

### McGOEY, HAUSER and EDSALL CONSULTING ENGINEERS D.P.C.

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**Principal Emeritus:** RICHARD D. McGOEY, P.E. (NY & PA) WILLIAM J. HAUSER, P.E. (NY, NJ & PA)

### TOWN OF NEWBURGH PLANNING BOARD TECHNICAL REVIEW COMMENTS

**PROJECT: PROJECT NO.: PROJECT LOCATION: REVIEW DATE: MEETING DATE:** 

LAKESIDE SENIOR HOUSING 2016-19 SECTION 86, BLOCK 1, LOT 39.22 & 39.23 14 JULY 2017 20 JULY 2017 PROJECT REPRESENTATIVE: MEDENBACH AND EGGERS

- 1. The Applicants representative have submitted aeronautical studies from the FAA regarding no hazard determinations for each of the proposed structures.
- 2. The Applicants representative have submitted a Water and Sewer Engineering report. Water flow portion of the report identifies that further calculations will be provided at Building permit.
- 3. The Planning Board has previously issued a Negative Declaration. NYS Department of Environmental Conservation provided a notice of complete application which was published in the first full week of July.
- 4. Status of the Senior Housing use approval from the Town Board should be addressed.
- A Stormwater Pollution Prevention Plan has been prepared and is being reviewed by this office.
- 6. Previous comments supplied to the Applicants representative have been addressed on the most recent submissions.
- 7. A Public Hearing for the project is required.
- 8. The Applicants have submitted the Architectural Review form for the Boards use.

• Regional Office • 111 Wheatfield Drive • Suite 1 • Milford, Pennsylvania 18337 • 570-296-2765 •

#### ACEC Member

Respectfully submitted,

McGoey, Hauser and Edsall Consulting Engineers, D.P.C. -2-

Patrick J. Hines Principal

### PJH/kbw

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## Medenbach & Eggers

Civil Engineering and Land Surveying P.C.

4305 US Highway 209 Stone Ridge, New York 12484-5620

Barry Medenbach, P.E. N.Y.Lic. No. 60142 N.J.Lic. No. 27646

Phone (845) 687-0047 Fax (845) 687-4783 www.mecels.com

William R. Eggers L.S. N.Y.Lic. No. 49785

July 5, 2017

McGoey, Hauser & Edsall Patrick Hines 33 Airport Center Drive, Suite 202 New Windsor, NY 12553

Re: Lakeside Seniors, Town of Newburgh.

Dear Patrick,

Please find enclosed a complete set of plans for the above project and the following comments in response to your April 14, 2017 memo.

- 1. City of Newburgh Flow Acceptance letter was issued on March 27, 2017 and previously submitted.
- 2. Outlet control structure has been added to the plans
- 3. New York State Department of Environmental Conservation (NYSDEC) permit notice will be published July 7, 2017 affidavit pending.
- 4. Architectural plans are attached.
- 5. Generator location has been revised.
- 6. Engineering Report for sanitary pump station has been completed and attached for your review.
- 7. Storm Water Pollution Prevention Plan (SWPPP) has been completed and attached.

Please contact our office with any questions on the above.

Yours truly, Bar<u>ry N</u>

Cc: John Ewasutyn, Planning Board Chairman Jay Feinberg

Attached:

Site Plan Architectural Plans Engineering Report Storm Water Pollution Prevention Plan FAA – No Obstruction Letter

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Phone (845) 687-0047 Fax (845) 687-4783 www.mecels.com William R. Eggers L.S. N.Y.Lic. No. 49785

July 5, 2017

Town of Newburgh Planning Board John Ewasutyn, Chairman 308 Gardnertown Road Newburgh, NY 12550

Re: Lakeside Seniors

Dear Mr. Ewasutyn,

As you may be aware, the Town Board approved our project for Senior Housing at the June 26, 2017 meeting and we submit the following for the July 20, 2017 Planning Board Meeting.

- 1. Four full size sets of site plans and Architectural Plans.
- 2. Twelve 11x17 sets of site plans and Architectural Plans.
- 3. One disc copy of Storm Water Pollution Prevention Plan.
- 4. Fifteen Engineering Reports
- 5. Fifteen FAA Letter No Obstruction for buildings #1,2 & 3.
- 6. Fifteen Architectural Forms

One copy of each of the items listed above is being mailed directly to Patrick Hines, Michael Donnelly and Ken Wersted.

Yours truly,

Barry Medenbach, P.E.

### ARCHITECTURAL REVIEW FORM TOWN OF NEWBURGH PLANNING BOARD

DATE:

 $s^{\lambda}$ 

NAME OF PROJECT: LAKESIDE SENIOR HOUSING

The applicant is to submit in writing the following items prior to signing of the site plans.

**EXTERIOR FINISH** (skin of the building):

Type (steel, wood, block, split block, etc.) HARDI-PANEL LAP SIDING & TRIM

### COLOR OF THE EXTERIOR OF BUILDING:

STO. HARDI CO	OLORS . TRIM. NAVAJO BEIGE'
	SIDING MONTEREY TAURE' A AUTUMN TAN', B HEATHERED MOSS' C
ACCENT TRIM:	HEMHENED MOSS
Location: _	SOFFITS, FASCIAS, CASING, CORNER BOS.
Color:	'NAVAJO BEIGE' - (STO. COLUR)
Type (mate	rial): HARDI - BOARDS

PARAPET (all roof top mechanicals are to be screened on all four sides):  $_{N/\!A}$ 

**ROOF:** 

Type (ga	bled, flat, etc.): _	HIP and	GABLED	
Material	(shingles, metal,	tar & sand, etc.	): ASPHALT	SHINGLES
Color: _	CERTAINTE	D: TEXTUU	250 / 40 yr	/ BIRCHWOOD'

#### WINDOWS/SHUTTERS:

Color (also trim if different):		<u>Sa</u>	NUTONE	
Туре:	ANDENSEN	200	SERIES	

**DOORS:** 

, P \*..

Color: SANDTONE

Type (if different than standard door entrée): <u>AMOEUSEN</u>

SIGN:

Color: Wood France to Make Sidn," Mantgonicos ~ Material: Wood + Stone	Tarpe.
Square footage of signage of site: 3 SF. One Side	

Please print name and title (owner, agent, builder, superintendent of job, etc.)

Lech PE Signature

### WATER AND SEWER ENGINEERING REPORT FOR LAKESIDE SENIOR A Proposed 102 Senior Housing Rental Facility Situate: Lakeside Road Town of Newburgh, Orange County, New York Tax Map Number: 86-1-39.22 and 86-1-39.23

Prepared By: Medenbach & Eggers Civil Engineering and Land Surveying P.C. 4305 US Highway 209 Stone Ridge, NY 12484

> March 28, 2017 Revised June 30, 2017

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- III. Sewer

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IV. Fire Flow

### <u>APPENDIX</u>

- A. Sewer Prep Specifications
- B. Fire flow Calculations

### I. General Description:

The project sponsors propose to construct a 102-unit Senior Housing Development within three 3-story buildings and developed in accordance with the Town of Newburgh Zoning Code, Section 185-48 "Senior Citizen Housing," age restricted to 55 years or older. The 19.23 acre site is situated off Lakeside Road, behind the Four Points Sheraton Hotel ("Hotel") and is approximately 1,000 ft northeast of Route 17K and ½ mile west of Exit 6 on US Interstate 84. Access is via a 50 ft Right of Way ("ROW") that is shared with the Hotel and runs in a northerly direction between the Hotel and a pond found on southeasterly portion of the Hotel parcel. The building site abuts the Ice Time skating rink parking lot where an emergency access agreement exists. The site contains a total of 9.25 acres of wetlands and wetlands buffers, leaving a net 9.98 acres of upland area available for development. The wetlands will be preserved with a conservation restriction and no disturbance to the wetlands will be required for construction of the proposed facility. A minor disturbance will be required to the 100 ft. adjacent area to the NYSDEC wetland for construction of an emergency access road.

Site improvements will consist of an extension of the access road along the water, sewer and electric utilities extending from Lakeside Road over an existing ROW. Onsite construction will include new parking lots, site lighting, storm water management, as well as landscaping and outside passive reaction areas.

The site is within the Town of Newburgh Cross Roads Sewer District and Town water district. The sewer will connect to the existing 4" force main situated on Lakeside Road. A new force main will be installed along the ROW from Lakeside Road approximately 1400 LF to the on-site proposed pump station. The connection to the force main on Lakeside Road is approximately 800 ft from the force main in 17K. The water will extend from the existing 12" main in Lakeside Rd. with a 8" water main along the ROW that connects to the three buildings and 3 fire hydrants.

II. Design Flow:

The project consists of 102 senior apartment units in three buildings. The buildings will contain 72 two bedroom and 30 one bedroom apartments for a total of 174 bedrooms. Using the New York State Department of Environmental Conservation (NYSDEC) design standards of 110 gallons per day per bedroom will produce 19,140 gallons per day. This assumes a population of 348, two per bedroom. However, the project is age restricted and it is anticipated one bedroom in the two bedroom units will be used as a guest room, office space or craft room and the population will be substantially less than 348 and more likely 204. This assures guests using the 2<sup>nd</sup> bedroom would equal the apartment with single residence. Therefore, we estimate

the average population of 204 and the average daily flow would be 11,220 gallons per day (7.8 gallons per minute). Peak flow assumed at 10 times average daily flow = 78 gpm.

### III. Sewer:

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The three buildings will be served with a 6" or 8" PVC gravity collection piping connected to a proposed duplex grinder pumping station centrally located on the site. The pump station will deliver the sewage with 800 LF of 3" force main to the existing 4" force main in Lakeside Rd.

#### **Design Data:**

ADF = 11,220 gallon Peak flow = 80 gallon/min

Pumps selected are 2" Goulds 2GA( 7.5 HP), these can deliver up to 180 gallons/min with 178' TDH. The pump system will be equipped with a automatic backup generator.

Friction loss with forcemains (FM) to Rt 17k are as follows:

1400 ft 3" FM through site to Lakeside Road at 80 gpm =22 ft. 1400 ft existing 4" FM in Lakeside to 17k at 80 gpm = 6 ft Total elevation life from Pump Station to 17k = 30 ft Operation pressure in 17 k = 30 psi = 69 ft. Total Dynamic Head = 127 ft (55 psi)

Minimum Velocity in 3" FM = 3.5 Fps Minimum Velocity in 4" FM = 2 Fps See Pump Curve in Appendix A

#### IV. Fire Flow:

The proposed three buildings with 102 senior housing units will be Type V construction and regiment a fire flow sprinkler system. The fire flow regiment based on NFPA 1142 is 1000 Gallon per minute (see appendix B for calculation). The proposed 8" water main extending to the site will be capable of this flow. A detail analysis will be performed with the building design and building permit application process. Current flow data for the existing 12" main in Lakeside will be obtained during this process.

Appendix - A

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### Sewer Prep Specifications

### **TECHNICAL BROCHURE**



#### FEATURES

Design: Capable of grinding municipal, commercial and industrial sewage.

Cutter System: Designed to reduce sewage to a fine slurry.

Impeller: Cast iron, semi-open, nontoverloading multivane design with pump-out vanes for mechanical seal protection.

Casing: Cast iron, volute type for high efficiency. Adaptable for slide rail system.

Paint: Two coat paint system for superior surface protection.

Float Leakage Sensor (FLS): a small internal float switch is used to detect the presence of water in the stator chamber. Standard on all models.

Leakage Sensor Detector Circuit: The FLS, when activated, will cause the patented 24 volt MiniCAS monitoring relay to signal an alarm and, if desired, stop the pump. The MiniCAS 24 volt relay can be ordered separately for installation in a control panel by a UL or CSA certified panel shop or as a built-in option in our control panel.





11/2" AND 2" DISCHARGE SUBMERSIBLE GRINDER PUMPS



a xylem brand



Wastewater

# **Goulds Pumps** 2GA 7.5HP Three Phase "A" Curve





Goulds Pumps is a brand of ITT Residential and Commercial Water. www.goulds.com

Engineered for life

Wastewater



# **Goulds Pumps**

1GA & 2GA MOTOR AND APPLICATION DATA

### **MOTOR DATA**

MODEL	HP	PHASE	VOLTS	RPM	MAX AMPS	SERVICE FACTOR	START AMPS	LOCKED ROTOR AMPS	KVA CODE	FULL LOAD MOTOR EFF
1GA71G1HD	2				12.0	1.5	62.0	52.0	D	77.1
1GA71G1LD				3450	13.0		02.0	52.0		77.1
1GA81H1GD	3	1	230	5450	21.5	1.8	140.0	100.0	E	80.0
2GA81H1KD					20.8		140.0	100.0	۳. ۱	ov.u
2GA31J1FD	5			3430	30.0	1.88	205.0	170.0	D	79.5
2GA31J1JD				5.55	34.2	1.00	200.0	170.0		/9.3
1GA71H2CD			200		12.0		76.0	62.0		
1GA71H3CD	3		230		10.0	1.33	66.0	54.0	1	04 5
1GA71H4CD			460		5.0		33.0	27.0	]	81.5
1GA71H5CD			575		4.0		26.0	22.0		
1GA81J2BD			200		17.0		115.0	79.0	- F	· · · · · · · · · · · · · · · · · · ·
1GA81J38D			230	3450	15.0		110.0	75.0		
1GA81J4BD			460	0140	7.6		60.0	41.0		
1GA81J5BD	5		575		6.0	1.2	44.0	30.0		79.5
2GA81J2ED			200		17.0	1,2	115.0	79.0		/9.5
2GA81J3ED		3	230		15.0		110.0	75.0		
2GA81J4ED			460		7.6		60.0	41.0	1	
2GA81J5ED			575		6.0		44.0	30.0	]	
2GA31K2AD			200		30.0		281.0	189.0		
2GA31K3AD			230		26.0		244.0	164.0	]	
2GA31K4AD		,	460		13.0		122.0	82.0	]	
2GA31K5AD	7.5	K	575	3475	10.0	1,47	94.0	66.0	G	84.5
2GA31K2DD	1	,	200		30.0		281.0	189.0	]	04.3
2GA31K3DD			230		26.0		244.0	164.0		
2GA31K4DD			460		13.0		122.0	82.0	]	
2GA31K5DD			575		10.0		94.0	66.0	]	

## GOULDS PUMPS

Goulds Pumps is a brand of ITT Residential and Commercial Water.

www.goulds.com

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### <u>Appendix - B</u>

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### Fire Safety Calculation for Water Storage and Water Flow

The following method has been provided by the National Fire Prevention Association (NFPA) to determining water storage and water flow needs in case of extreme event such as fire. The following data has been determined by NFPA 1142 "Standard on Water Supplies for Suburban and Rural Fire Fighting."

### Water Storage

- 1. Over approximate area of buildings =  $77' * 216' = 16632 \text{ ft}^2$
- Height of building section 'H' of NFPA 1142 states 1/2 of roof peak from eave= 8ft \* 3 (floors) + (1/2) 10ft(roof peak) = 29 ft
- 3. Total volume (cubic feet) = 16632 ft<sup>2</sup> \*(29 ft) =VS<sub>tot</sub>= 482328 ft<sup>3</sup>
- 4. Occupancy hazard rating as seen in chapter 5 (for an apartment, nursing, or convalescent home)=OHC= 7
- Construction classification Number as seen in chapter 6 ( for wood structures) = C.C. = 1.5

By utilizing NFPA's formula in chapter 4 :

$$WS_{\text{min}} = \frac{VS_{\text{tor}}}{OHC}(CC)$$
[4.2.1]

where:

WS min = minimum water supply in gal (For results in L, multiply by 3.785.)

VS tot = total volume of structure in ft<sup>3</sup> (If volume is measured in m<sup>3</sup>, multiply by 35.3.)

OHC = occupancy hazard classification number

CC = construction classification number

So,

$$WS_{min} = \left(\frac{482328 \, cf}{7}\right) * 1.5 = 103356 \, gal$$

Therefore, a storage of 103,356 gallon would be needed in case of an event of fire and by NFPA chapter 4 table 4.6.1 Water Delivery Rate would need to be 1000 gal/min.



Mail Processing Center Federal Aviation Administration Southwest Regional Office Obstruction Evaluation Group 10101 Hillwood Parkway Fort Worth, TX 76177

Aeronautical Study No. 2017-AEA-5396-OE

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Issued Date: 06/14/2017

Barry Medenbach PE Lakeside 4305 US Hwy 209 Stone Ridge, NY 12484

### **\*\* DETERMINATION OF NO HAZARD TO AIR NAVIGATION \*\***

The Federal Aviation Administration has conducted an aeronautical study under the provisions of 49 U.S.C., Section 44718 and if applicable Title 14 of the Code of Federal Regulations, part 77, concerning:

Structure:	Building Lakeside Senior Build #1
Location:	Newburgh, NY
Latitude:	41-31-31.50N NAD 83
Longitude:	74-06-37.10W
Heights:	508 feet site elevation (SE)
	42 feet above ground level (AGL)
	550 feet above mean sea level (AMSL)

This aeronautical study revealed that the structure does not exceed obstruction standards and would not be a hazard to air navigation provided the following condition(s), if any, is(are) met:

It is required that FAA Form 7460-2, Notice of Actual Construction or Alteration, be e-filed any time the project is abandoned or:

\_\_\_\_ At least 10 days prior to start of construction (7460-2, Part 1)

X\_\_\_\_ Within 5 days after the construction reaches its greatest height (7460-2, Part 2)

Based on this evaluation, marking and lighting are not necessary for aviation safety. However, if marking/ lighting are accomplished on a voluntary basis, we recommend it be installed in accordance with FAA Advisory circular 70/7460-1 L Change 1.

The structure considered under this study lies in proximity to an airport and occupants may be subjected to noise from aircraft operating to and from the airport.

This determination expires on 12/14/2018 unless:

- (a) the construction is started (not necessarily completed) and FAA Form 7460-2, Notice of Actual Construction or Alteration, is received by this office.
- (b) extended, revised, or terminated by the issuing office.
- (c) the construction is subject to the licensing authority of the Federal Communications Commission (FCC) and an application for a construction permit has been filed, as required by the FCC, within

6 months of the date of this determination. In such case, the determination expires on the date prescribed by the FCC for completion of construction, or the date the FCC denies the application.

NOTE: REQUEST FOR EXTENSION OF THE EFFECTIVE PERIOD OF THIS DETERMINATION MUST BE E-FILED AT LEAST 15 DAYS PRIOR TO THE EXPIRATION DATE. AFTER RE-EVALUATION OF CURRENT OPERATIONS IN THE AREA OF THE STRUCTURE TO DETERMINE THAT NO SIGNIFICANT AERONAUTICAL CHANGES HAVE OCCURRED, YOUR DETERMINATION MAY BE ELIGIBLE FOR ONE EXTENSION OF THE EFFECTIVE PERIOD.

This determination is based, in part, on the foregoing description which includes specific coordinates, heights, frequency(ies) and power. Any changes in coordinates, heights, and frequencies or use of greater power will void this determination. Any future construction or alteration, including increase to heights, power, or the addition of other transmitters, requires separate notice to the FAA.

This determination does include temporary construction equipment such as cranes, derricks, etc., which may be used during actual construction of the structure. However, this equipment shall not exceed the overall heights as indicated above. Equipment which has a height greater than the studied structure requires separate notice to the FAA.

This determination concerns the effect of this structure on the safe and efficient use of navigable airspace by aircraft and does not relieve the sponsor of compliance responsibilities relating to any law, ordinance, or regulation of any Federal, State, or local government body.

If we can be of further assistance, please contact our office at (404) 305-6531, or darin.clipper@faa.gov. On any future correspondence concerning this matter, please refer to Aeronautical Study Number 2017-AEA-5397-OE.

(DNE)

Signature Control No: 332347918-335177254 Darin Clipper Specialist

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Attachment(s) Case Description Map(s)

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Proposed 3 - 3 story apartments building for senior housing

Page 3 of 4

### TOPO Map for ASN 2017-AEA-5396-OE





Mail Processing Center Federal Aviation Administration Southwest Regional Office Obstruction Evaluation Group 10101 Hillwood Parkway Fort Worth, TX 76177

Issued Date: 06/14/2017

Barry Medenbach PE Lakeside 4305 US Hwy 209 Stone Ridge, NY 12484

### **\*\* DETERMINATION OF NO HAZARD TO AIR NAVIGATION \*\***

The Federal Aviation Administration has conducted an aeronautical study under the provisions of 49 U.S.C., Section 44718 and if applicable Title 14 of the Code of Federal Regulations, part 77, concerning:

Structure:	Building Lakeside Senior Build #2
Location:	Newburgh, NY
Latitude:	41-31-34.50N NAD 83
Longitude:	74-06-35.50W
Heights:	510 feet site elevation (SE)
	42 feet above ground level (AGL)
	552 feet above mean sea level (AMSL)

This aeronautical study revealed that the structure does not exceed obstruction standards and would not be a hazard to air navigation provided the following condition(s), if any, is(are) met:

It is required that FAA Form 7460-2, Notice of Actual Construction or Alteration, be e-filed any time the project is abandoned or:

At least 10 days prior to start of construction (7460-2, Part 1)

\_X\_\_ Within 5 days after the construction reaches its greatest height (7460-2, Part 2)

Based on this evaluation, marking and lighting are not necessary for aviation safety. However, if marking/ lighting are accomplished on a voluntary basis, we recommend it be installed in accordance with FAA Advisory circular 70/7460-1 L Change 1.

The structure considered under this study lies in proximity to an airport and occupants may be subjected to noise from aircraft operating to and from the airport.

This determination expires on 12/14/2018 unless:

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- (c) the construction is subject to the licensing authority of the Federal Communications Commission (FCC) and an application for a construction permit has been filed, as required by the FCC, within

Proposed 3 - 3 story apartments building for senior housing

Aeronautical Study No. 2017-AEA-5398-OE



Mail Processing Center Federal Aviation Administration Southwest Regional Office Obstruction Evaluation Group 10101 Hillwood Parkway Fort Worth, TX 76177

Issued Date: 06/14/2017

Barry Medenbach PE Lakeside 4305 US Hwy 209 Stone Ridge, NY 12484

### **\*\* DETERMINATION OF NO HAZARD TO AIR NAVIGATION \*\***

The Federal Aviation Administration has conducted an aeronautical study under the provisions of 49 U.S.C., Section 44718 and if applicable Title 14 of the Code of Federal Regulations, part 77, concerning:

Structure:	Building Lakeside Senior Build #3
Location:	Newburgh, NY
Latitude:	41-31-37.60N NAD 83
Longitude:	74-06-33.70W
Heights:	505 feet site elevation (SE)
	42 feet above ground level (AGL)
	547 feet above mean sea level (AMSL)

This aeronautical study revealed that the structure does not exceed obstruction standards and would not be a hazard to air navigation provided the following condition(s), if any, is(are) met:

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This determination is based, in part, on the foregoing description which includes specific coordinates, heights, frequency(ies) and power. Any changes in coordinates, heights, and frequencies or use of greater power will void this determination. Any future construction or alteration, including increase to heights, power, or the addition of other transmitters, requires separate notice to the FAA.

This determination does include temporary construction equipment such as cranes, detricks, etc., which may be used during actual construction of the structure. However, this equipment shall not exceed the overall heights as indicated above. Equipment which has a height greater than the studied structure requires separate notice to the FAA.

This determination concerns the effect of this structure on the safe and efficient use of navigable airspace by aircraft and does not relieve the sponsor of compliance responsibilities relating to any law, ordinance, or regulation of any Federal, State, or local government body.

If we can be of further assistance, please contact our office at (404) 305-6531, or darin.clipper@faa.gov. On any future correspondence concerning this matter, please refer to Aeronautical Study Number 2017-AEA-5398-OE.

( DNE )

Signature Control No: 332347919-335177256 Darin Clipper Specialist

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Attachment(s) Case Description Map(s) Proposed 3 - 3 story apartments building for senior housing

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T100	COVER SHEET & RENDERING
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A200	BLDG. #1 & 2 ELEVATIONS
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A300	UNITS A & B - PLAN DETAILS

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Proposed New Multi-Family Residence for: LAKESIDE SENIOR HOUSING Lakeside Road Town of Newburg, New York 12550

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BUILDING	#
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GROSS SQUARE FOOTAGE / BUILDING: BUILDING #1, 2,& 3/FL 13,515 sq.ft. BUILDING #1, &2/BLDG 40,545 sq.ft. BUILDING #3 / BLDG 27,030 sq.ft. DVELLING UNIT CALC, per BUILDING: BUILDING #1 & #2 24 @ 2BR 12 @ 1BR TOTAL UNITS/BLDG 36 UNITS BUILDING #3 30 @ 2BR 6 @ 1BR TOTAL UNITS/BLDG 36 UNITS	Proposal for Renovation LAKE,SIDE SENIOR HOUSING LAKESIDE ROAD TOWN of NEWBURG, NEW YORK 12550
	JAMES LYMAN REYNOLDS ARCHITEKT 4303 US Route 209 Bibore Rüge, NY 12834 Ph (243) BOT-3101 ISSUE: Design Development SET 6-20-16 ADDENDA: ADDENDA: COVER SHEET 5/64 <sup>-</sup> =1 <sup>-</sup> -0 <sup>+</sup> Sect # 1/64 <sup>-</sup> =1 <sup>-</sup> -0 <sup>+</sup> Design Development ADDENDA: ADDENDA: T100

-206'-6" CP C TORCH 8 Line . COMMUNITY Unit B Unit 'A' Unit 'A ROOM Unit 'A' 8 029 Ĵ, 88.00 18.81 DØ, T Dinne Ale THE A Unit 'A' Unit 'A Unit B Unit 'A' Unit 'B' LIVING ROOM 1 趨難 CO.C. PORCH DORGH COVERED FORCH DWELLING UNIT CALC. per FLOOR: BUILDING #1 & #2 BUILDING # 1 & 2 - FIRST FLOOR PLAN TOTAL UNITS/FLOOR 11 UNITS SCALE 1/16"=1"-0"

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BED ROOM Ŗ BATH <u>\_</u><u>E</u>. 23'-

FLOOR PLAN -UNIT 'A' @ 900 s.f.
 SCALE 3/18"≈1'-0"
 SCALE 3/18"≈1'-0"

(1) FLOOR PLAN - UNIT 'B' @ 690 s.f.





						EVICTINO
PROJECT PARC	EL BOUNDARY		PROPOSED RELOCATED LIGHT FIXTURE			EXISTING
ADJACENT PAR	CEL BOUNDARY		EXISTING FENCE		·s	PROPOSE
PROPOSED STR	RUCTURE	<del></del>	PROPOSED GUIDE RAIL		8	PROPOSEI
EXISTING STRU	CTURE	- <u>u</u>	EXISTING GUIDE RAIL		s	EXISTING
ELV-EXISTING 1 FO	OT CONTOUR		PROPOSED RETAINING WALL		\$	EXISTING
ELVEXISTING 5 FO	OT CONTOUR		PROPOSED STORM DRAIN	}	x" w	PROPOSEI
-ELV PROPOSED 1 I	FOOT CONTOUR		PROPOSED CATCH BASIN	<u> </u>	x" w —	EXISTING
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PROPOSED CU		)	PROPOSED STORMWATER MANHOLE			EXISTING
EXISTING CURB			EXISTING STORM DRAIN		-@	EXISTING

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Test Hol	e Data
21 Lakeside	Senior Housing
15-Sep-16	
	· ·
<u>TH#1:</u>	
0-6"	Topsoil
6"-48"	Gravelly silt loam with broken & weathered shale
48"	Shale-no water or mottling
<u>TH#2</u>	
0-6"	Topsoil
6"-72"	Gravelly silt loam-no water, rock or mottling
TH#3	
0-6"	Topsoil
6"-72"	Gravelly silt loam-no water, rock or mottling
TH#4	
0-6"	Topsoil
6"-60"	Gravelly silt loam with broken shale
<u></u>	
0-6"	Topsoil
6"-18"	Gravelly silt loam
18"-36"	Fractured shale
36"	Shale, no water or mottling
TH#6	
0-6"	Topsoil
6"-60"	Gravelly silt loam
60"	Broken shale, no water

ABBR	EVIATION
TC	TOP OF
G	GROUND
	PAVEME
FF	FINISH F
NV	INVERT
TG	TOP OF
HP	HIGH PC
	PAVEME
·	DEBBES

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YMBOL	SPECIES	COMMON NAME	QUANTITY	SIZE UPON PLANTING	HEIGHT	4
	Hamamelis Virginiana	American Witchhazel	4	3 Gal. Cont.	36" TO 48"	
$\bigcirc$	llex	China Girl Holly Bush	36	3 Gal. Cont.	24" TO 36"	
Share of the state	Spirea	Limemound Spirea	22	3 Gal. Cont.	12" TO 18"	
Junity · Mary	Spirea	Little Princess Spirea	60	3 Gal. Cont.	12" TO 18"	TED MAP
	Viburrium dentatum	Arrowwood	11	3 Gal. Cont.	24" TO 36"	AS PER FILED WAP
$\Im$	Buxus Microphylla	Winter Gem Boxwood	41	3 Gal. Cont.	12" TO 18"	
	Syringa Patula	Miss Kim Lilac	30	5 Gal. Cont.	2' TO 3'	
	Rosa 'Knockout'	Knockout Rose	7	3 Gal. Cont.	2' TO 3'	
$\mathfrak{B}$	Cornus stolonifera	Red Osier Dogwood	17	3 Gal. Cont.	±18"	
	Aster novae-angliae	New England Aster	9	1 Gal. Cont.	±18*	
$\tilde{\mathbf{O}}$	Iris versicolor	Blue Flag Iris	42	1 Gal. Cont.	±18"	(3) Norway Spruce
${}$	Hemerocallis	Stella Doro Daylily	29	1 Gal. Cont.	12" TO 18"	LEAR -
0	Lobelia siphatica	Great Blue Lobelia	34	1 Gal. Cont.	±2'	1 Sent it
繁	Rudbeckia lacinata	Cutleaf Coneflower	14	1 Gal. Cont.	±18"	A The second sec
	Mondarda fistulosa	Wild Bergarnot	43	1 Gal. Cont.	2' TO 3'	
	Scirpus cyprinus	Woolgrass	35	1 Gal. Cont.	±3'	1
	Spartina Pectinata	Cordgrass	23	3 Gal. Cont.	3' TO 4'	
	Panicum virgatum	Switchgrass	9	3 Gal. Cont.	3' TO 4'	
$\bigcirc$	Calamagrostis Canadensis	Blue Joint	8	1 Gal. Cont.	<b>±3'</b>	
	<u></u>	TREE SCHEDULE				
YMBOL	SPECIES	COMMON NAME	QUANTITY	SIZE UPON PLANTING		
YMBOL	Acer rebrum	Red Maple	6	2.5"-3" Caliper	±14'	
	Betula nigra	River Birch	4	2.5"-3" Caliper	±12'	-
					±6'	-
	Cornus amomum	Silky Dogwood	5	3 Gal. Cont.		-
$\odot$	Quercus bicolor	Swamp White Oak	2	2.5"—3" Caliper	±14'	
$\odot$	Gleditsia triacanthos	Thorniess Honey Locust	15	2.5"—3" Caliper	±14'	
R	Tilia Americana "Redmond"	Redmond Linden	3	2.5"—3" Caliper	±14'	
	Pyrus calleryana 'Holmford'	New Bradford Pear	9	2.5"-3" Caliper	±12'	
	Malus floribunda	Flowering Crabapple	1	2.5"-3" Caliper	±8'	-
	<u></u>		8	2-1/2" - 3" Caliper	±8'	-

# SEEDING MIX FOR SITE LAWNS

APPLICATION	SPECIES	% PURE LIVE SEED	APPLICATION RATE	FERTILIZER	LIMING RATE	SEEDING DATE
TEMPORARY	ANNUAL RYE	88.2%	10 LBS./1000 S.Y.	5-5-5 AT 207 LBS./ 1000 S.Y.	413 LBS./ 1000 S.Y.	3/15 TO 10/15
PERMANENT	PERENNIAL RYE	88.2%	4 LBS./1000 S.Y.			3/15 TO 6/1
	KENTUCKY BLUE GRASS MIX*	78.4%	6 LBS./1000 S.Y.	SEE NOTE 1	800 LBS./ 1000 S.Y.	AND 9/1 TO 10/15
	CREEPING RED	83.3%	11 LBS./1000 S.Y.	BELOW		9/11010/15
	TALL FESCUE (VAR.	83.5%	7.5 LBS./1000 S.Y.			
PERMANENT	KENTUCKY 31) BIRDSFOOT TREFOIL MIX	**78.4%	2.0 LBS./1000 S.Y.	SEE NOTE 1 BELOW	800 LBS./ 1000 S.Y.	4/1 TO 6/15 AND 9/1 9/15
	REDTOP	73.6%	1.0 LBS./1000 S.Y.		<u> </u>	<u> </u>

1. FERTILIZER SHALL BE APPLIED IN ACCORDANCE WITH A SOIL TEST. IN THE ABSENCE OF A SOIL TEST, FERTILIZER SHALL BE APPLIED AS FOLLOWS:

A. 10-20-20 ANALYSIS COMMERCIAL FERTILIZER AT 140 LBS./1000 S.Y.

\*\* MINIMUM 20% HARDSEED AND 60% NORMAL SPROUTS.

- 38-0-0 UREA FORM FERTILIZER AT 50 LBS./1000 S.Y.
- B. 32-0-0 TO 38-0-0 SULFUR COATED UREA FERTILIZER AT 59-50 LBS./1000 S.Y.
  C. 31-0-0 IBDU FERTILIZER AT 61 LBS./1000 S.Y.
- 2. ALL SEEDED AREAS SHALL BE MULCHED WITH HAY OR STRAW APPLIED AT A RATE OF 6000 LBS./AC.
- 3. ALL AREAS RECEIVING SEEDING SHALL HAVE A MINIMUM OF 6" OF ORGANIC TOPSOIL. (1240 LBS./1000 S.Y.), MULCH TO BE ANCHORED WITH WOOD CELLULOSE FIBER AT 750 LBS./AC. OR EQUAL. \* BLUEGRASS MIX: A COMBINATION OF CERTIFIED VARIETIES EACH AT 25% OR LESS OF MIX.

# LANDSCAPING NOTES:

- . The contractor shall furnish and plant all plants in quantities as shown on this plan. No substitutes will be permitted unless approved by the owner. All plants shall be nursery grown.
- Plants shall be in accordance with the current "American Association for Nursery Stock" as published by the American Association of Nurserymen.
- Plant stock shall be grown within the hardiness zone 5 established by the plant hardiness zone map, miscellaneous publications no.814, agricultural research service, United States Department of Agriculture, latest revision.
- 4. All plants must be moved with the root systems as solid units with the balls of earth firmly wrapped with burlap. No plants shall be accepted when the ball of earth surrounding its roots has been badly cracked or broken before planting. All plants shall be freshly dug. All plants that cannot be planted at once must be heeled-in by setting in the ground, and covering the balls with soil and then watering during transport. All plant materials shall be wrapped with wind proof covering.
- Plant material shall bear the same relationship to finished grade as to the original planting grade prior to digging.
- 6. All disturbed areas not to be paved or otherwise treated shall receive four (4) inch loam and
- See planting details and specifications for additional requirements.
- Tree stakes and wrap shall remain in place for no less than 6 months and no more than 1 year.
- Planting shall be completed from April 1st through November 1st.
- 10. Maintenance shall consist of keeping the plants in a healthy growing condition and shall include weeding, cultivating, remulching, tightening and repairing of guys, removal of dead material, resetting plants to proper grades or upright position and maintaining the planting saucer.
- 11. All vegetation shown on this plan shall be maintained in a healthy and vigorous growing condition throughout the duration of the proposed use. All vegetation not so maintained shall be replaced with new same size and type vegetation at the beginning of the next planting year.
- 12. Replacements shall conform in all respects to the specifications for new plants and shall be planted in the same manner.





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CONSTRUCTION SPECIFICATIONS

- 1. DESIGNATED TEMPORARY, BELOW GROUND CONCRETE WASHOUT FACILITIES WILL BE CONSTRUCTED AS SHOWN ABOVE. WASHOUTS WILL BE CENTRALLY LOCATED AT THE DISCRETION OF THE INDIVIDUALS WHO MANAGE DAY TO DAY CONSTRUCTION ACTIVITIES. WASHOUTS SHALL HAVE A MINIMUM LENGTH AND WIDTH OF 10 FEET BUT MUST HAVE SUFFICIENT VOLUME TO CONTAIN ALL LIQUID CONRETE WASTES GERNERTATED FROM WAHOUT OPERTIONS. THE WASHOUT AREAS WILL BE LINED WITH PLASTIC SHEETING AT LEAST 10 MILS THICK AND FREE OF ANY HOLES OR TEARS. SIGNS WILL BE POISTED MARKING THE LOCATION OF THE WAHOUT AREAS.
- 2. TEMPORARY CONCRETE WASHOUT FACILITIES WILL BE LOCATED A MINIMUM OF (50 FEET) FROM DRAIN INLETS.
- 3. KEEP THE WASHOUT AREAS WILL BE INSPECTED DAILY TO ENSURE THAT ALL CONRETE WASHING IS BEING DISCHARDE INTO THE WASHOUT AREA, NO LEAKS OR TEARS ARE PRESENT, AND TO IDENTIFY WHEN CONCRETE WASTES NEED TO BE REMOVED. THE WAHOUT AREAS WILL BE CLEANED OUT ONCE THE AREA IS FILLED TO 75 PERCENT OF THE HOLDING CAPACITY. ONCE THE AREA'S HOLDING CAPACITY HAS BEEN REACHED THE CONRETE WASTES WILL BE ALLOWED TO HARDEN, THE CONRETE WILL BE BROKEN UP, REMOVED, AND DISPOSED IN ACCORDANCE WITH LOCAL REGULATIONS. THE PLASTIC SHEET WILL BE REPLACED IF TEARS OCCCUR DURING REMOVAL OF CONCRETE WASTES FROM THE WASHOUT AREA.

5 CONCRETE WASHOUT DETAIL

DATE REVISION	ВҮ
SITE DETAILS	
FOR SENIOR HOUSING AT	
21 LAKESIDE PROPERTIES IN	C.
SITUATE - LAKESIDE ROAD	
TOWN OF NEWBURGH ORANGE COUNTY, NEW YORK	
FEBRUARY 8, 2016	
MEDENBACH & EGGERS	
CIVIL ENGINEERING & LAND SURVEYING, STONE RIDGE, NEW YORK (845) 687-0047	P.C.
Dan Meder hal	
BARRY MEDENBACH, P.E. NEW YORK LIC, NO. 60142	D2 SHEET 13 OF 17

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# Water Main Notes and Specifications

## General Provisions:

2. The most recent revisi Ductile Iron Pipe Valves - Metal Seate Hydrants - Wet Barr Hydrants - Dry Barre **Valves-Resiliant Sea** Pipe Laying **Hydrostatic Testing** Disinfection Service Lines, Corp.

Ductile Iron Fitting

3. Water lines shall be equiped with Megalug - series 1100 for pipe restraining, or as required by Town Water Dept.

4. All water lines shall be installed a minimum of 4.5 (four and half) feet below grade. The water line maybe flexed within pipe specifications or laid deeper in areas where crossings with the sanitary line occur, to achieve the required 18 inch vertical separation distance. (See sewer specifications for further information)

5. Water line is to be pressure tested and leakage tested in accordance with Great Lakes Upper Mississippi River Board of State and Provincial Public Health and Environmental Managers' Recommended Standards for Water Works, Section 8.7.6 2012, (AWWA C-600-05).

7. All water lines shall be in compliance with the "No Lead" law for waterworks.

8. Whenever pipe laying is not actively in progress, the open ends of the pipe must be closed by a temporary watertight plug or cap to prevent soil, water or other foreign matter from entering the

9. Deflection of pipes at a joint must not exceed 80% of the manufactures recommended maximum 10. Sufficent notice must be given to the head of the municipal water department, the privated owner or a designated representative of any testing so that they can witness if desired.

hours apart 12.1. One from each 1200' of new watermain 12.2. Once from each branch of the watermain

13. The tablet method of chlorinating the watermain, as described in AWWA, C651 is not acceptable

14. Refer to sections 8.7-8.13 of the "Recommended Standards for Water Works" for the installation seperation and protection of watermains.

Original 12-06-96 Revised 04-24-02 Revised 01-2015 TOWN OF NEWBURGH WATER SYSTEM NOTES FOR SITE PLANS

2. All water service lines four (4) inches and larger in diameter shall be cement lined class 52 ductile iron pipe conforming to ANSI\AWWA C151\A21.51 for Ductile Iron Pipe, latest revision. Joints shall be either push-on or mechanical joint as required.

3. Thrust restraint of the pipe shall be through the use of joint restraint. Thrust blocks are not acceptable. Joint restraint shall be through the use of mechanical joint pipe with retainer glands. All fittings and valves shall also be installed with retainer glands for joint restraint. Retainer glands shall be EBBA Iron Megalug Series 1100 or approved equal. The use of a manufactured restrained joint pipe is acceptable with prior approval of the Water Department.

4. All fittings shall be cast iron or ductile iron, mechanical joint, class 250 and conform to ANSI\AWWA C110\A21.10 for Ductile and Gray Iron Fittings or ANSI\AWWA C153\A21.53 for Ductile Iron Compact Fittings, latest revision.

5. All valves 4 to 12 inches shall be Resilient Wedge Gate Valves conforming to ANSI\AWWA C509 such as Mueller Model A-2360-23 or approved equal. All gate valves shall open left (counterclockwise).

6. Tapping sleeve shall be mechanical joint such as Mueller H-615 or equal. Tapping valves 4 to 12 inches shall be Resilient Wedge Gate Valves conforming to ANSI\AWWA C509 such as Mueller Model T-2360-19 or approved equal. All tapping sleeves and valves shall be tested to 150 psi minimum; testing of the tapping sleeve and valve must be witnessed and accepted by the Town of Newburgh Water Department prior to cutting into the pipe. Original 12-06-96 Revised 04-24-02 Revised 01-2015

7. All hydrants shall be Clow-Eddy F-2640 conforming to AWWA Standard C-502, latest revision. All hydrants shall include a 5 ¼ inch main valve opening, two 2 ½ inch diameter NPT hose nozzles, one 4 inch NPT steamer nozzle, a 6 inch diameter inlet connection and a 1 % inch pentagon operating nut. All hydrants shall open left (counter-clockwise). Hydrants on mains to be dedicated to the Town shall be Equipment Yellow. Hydrants located on private property shall be Red.

8. All water service lines two (2) inches in diameter and smaller shall be type K copper tubing. Corporation stops shall be Mueller H-15020N for ¾ and 1 inch, Mueller H-15000N or B-25000N for 1 ½ and 2 inch sizes. Curb valves shall be Mueller H-1502-2N for % and 1 inch and Mueller B-25204N for 1 %and 2 inch sizes. Curb boxes shall be Mueller H-10314N for ¾ and 1 inch and Mueller H-10310N for 1 ½ and 2 inch sizes.

9. All pipe installation shall be subject to inspection by the Town of Newburgh Water Department. The contractor shall be responsible for coordinating all inspections as required with the Town of Newburgh Water Department.

10. The water main shall be tested, disinfected and flushed in accordance with the Town of Newburgh requirements. All testing, disinfection and flushing shall be coordinated with the Town of Newburgh Water Department. Prior to putting the water main in service satisfactory sanitary results from a certified lab must be submitted to the Town of Newburgh Water Department. The test samples must be collected by a representative of the testing laboratory and witnessed by the Water Department.

11. The final layout of the proposed water and/or sewer connection, including all materials, size and location of service and all appurtenances, is subject to the review and approval of the Town of Newburgh Water and/or Sewer Department. No permits shall be issued for a water and/or sewer connection until a final layout is approved by the respective Department.

1. All water lines shall be Class 52 ductile (AWWA C151) iron pipe unless otherwide noted or approved by engineer. All ductile fittings are to meet AWWA Standards C110.

sion of the AWM	A standards are to be used	•
	C151	
d .	C500	
el	C502	
el	C503	
t	C509	
	C600	
	C600	
	C651	
& Curb Stops	C800	
-	C110	

6. Water line is to be disinfected in accordance with Great Lakes Upper Mississippi River Board of State and Provincial Public Health and Environmental Managers' Recommended Standards for Water Works, Section 8.7.6 2012 (AWWA C-651).

11. The head of the municipal water department or the private owner or their designated representative must review and accept the testing, hydrostatic and bacteriological, as adequate.

12. Bacteriological testing must include two consecutive sets of acceptable samples taken at least 24

12.3. One from each end of the watermain.

1. "Construction of potable water utilities and connection to the Town of Newburgh water system requires a permit from the Town of Newburgh Water Department. All work and materials shall conform to the requirements of the NYSDOH and the Town of Newburgh."

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Water Main Notes and Specifications Continued

**Pressure Test Procedure:** 

- 1. After trench has been backfilled. hydrostatic acceptance tests, consisting of a pressure test and a leakage test shall be performed on all sections of water mains installed. leakage test shall be conducted concurrently with pressure test. Test section shall be limited to about 2000 ft (max.) unless otherwise approved by the engineer.
- 2. After all tests and inspections have been performed evidence of compliance shall be forwarded to owner/engineer prior to acceptance.
- 3. All water for tests shall be furnished and disposed of by the contractor at the contractor's expense. Source and/or quality of water which the contractor proposes to use in testing lines shall be acceptable to the engineer
- 4. For the pressure test, system shall be pressurized and maintained at a minimum of 150 psi, or 1.5 times the working pressure, whichever is greater, based on the elevation of the lowest point in the section being tested and corrected to the elevation of the gauge. Provisions shall be made to relieve air trapped at high points in the system through adjacent hydrants or through taps and corporation stops installed for this purpose by the contractor. After said pressure has been maintained successfully, with further pumping as required, for a period of at least two hours. The section under test shall be considered to have passed the pressure test.
- 5. Leakage test shall be performed concurrently using a minimum test pressure of 150 psi, or 1.5 times the working pressure. whichever is greater. Based on the elevation of the lowest point in the section under test and corrected to elevation of the gauge. leakage test duration shall be a minimum of 2 hours after leakage rate has stabilized.
- 6. Maximum allowable leakage shall be as shown in the following table: allowable leakage per 1000 ft of pipeline per hour (gph)

Avg Test Pressure	Nominal Pipe Dia. Inches		
PSI (BAR)	<u>2</u> *	<u>4"</u>	<u>6</u>
450 (31)	0.32	0.64	0.95
400 (28)	0.30	0.60	0.90
350 (24)	0.28	0.56	0.84
300 (21)	0.26	0.52	0.78
275 (19)	0.25	0.50	0.75
250 (17)	0.24	0.47	0.71
225 (16)	0.23	0.45	0.68
200 (14)	0.21	0.43	0.64
175 (12)	0.20	0.40	0.59
150 (10)	0.19	0.37	0.55
125 (9)	0.17	0.34	0.50
100 (7)	0.15	0.30	0.45

**Disinfection Procedure:** 

- Water from an approved source of supply shall be made to flow at a constant rate in to the newly laid water main.
- 2. Water entering the new main shall receive a dose of chlorine fed at a constant rate such that the water will not have less than 25 mg/l free chlorine.
- 3. Measure chlorine concentration at regular intervals. Chlorine application shall not cease until the entire main is filled with heavily chlorinated water. The chlorinated water shall be retained for a minimum of 24 hours. The treated water in all portions of the main at the end of the 24 hour period shall have a residual of not less than 10 mg/l free chlorine.

4. After all tests and inspections have been performed evidence of compliance shall be forwarded to owner/engineer prior to acceptance.

## Sevice Pipe Conections:

- 1. Corporation stops for three-forths-inch and one-inch service lines shall be Mueller H-15008 conductive compression or equal. Corporation stops for one-and-one-half-inch and two-inch service lines shall be Mueller H-15013 conductive compression or equal. Corporation stops shall be in accordance with AWWA C800, latest revision .
- 2. Curb stops for three-forths-inch through two inch shall be mueller H-15219 conductive compression, with drain or equal. Curb stops shall be provided with an extension service box to grade. Curb stops shall be accordance with AWWA C800, latest Revision.
- 3. Underground service lines for sizes three-forths-inch through two-inch shall be Type K copper, supplied in conformance with ASTM 888, in accordance with AWWA C800, latest revision.
- 4. Service Connections or water main extension connections of three inch or larger shall be made by means of approved tapping sleeve and tapping valve. Mechanical joint tapping sleeves shall be provided with duck-tipped end gaskets. Outlet flange be class 125, ANSI B16.1.





TYPICAL FORCEMAIN TRENCH DETAIL

7 NOT TO SCALE

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SEWER MANHOLE W/ FORCE MAIN CONNECTION DETAIL 5 NOT TO SCALE



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## **Sanitary Sewer Notes and Specifications**

**General Provisions:** 

- 1. Gravity sewer pipes shall be PVC SDR 35 with ring-tight joints in compliance with ASTM D-3212
- 2. Sewer mains in relation to water mains: where possible, sewers shall be laid at least 10 (ten) feet horizontally from any existing or proposed water main. Vertical separation shall be maintained to provide 18 (eighteen) inches between top of sewer invert of the water main at utility crossings. When not possible to obtain the proper vertical separation, SDR-26 PVC pipe shall be used 10 (ten) feet on each side of the water main being crossed.
- 3. No roof, foundation or storm drains may discharge into the sewage disposal system.
- 4. All concrete tanks, manholes and chambers etc. shall be pre cast concrete to the specifications and dimensions shown hereon. Frames and covers shall be gray iron or ductile iron. Gray iron shall conform with ASTM A 48, Class 30B and ductile iron shall conform with ASTM A 536 and be of a grade appropriate to its intended use to the dimensions and specifications as shown hereon. Any structures subject to vehicle loads shall be able to withstand an H20 loading. Shop drawings shall be submitted to the design engineer for approval prior to construction.

## Gravity Sewer System Testing:

- 1. Contractor shall inspect and test the sewer installations as required by the authority having jurisdiction when work is ready for testing. After all tests have been performed, evidence of compliance shall be forwarded to owner/engineer and the authority having jurisdiction prior to acceptance.
- 2. The contractor shall test and inspect for alignment and infiltration and exfiltration of all sanitary sewers, Infiltration or exfiltration of the sanitary sewer system shall not exceed 0.60 gal/inch of internal pipe diameter per 100' of pipeline per hour with a maximum hydrostatic head at the centerline of the pipe of 25 ft, or as required by the authority having jurisdiction.
- 3. Infiltration leakage tests shall be run on each single manhole-to-manhole section, or reach, independently of all other manhole-to-manhole sections. A pipeline section under test shall include all pipe and fittings between the two manholes plus the upstream manhole.
- 4. Each manhole-to-manhole section shall be rejected or accepted based only on results of its own independent section test and not on results of any one test run simultaneously over more than one consecutive manhole-to-manhole section. The only exception allowed: accepting several consecutive manhole-to-manhole sections based on one combined infiltration test indicating zero infiltration.
- 5. Infiltration tests shall be made by installing a flow measuring device in the downstream manhole of section being tested. Test duration shall be 24 hrs, or for shorter period, provided a steady state flow condition has been achieved in the test period, and results projected to a 24 hr period.
- 6. Exfiltration tests shall be run on each single manhole-to-manhole section, or reach, independently of all other manhole-to-manhole sections. A pipeline section under test shall include all pipe and fittings between the two man-holes plus the upstream manhole.
- 7. Exfiltration tests shall be made by measuring the drop in water elevation in the upstream manhole 24 hrs after initial water level is recorded. Initial water level in upstream manhole shall be 2 feet higher than either the top of pipe or groundwater elevation at the downstream manhole. Any manhole-to-manhole section undergoing an exfiltration test must have the next adjacent sections, both upstream and downstream, dry and not under test.
- 8. Low pressure air testing may be allowed in lieu of exfiltration tests only. When so allowed, test shall be performed under direction of engineer according to ASTM F1417. An air test shall not be run until section of line to be tested has been cleaned of all foreign material by flushing and has been visually inspected.
- 9. Sewers shall be laid with straight alignment between manholes. Straight alignment shall be checked either using a laser beam or lamping. Testing shall comply with requirements of the authority having jurisdiction.
- 10. Manholes, which cannot be properly air tested, should be visually inspected and leakage-tested using internal or external hydrostatic pressure. Leakage testing shall comply with requirements of the authority having jurisdiction.
- 11. In areas where conventional testing is impractical (i.e. areas designated by Engineer where existing services are tied into new line immediately and any blockage could result in health problems) no lines shall be backfilled until each pipe section and connection is inspected and approved
- 12. If the allowable rate of infiltration, exfiltration, or air leakage is exceeded, the contractor shall locate points of excessive leakage and shall promptly correct, repair, and bring system up to the standard. Costs of all such repairs and corrective measures, including costs of repeated tests, shall be born by contractor, the sewer line section (including manholes and building services) under test shall not be accepted until these test criteria are met.

	MAP REVISION DATES	
DATE	REVISION	BY

SEWER DETAILS FOR SENIOR HOUSING AT 21 LAKESIDE PROPERTIES INC.

> SITUATE - LAKESIDE ROAD TOWN OF NEWBURGH ORANGE COUNTY, NEW YORK **FEBRUARY 8, 2016**

MEDENBACH & EGGERS CIVIL ENGINEERING & LAND SURVEYING, P.C. STONE RIDGE, NEW YORK (845) 687-0047

> BARRY MEDENBACH, P.E. NEW YORK LIC. NO. 60142

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# PROPOSED KEY

- 1. STAINLESS STEEL LIFTING CHAIN 2. STAINLESS STEEL PUMP GUIDE RAILS
- 3. ALUMINUM LADDER WITH RUNGS AT 12" O.C. WITH RETRACTABLE 1" O.D. ALUMINUM EXTENSION TUBES FOR HANDRAIL (LOCATE TO SUIT CONDITIONS)
- 4. PUMP ELECTRICAL SERVICE, (UNDERGROUND CONDUIT) TO REMOTE MOUNTED CONTROL PANEL.
- 5. SEAL WITH NON-SHRINK GROUT SEE GENERAL NOTE #3. 6. FLOAT ELECTRICAL SERVICE (RIGID CONDUIT) TO REMOTE MOUNTED CONTROL PANEL.
- 7. EXPLOSION-PROOF SEAL (TYP. 2) 8. SEALED MERCURY SWITCH AND WATERPROOF CABLE ASSEMBLY (FLOAT SWITCHES)
- 9. STAINLESS STEEL BRACKET WITH ADJUSTABLE CABLE CONNECTORS (ACCESSIBLE THROUGH ACCESS HATCH) 10. CONCRETE PIPE SUPPORT (WHERE REQUIRED)
- 11. RESTRAINED CONNECTION: STAINLESS STEEL 1/2" TIE ROD PLATES, 2-3/4"\$ TIE RODS, BOLTS, WASHERS, AND 3/8"x4" SQUARE BACKING PLATE WITH GROUT COVER
- 12. COMPRESSION GASKET OR LINK-SEAL WITH GROUT COVER 13. SCH 80 TEE
- 14. COMPRESSION COUPLING
- 15. PVC TEE WITH THREADED REDUCER BUSHING 16. PRESSURE GAUGE ASSEMBLY WITH DIAPHRAGM SEAL AND ISOLATION VALVE 17. SCH 80 PVC TO SDR 26 PVC TRANSITION COUPLNG 18. CONCENTRIC REDUCER (IF REQUIRED)
- 19. BALL CHECK VALVE
- 20. TRUE UNION BALL VALVE 21. ALTERNATING DUPLEX "GOULDS" GRINDER PUMPS 2GA (7.5 HP)
- 22. DUPLEX CONTROL PANEL 23. OPTIONAL STANDBY GENERATOR 24. OPTIONAL AUTOMATIC GENERATOR TRANSFER SWITCH

# GENERAL NOTES

- 1. NO ELECTRICAL SPLICES, JUNCTION BOXES, OR CONNECTIONS OF ANY KIND SHALL BE IN THE PUMP CHAMBER.
- 2. PUMP CONTROLS SHALL BE WIRED INTRINSICALLY SAFE. 3. JUNCTION BOXES SHALL BE ACCESSIBLE WITHOUT NEED FOR ENTERING WETWELL. CONTRACTOR HAS THE OPTION OF PROVIDING PUMPING STATION WITH NEMA 4X JUNCTION BOX AND
- APPROPRIATE GAS SEAL-OFF FITTINGS CAST INTO TOP SLAB. 4. PUMP CONTROL PANEL TO BE PEDESTAL MOUNTED, ALL CONDUIT AND CONDUCTORS FOR BOTH POWER AND CONTROL TO BE SIZED BY BUILDING DESIGNER. CONTRACTOR TO PROVIDE LOCAL DISCONNECTS FOR THE PUMPS.
- 5. THE PUMP STATION SHALL HAVE AN ALARM SYSTEM WITH TELEMETRY THAT REPORTS TO THE HOTEL OFFICE WHICH WILL BE MANNED 24 HOURS A DAY AT THE HOTEL.

# DESIGN DATA

AVERAGE DAILY FLOW = 11.220 =G.P.D.

> PEAK FLOW = 10 X ADF = 80 gpm TOTAL DYNAMIC HEAD = 127PUMP CYCLE VOLUME = 576 gallons @ (7.2 Minutes) PUMP CYLCLE PER DAY = 19

Bedding: **Back Filling:** Procedure:

# UTILITY CONSTRUCTION AND TESTING SPECIFICATIONS:

# **General Provisions:**

1. All construction activities shall be in compliance with municipal, county state and federal regulations.

2. The protection of adjacent properties or areas on site that are not to be disturbed during construction, shall be the responsibility of the contractor.

## Excavation:

1. Excavation shall be carried to the lines, grades and slopes shown on the approved plans.

2. Where unstable or unsuitable material is encountered at the prescribed bottom grade of the trenches it shall be removed.

1. Selected bedding shall be provided for the construction of pipe foundations at those locations where the foundations or excavated material, or any portion thereof deemed to be unsuitable for supporting the pipe or structure, or for back filling the cover portion of the trenches to a level one foot above the pipe, or where excavated material consist of a predominance of large stone, boulders or rock which is not suitable for placing in the trench. Certified sieve analysis shall be submitted from the supplier for the engineer's review prior to use.

1. all back fill material shall be placed in layers not exceeding twelve (12) inches in depth, (loose measure), and shall be thoroughly tamped and compacted to a minimum density of 95% standard AASHTO-T99 (ASTM-D698, as amended) compacting test. Compacting equipment shall be of a suitable type for the various back filling operations.

## **Obstructions:**

1. Where underground or overhead obstructions are encountered in the work, the contractor shall assume all costs for direct or indirect injury to them. Any valve box, valve pit, water service, water main, catch basin, manhole etc. whether or not shown on the drawings shall be protected from damage. The contractor shall have all utilities identified and located prior to any construction.

## Sanitary Sewers:

1. Gravity sewer pipes shall be 8", 6" or 4" PVC SDR 35 with ring-tight joints in compliance with ASTM D-3212. 2. Manholes shall be pre cast concrete. Manhole is to be infiltration/exfiltration tested in accordance with NYSDEC design standards for Wastewater Treatment Works 1988

Fill manhole with water. Let sit for 24 hours. Maximum allowable rate of infiltration/exfiltration not to exceed 100 gallons per inch diameter per mile per day.

3. 10 - foot horizontal and 2 - foot vertical distance shall be maintained between all water and sewer lines.

4. No roof or foundation drains may discharge into the sewage disposal system 5. Sewer main is to tested in accordance with ASTM F 1417-92 (standard test method for installation acceptance of plastic gravity sewer lines using low-pressure air)

5.1 Clean section of sewer line to be tested by flushing or other means prior to conducting the low pressure air test. this cleaning serves to eliminate debris and produce the most consistant results. 5.2 Isolate the section of sewer line to be tested by inflatable stoppers or other suitable test plugs. 5.3 Plug or cap the ends of all branches, laterals, tees, wyes, and stubs to be included in the test to prevent air leakage. All plugs and caps shall be securely braced to prevent blowout. One of the plugs or caps should have an inlet tap, or other provision for connecting a hose to a portable air control source.

5.4 Connect the air hose to the inlet tap and portable air control source. The air equipment shall consist of necessary valves and pressure gages to control an oil-free air source and the rate at which air flows into the test section to enable monitoring of the air pressure within the test section. 5.5 Add air slowly to the test section until the pressure inside the pipe reaches 4.0 psig.

5.6 After the pressure of 4.0 psig is obtained, regulate the air supply so that the pressure is maintained between 3.5 and 4.0 psig for at least 2 min. Depending on air/ground temperature conditions. the air temperature should stabilize in equilibrium with the temperature of the pipe walls. the pressure will normally drop slightly until equilibrium is obtained; however, a minimum of 3.5 psig is required. 5.7 Determine the rate of air loss by either the constant pressure method or the time-pressure drop method (see ASTM F 1417-92 sections 8.2.1 and 8.2.2 for procedures) 5.8 Upon completion of the test, open the bleeder valve and allow all air to escape. Plugs shall not be removed until all air pressure in the test section has been reduced to atmospheric pressure. 6. Sewer shall be tested with mandrel 95% of pipe diameter for deflection and lamp tested. 7. Forcemains shall be tested using ASTM F 2164

### Forcemain Test Procedure:

1. Flush and purge all air from the piping to be tested.

2. Close off by valves or other method the piping to be tested.

3. Slowly, add water with a positive displacement pump to raise the system pressure to the maximum determined by the authority having jurisdiction. (The maximum pressure is 1.5 times the design working pressure less the elevation hydrostatic head. Typical design (maximum operating) pressures: for SDR-9 is 200 psi, for SDR-11 is 160 psi, and SDR-13.5 is 128 psi; and is to be reduced for higher temperatures.

Allow the test section of piping and test liquid to equalize in temperature. Add make up water as necessary for four (4) hours to maintain test pressure.

Reduce pressure by ten (10 psi), by letting water out and then closing the system.

Monitor for one (1) hour, do not increase pressure or add water. 8. Pass/Fail Criteria: if no leakage is visually observed and the pressure remains steady (within 5% of the pressure at item # 6) then a passing test is indicated.

### **TOWN SEWER SYSTEM NOTES**

Construction of sanitary sewer facilities and connection to the Town of Newburgh sanitary sewer system requires a permit from the Town of Newburgn Sewer Department. All construction shall conform to the requirements of the NYSDEC and the Town of Newburgh. 2. All sewer pipe installation shall be subject to inspection by the Town of Newburgh Sewer Department. The Contractor shall be responsible for coordinating all inspections as required with the Town

All gravity sanitary sewer service lines shall be 4 inches in diameter or larger and shall be SDR-35 PVC pipe conforming to ASTM D-3034-89. Joints shall be push-on with elastomeric ring gasket conforming ASTM D-3212. Fittings shall be as manufactured by the pipe supplier or equal and shall have a bell and spigot configuration compatible with the pipe.

The sewer main shall be tested in accordance with Town of Newburgh requirements. All testing shall be coordinated with the Town of Newburgh Sewer Department The final layout of the proposed water and/or sewer connection, including all materials, size and location of service and all appurtenances, is subject to the review and approval of the Town of Newburgh Water and/or Sewer Department. No permits shall be issued for a water and/or sewer connection until a final layout is approved by the respective Department.

		MAP REVISION DATES	
	DATE DATE	REVISION	BY
	03-27-2017	ADDED TOWN OF NEWBURGH SEWER NOTES, CORRECTED PIPE TYPE PER NEWBURGH TOWN NOTE	SL
	04-04-2017	ADDED DATA TO PUMP STATION AND VALVE CHAMBER DETAILS	SL
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		SEWER DETAILS	
		CONTINUED	
		FOR SENIOR HOUSING AT	
		21 LAKESIDE PROPERTIES INC.	
		SITUATE - LAKESIDE ROAD	
		TOWN OF NEWBURGH	
		ORANGE COUNTY, NEW YORK	
		FEBRUARY 8, 2016	
		MEDENBACH & EGGERS	
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Parameter	Value
PH range	5.2 to 7.00
Organic matter	1.5 to 4.0%
Magnesium	35 lbs. per acre, minimum
Phosphorus (P2O5)	75 lbs. per acre, minimum
Potassium (K <sub>2</sub> O)	85 lbs. per acre, minimum
Soluble salts	• •500 ppm
Clay	10 to 25%
Silt	30 to 55%
Sand	35 to 60%

# **General Utility Notes and Specifications:**

General Provisions

- 1. All construction activities shall be in compliance with municipal, county state and federal regulations
- 2. The protection of adjacent properties or areas on site that are not to be disturbed during construction, shall be the responsibility of the contractor.
- 3. Any conditions encountered in the field differing from those shown hereon, shall be reported to the design engineer before construction is to proceed.
- 4. Exploratory excavations shall be performed as needed at all utility connection locations by the contractor to verify existing conditions prior to work. Before connecting to existing utilities, verify existing utility inverts and notify the engineer if any deviation from the plan is required.
- Where underground or overhead obstructions are encountered in the work, the contractor shall assume all costs for direct or indirect injury to them. Any valve box, valve pit, water service, water main, catch basin, manhole etc. whether or not shown on the drawings shall be protected from damage.
- 6. The contractor shall maintain service for all existing utilities until no longer necessary.
- 7. All trenching and shoring shall adhere to OSHA guidelines.
- 8. Contractor shall comply with all the requirements of the SPDES General Permit for Stormwater Discharges from Construction Activity - GP-0-10-001. A current copy of the Stormwater Pollution Prevention Plan (SWPPP) shall be keep on site at all times. Contractor is responsible for conducting weekly inspections (must be qualified by NYSDEC) or retaining a qualified inspector such as the design engineer to perform such inspections.

# **Excavation and Earthwork:**

- Prior to site disturbance the contractor shall install required erosion & sediment control
- 2. Strip all topsoil prior to commencing earthwork operations. Topsoil may be stored and reused in lawn and planting areas only.
- 3. Excavation shall be carried to the lines, grades and slopes shown on the approved plans. All final earthwork shall be smoothly and evenly blended into existing conditions.
- 4. Remove all vegetation, trees, stumps, grasses, organic soils, debris and deleterious materials from excavated soils to be reused as fill onsite.
- 5. Where unstable or unsuitable material is encountered at the prescribed bottom grade of the trenches it shall be removed.
- 6. Contractor shall be responsible for dewatering utility trenches and excavations and for the maintenance of surface drainage during the course of the work.
- 7. After final grading the contractor shall reapply stockpiled top soil on all lawn and planting areas. Topsoil shall be evenly spread a minimum of 4 (four) inches over all planting areas seeded and mulched in lawn areas or planted as per landscaping plan in planting beds. The contractor shall restore lawns, driveways and other disturbed areas to at least as good a condition as before being disturbed.

Utility Bedding and Backfill:

- Selected bedding (as specified on the utility typical trench sections heron) shall be provided for the construction of pipe foundations at those locations where the foundations or excavated material, or any portion thereof deemed to be unsuitable for supporting the pipe or structure, or for back filling the cover portion of the trenches to a level one foot above the pipe, or where excavated material consist of a predominance of large stone, boulders or rock which is not suitable for placing in the trench. Certified sieve analysis shall be submitted from the supplier for the engineer's review prior to use.
- 2. All suitable back fill material shall be placed in layers not exceeding twelve (12) inches in depth, (loose measure), and shall be thoroughly tamped and compacted to a minimum density of 95% standard AASHTO-T99 (ASTM-D698, as amended) compacting test. Compacting equipment shall be of a suitable type for the various back filling operations.

## Drainage:

1. All storm sewer pipe shall be smooth interior HDPE pipe unless noted otherwise

- HDPE end sections shall be provided on all drainage pipe inlets or of catch basins or other drainage structures. All outlets shall also be stabilized with rip-rap as per
- 3. All concrete chambers shall be pre cast concrete to the specifications and dimensions shown hereon. Frames and grates shall be gray iron or ductile iron. Gray iron shall conform with ASTM A 48, Class 30b and ductile iron shall conform with ASTM A 536 and be of a grade appropriate to its intended use to the dimensions and specifications as shown hereon. Any structures subject to vehicle loads shall be able to withstand an H20 loading. Shop drawings shall be submitted to the design engineer for approval prior to construction.
- 4. The gutters and ditches shall be kept open at all times for surface drainage. No damming or ponding of water, in gutters or other waterways will be permitted except where the engineer shall consider it necessary.
- 5. The transport of soils to the drainage system shall be avoided during and after construction.
- 6. All exposed soils shall be stabilized with vegetation, stone or as directed by the engineer.
- Methods used to control soil erosion and sediment control shall be in accordance with the approved soil erosion and sediment control plan or as directed by the engineer. Contractor shall comply with all the requirements of the SPDES General Permit for Stormwater Discharges from Construction Activity - GP-0-10-001. A current copy of the Stormwater Pollution Prevention Plan (SWPPP) shall be keep on site at all times. Contractor is responsible for conducting weekly inspections (must be qualified by NYSDEC) or retaining a qualified inspector such as the design engineer to perform such inspections.

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	MAP REVISION DATES	
DATE	REVISION	BY
03-27-2017	ADDED BIO SOIL CHARACTERISTICS CHART	SL
05-31-2017	ADDED WATER QUALITY BASIN OUTLET STRUCTURE DETAILS	CC
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	DRAINAGE DETAILS	
	FOR SENIOR HOUSING AT	
	21 LAKESIDE PROPERTIES INC.	
	SITUATE - LAKESIDE ROAD	
	TOWN OF NEWBURGH	
	ORANGE COUNTY, NEW YORK	
	FEBRUARY 8, 2016	
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