North		Bp Cc	10	Cercis canadensis	Eastern Redbuc
		Qp	13	Quercus palustris "Sovereign"	Sovereign Pin (
	Evergreen Trees	Np	2Ø	Picea Abies	Norway Spruce
	Deciduous Shrubs	Ac	1	Amelanchier canadensis	Shadblow Servi
		Fi	2	Forsythia x intermedia	Spring Glory Fo
		Ì٧	43	llex verticullata "Red Sprite"	Red Sprite Hol
		Sd	13	Cornus amomium	Silky Dogwood
		S s	6	Rhus typhina	Staghorn Sumac
		Yd	18	Vibernum dentatum	Arrowwood Vib
	Perennials	МЬ	54	Monarda didyma	Oswego Tea
	Grasses	Сз	72	Carex stipata	Tussock Sedge
				그는 것이 한 것이 있는 것 같은 것이 같은 것이 것이 같은 것이 있는 것이 가지 않는 것 같은 것을 수 없다.	

GENERAL NOTES:

- I. CONTRACTOR SHALL FAMILIARIZE HIM/HERSELF WITH THE LIM TS OF WORK AND EXISTING CONDITIONS AND VERIFY ALL INFORMATIC . IF DISCREPANCIES EXIST, CONTRACTOR SHALL NOTIFY OWNER REPRESENTATIVE IN WRITING WITHIN SEVEN (T) CALENDAR D. 17 , OF NOTICE TO PROCEED.
- 2. CONTRACTOR SHALL VERIFY LOCATIONS OF ALL UNDERGRO IN UTILITIES AND OBTAIN AS-BUILT INFORMATION. DRAWINGS WIRE PREPARED ACCORDING TO THE BEST INFORMATION AVAILA 31 3 AT THE TIME. CONTRACTOR SHALL NOTIFY OWNER'S REPRESEN A IVE IN WRITING WITHIN SEVEN (7) CALENDAR DAYS OF NOTICE TO PROCEED OF ANY DISCREPANCIES.
- 3. CONTRACTOR SHALL NOTIFY ALL NECESSARY UTILITY COMP AN ES 48 HOURS MINIMUM PRIOR TO DIGGING FOR VERIFICATION OF ALL UNDERGROUND UTILITIES AND OTHER OBSTRUCTIONS AND COORDINATE WITH OWNER'S REPRESENTATIVE IN WRITING PRIOF TO INITIATING OPERATIONS.
- 4. FOR DIMENSIONS OF BUILDINGS, SEE ARCHITECTURAL DRAWING
- 5. THE CONTRACTOR SHALL BE RESPONSIBLE FOR MAINTAINING & AFE CONDITIONS AT ALL TIMES.



RETENTION BASIN PLANTINGS

ceberry orsythia

61ZE REI 3"-3-1/2" c B41 14'-16' hgt B41 2-1/2"-3" c B41 3"-3-1/2" c B41					
3"-3-1/2" c B41 14'-16' hgt B41 2-1/2"-3" c B41 3"-3-1/2" c B41	SIZ	E		F	TE
14'-16' hgt B41 2-1/2"-3" c B41 3"-3-1/2" c B41			2" C	E	34E
2-1/2"-3" c B41 3"-3-1/2" c B41					e 1 e
	1 A A A A A A A A A A A A A A A A A A A			E	34E
	3"-	3-17	2" C	ં દ	34E
7' - 8' hgt B4	. T ' '	- 8	hgt	E	34E

30"-34" 34"-36" 24"-3Ø" 30"-34" 3Ø"-34" 30"-34"

"" " " and a second second second second second

18" 00 18" oc

I gal. l gal.

B4B

Container

Container

BAB

BAB

BAB

IARKS



BEFORE YOU DIG, DRILL OR BLAST CALL US TOLL FREE 1-800-962-7962

IT INDUSTRIAL CODE RULE 153 REQUIRES NO LESS THAN TWO WORKINGDAYS NOTICE, BUT NOT MORE THAN TEN DAYS

AUTHORIZED ALTERATION OR ADDITION TO THIS DRAWING IS A MOLATION OF SECTION 1209 (2) OF THE NYS EDUCATION LAW. THIS DRAWING IS ONE IN A SET OF DRAWINGS AND IS INCOMPLETE AND NVALID WHEN IT IS SEPARATED FROM THE SET.



